



TRANSNET SOC LTD

**DCT BERTHS 203 TO 205 - RECONSTRUCTION, DEEPENING AND
LENGTHENING**

PORT OF DURBAN

FACTUAL GEOTECHNICAL REPORT

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CONTENTS

1.0	INTRODUCTION AND TERMS OF REFERENCE.....	1
1.1	Introduction	1
1.2	Site Description	1
1.3	General Geological Environment	1
2.0	SUMMARY OF GEOTECHNICAL INVESTIGATIONS AND REPORTS.....	3
3.0	FIELD INVESTIGATIONS	4
3.1	Locations of Exploratory Points.....	4
3.2	Exploratory Drilling, SPT Testing and Sampling.....	4
3.2.1	Quay Wall	4
3.2.2	Dredge Area	5
3.3	Vibro-Coring and Grab Sampling.....	5
3.3.1	Borrow Site.....	5
3.4	Cone Penetration Testing (CPTu)	5
3.4.1	Quay Wall	5
3.4.2	Dredge Area	6
3.4.3	Lot 10 Casting Yard.....	6
4.0	LABORATORY TESTING	7
4.1	Classification Tests	7
4.1.1	Quay Wall	8
Dredge Area	9	
4.1.2	Borrow Site.....	9
4.2	Compressibility Tests	9
4.2.1	Quay Wall	10
4.3	Shear Strength Tests	10
4.3.1	Quay wall.....	10
5.0	GENERAL GEOTECHNICAL CONDITIONS	12
5.1	Quay Wall.....	12
5.1.1	Geology between berth chainage 960 m and 1220 m (Zone 1)	12
5.1.2	Geology between berth chainage 750 m and 960 m (Zone 2)	12
5.1.3	Geology between berth chainage 430 m and 750 m (Zone 3)	12
5.1.4	Geology between berth chainage 170 m and 430 m (Zone 4)	12
5.1.5	Geology between berth chainage 40 m and 170 m (Zone 5)	13
5.1.6	Geology between berth chainage 0 m to 40 m and return wall chainage 0 to 72 m (Zone 6)	13
5.1.7	Geology between return wall chainage 72 m and 221 m (Zone 7)	13
5.2	Dredge Area	15
5.3	Borrow Site.....	15
5.4	Lot 10 Casting Yard.....	15
ANNEXURES	1	
ANNEXURE 1: DRAWINGS.....	2	
ANNEXURE 2: EXPLORATORY BOREHOLE LOGS.....	3	



2.1: EXPLANATORY NOTE ON BOREHOLE LOGS AND SAMPLING	4
2.2: FAIRBROTHER GEOTECHNICAL METHOD STATEMENT	5
2.3: BOREHOLE LOGS – QUAY WALL	6
2.4: BOREHOLE LOGS – BASIN DREDGE AREA.....	7
ANNEXURE 3: CPTU RESULTS	8
3.1: EXPLANATORY NOTE ON CPTU LOGS	9
3.2: OSIMO METHOD STATEMENT	10
3.3: CPTU LOGS – QUAY WALL	11
3.4: CPTU LOGS – BASIN DREDGE AREA	12
3.5: CPTU LOGS – LOT 10 CASTING YARD	13
ANNEXURE 4: LABORATORY TEST RESULTS.....	14
4.1: LABORATORY TEST RESULTS – QUAY WALL.....	15
4.2: LABORATORY TEST RESULTS – BASIN DREDGE AREA.....	16
ANNEXURE 5: PREVIOUS GEOTECHNICAL INVESTIGATION REPORTS	17
5.1: FACTUAL GEOTECHNICAL REPORT FOR PHASE 2: PIER 2: BERTHS 203 TO 205	18
5.2: LABORATORY REPORT FOR PHASES 1, 2 & 3: VOLUME 1 & 2.....	19
5.3: FACTUAL REPORT ON MARINE CONE PENETRATION TESTS	20
5.4: DESK STUDY APPRAISAL FOR THE DURBAN BERTH DEEPENING FEASIBILITY STUDY	21
5.5: A GEOPHYSICAL AND SEDIMENT SAMPLING SURVEY OF TWO PROPOSED SAND WINNING AREAS IN THE DURBAN BIGHT	22
5.6: A GEOPHYSICAL AND SEDIMENTOLOGICAL INVESTIGATION OF THE OFFSHORE DREDGE DISPOSAL SITE, DURBAN	23

LIST OF FIGURES

Figure 1.2-1: Satellite Image Showing the Location of Pier 2 and Dredge Area.....	1
Figure 1.3-1: Satellite Image Showing the Location of the Offshore Borrow Site and the Lot 10 Casting Yard.	2
Figure 1.3-2: Satellite Image Showing the Approximate Boundaries of the Lot 10 Casting Yard.....	2
Figure 5.1-1: Quay Wall Geological Long Section	14

LIST OF TABLES

Table 1.3.1: Previous Studies and Factual Geotechnical Reports Relevant to the Project.....	3
Table 3.4.1: Summary of Laboratory Testing.....	7
Table 4.1.1: Summary of Classification Tests for the Quay Wall Area.....	8
Table 4.1.2: Summary of Classification Tests for the Basin Dredge Area	9
Table 4.1.3: Summary of Classification Tests for the Borrow Site.....	9
Table 4.2.1: Summary of Oedometer Tests for the Quay Wall Area	10
Table 4.3.1: Summary of Shear Strength Tests for the Quay Wall Area.....	11

1.0 INTRODUCTION AND TERMS OF REFERENCE

1.1 Introduction

Extensive geotechnical site investigations have been undertaken as part of the DCT Berths 203 to 205 Reconstruction, Deepening and Lengthening Project in the Port of Durban. This Report is a collation of the factual geotechnical information relevant to the Project.

The geotechnical site investigations have concentrated on four main areas, namely the new quay wall line, within the dredge basin, at the offshore borrow site and at the Lot 10 casting yard. The field investigation data and laboratory testing data are presented separately and further subdivided according to the exploratory area as described above.

1.2 Site Description

Berths 203 to 205 are being deepened and a new quay wall constructed which is to be offset by approximately 50 m seaward of the existing quay wall, as shown in Figure 1.2-1. The quay is also being lengthened by extending the western (205) end of the new quay into the existing crane yard, known as the Berth 205 extension.

The basin dredge area comprises the existing basin adjacent to the existing Berths 203 to 205 and the existing turning circle adjacent to the T-Jetty and Pier 1. The offshore borrow site is located 2 km east of the Port entrance and is shown in Figure 1.3-1. The lot 10 casting yard is located approximately 1.2 km south west of Pier 2 as seen in Figures 1.3.1 and 1.3.2.



Figure 1.2-1: Satellite Image Showing the Location of Pier 2 and Dredge Area.

1.3 General Geological Environment

Durban harbour is underlain at depth by bedrock of Cretaceous age. This Cretaceous bedrock in the vicinity of Pier 2 is generally located at a depth of between -30 m CDP and -40 m CDP (See Figure 5.1-1). The bedrock depth is locally as deep as -65 m CDP where a paleo channel cuts across the site at Berths 203 and 204.

Overlying the bedrock is a thick sequence of unconsolidated soils comprising sands, silts, and clay. These transported soils are known as the Harbour Beds and are of variable but mostly estuarine origin. Their depositional environment has resulted in both lateral and vertical variation of sediment type and thickness. The soil profile at Pier 2 can generally be described as having an upper unit comprising silty clays, which is generally underlain by a unit comprising mainly sands, with bedrock at depth.

The offshore borrow site comprises thick accumulations of slightly calcareous sands with a few scattered reef outcrops. The sea bed level at the borrow site is at approximately -20 m CDP.



Figure 1.3-1: Satellite Image Showing the Location of the Offshore Borrow Site and the Lot 10 Casting Yard.



Figure 1.3-2: Satellite Image Showing the Approximate Boundaries of the Lot 10 Casting Yard.



2.0 SUMMARY OF GEOTECHNICAL INVESTIGATIONS AND REPORTS

Various geotechnical investigations have been undertaken as part of the Project. The latest phase of geotechnical investigations, namely the FEL 3 phase, supplemented the FEL 2 investigation, adding exploratory data points along the new quay wall line as well as in the basin dredge area.

Table 1.3.1 presents the previous geotechnical investigation reports relevant to the Project. These reports are provided in the Annexure 5 of this Report.

Table 1.3.1: Previous Studies and Factual Geotechnical Reports Relevant to the Project

Annexure 5.1	Factual Geotechnical Report for Phase 2: Pier 2: Berths 203 to 205 (MSJ RPT No. 07-395R07Rev0; May 2009)
Annexure 5.2	Laboratory Report for Phases 1, 2 & 3: Volume 1 & 2 (MSJ RPT No. 07-395Rev0; June 2009)
Annexure 5.3	Factual Report on Marine Cone Penetration Tests (Frankie RPT No. Rev.0; June 2009)
Annexure 5.4	Desk Study Appraisal for the Durban Berth Deepening Feasibility Study (MSJ RPT No. 07-395; May 2008)
Annexure 5.5	A Geophysical and Sediment Sampling Survey of Two Proposed Sand Winning Areas in the Durban Bight (CGS RPT No. 2001-0158; 2001)
Annexure 5.6	A Geophysical and Sedimentological Investigation of the Offshore Dredge Disposal Site, Durban (CGS RPT No. 2001-0002; 2001)



3.0 FIELD INVESTIGATIONS

The following subsections summarise the field investigations relevant to the Project.

3.1 Locations of Exploratory Points

The various geotechnical investigations have concentrated on four main areas. The first area covers the new quay wall, including the return quay. The second area is defined by the dredging limits for the berth deepening. The third area is defined by the extents of the offshore borrow site where sand will be won for reclamation and the fourth area covers the Lot 10 casting yard.

The exploratory points comprise borehole drilling with *in situ* SPT testing and sampling, cone penetration testing (CPTu), vibro coring, and grab sampling.

Drawing 1370-CO-180-DWG-0001-01 and 1370-CO-180-DWG-0003-01 in Annexure 1 show the locations of the various types of exploratory points within the new quay wall area and dredge area. The points relevant to the offshore borrow site are shown in Figure 1 of geotechnical report A6 (Refer to Section 2.0). The exploratory point locations at Lot 10 can be seen in Figure 1.3-2.

3.2 Exploratory Drilling, SPT Testing and Sampling

3.2.1 Quay Wall

General:

A total of 23 No. exploratory boreholes have been drilled along a centreline that coincides with the new quay wall cope line (Refer to drawings 1370-CO-180-DWG-0001-01 and 02, and 1370-CO-180-DWG-0002-01 and 02). This centreline is approximately 50 m seaward of the existing quay wall cope line. Of the 23 No. exploratory boreholes, 9 were land based boreholes and 8 were marine based boreholes. An additional 6 No. marine based boreholes from the FEL 2 study have been included in this Report.

The boreholes were drilled from land or seabed level to a depth terminating 10 m to 15 m into bedrock. The borehole drilling involved Standard Penetration Testing, disturbed sampling, and undisturbed thin wall Shelby sampling.

The exploratory boreholes were drilled during the FEL 2 and FEL 3 phases of investigation. The “BHM 2##” and “BHL 2##” series of boreholes are from the FEL 2 investigation. The “BHS##” and “BHL##” series of boreholes are from the FEL 3 investigation.

Methodology:

The exploratory drilling, sampling and logging methodologies are detailed in Annexure 2 of this Report for the FEL 3 phase investigation, and in geotechnical report A4 (Refer to Section 2.0) for the FEL 2 phase investigation.

Results:

As-built coordinates, elevations, logs and photographs for the exploratory boreholes are located in Annexure 2 of this Report.

The laboratory test results on disturbed and undisturbed samples are described in Section 4.0 of this Report.



3.2.2 Dredge Area

General:

A total of 10 No. marine based exploratory boreholes have been drilled in the dredge area. These boreholes were used to proof selected CPTu profiles. The boreholes were positioned directly over selected CPTu's (Refer to drawings 1370-CO-180-DWG-003-01 & 02) in order to calibrate the CPTu classification of soil type and state.

The boreholes were drilled from seabed level, to a depth approximately 7 m below seabed level. The borehole drilling involved Standard Penetration Testing and disturbed sampling.

The boreholes were drilled during the FEL 3 phases of investigation and are identified as the "BDS#" series of boreholes.

Methodology:

The exploratory drilling, sampling and logging methodologies are detailed in Annexure 2 of this Report.

Results:

As-built coordinates, elevations, logs and photographs for the exploratory boreholes are summarised Annexure 2.

The laboratory test results on disturbed and undisturbed samples are described in Section 4.0 of this Report, and are presented in Annexure 4.

3.3 Vibro-Coring and Grab Sampling

3.3.1 Borrow Site

General:

A total of 3 No. vibro-cores were collected at the borrow site to a depth of approximately 4.0 m below seabed level. A total of 16 No. grab samples were collected from the seabed.

Methodology:

The coring, sampling and logging methodologies are detailed in geotechnical report A5.6 (Refer to Section 2.0).

Results:

The as-built coordinates, location and elevations for the vibro-core and grab samples, as well as the core logs are provided in geotechnical report A5.6.

The laboratory testing on disturbed samples is also described in geotechnical report A5.6. A summary table of the results of the testing can be found in Section Table 4.1.3 of this Report.

3.4 Cone Penetration Testing (CPTu)

3.4.1 Quay Wall

General:

A total of 50 No. CPTu tests with dissipation testing have been carried out along the centreline that delineates the proposed new quay wall copeline (Refer to drawings 1370-CO-180-DWG-0001-01 to 03). This centreline is approximately 50 m seaward of the existing quay wall cope line. Of the 50 No. CPTu probes, 8 were land based probes and 42 were marine based probes. 2 No. CPTu's from the FEL 2 study have been included in this Report.

The "CPT##" series of CPTu's are from the FEL 3 investigation. The "CPTM 2##" series of tests are from the FEL 2 investigation.



Methodology:

The CPTu test methodology is detailed in Annexure 2 and Annexure 3 of this Report for the FEL 3 phase investigation and in geotechnical report A5.3 and A5.4 (Refer to Section 2.0) for the FEL 2 phase investigation.

Limitations and Exceptions:

The marine CPTu's refused at approximately -24 m CDP. Sleeve friction was not measured for the majority of tests (FEL3).

Results:

As-built coordinates, elevations, dissipation tests and logs for the CPTu tests are presented in Annexure 3 of this Report.

3.4.2 Dredge Area

General:

A total of 88 No. CPTu tests with dissipation tests have been carried out within the dredge area on a grid spacing of 100 m by 100 m. The CPTu estimation of soil type and state has been calibrated with proofing boreholes as discussed in Section 3.2.2.

CPTu testing was carried out from seabed level, to a depth of approximately 7 m below seabed level.

The CPTu tests formed part of the FEL 3 phase of investigation and are identified as the "CPTS## or CPTP##" series of CPTu's.

Methodology:

The CPTu test methodology is detailed in Annexure 2 and Annexure 3 of this Report.

Limitations and Exceptions:

Sleeve friction was not measured for the dredge area tests.

Results:

As-built coordinates, elevations, dissipation tests and logs for the CPTu tests are tabled in Annexure 3 of this Report.

3.4.3 Lot 10 Casting Yard

General:

A total of 5 No. CPTu tests with dissipation testing have been carried out within the Lot 10 casting yard.

CPTu testing was carried out from ground level to a depth of approximately 20 m. The CPTu tests formed part of the FEL 3 phase of the investigation and are identified as the "CPTU" series of CPTu's.

Methodology:

The CPTu test methodology is detailed in Annexure 2 and Annexure 3 of this Report.

Limitations and Exceptions:

Sleeve friction was not measured for the casting yard tests.

Results:

As-built coordinates, elevations, dissipation tests and logs for the CPTu tests are tabled in Annexure 3 of this report.



4.0 LABORATORY TESTING

The following subsections summarise the laboratory testing relevant to the Project. Laboratory testing was carried out for both the FEL 2 and FEL 3 phases of investigation. Various laboratories were used and therefore there is some variation in the testing standards used. The summarised laboratory test results presented in the following tables represent the factual information only without engineering interpretation (unless otherwise stated).

The type of laboratory test, numbers of tests, testing laboratory and testing standards, are summarised in Table 3.4.1. The laboratory test results are presented in Annexure 4.

The soils have been classified according to BS EN ISO 14688-1:2002+A1:2013, BS EN ISO 14688-2:2004+A1:2013 and BS 5930:2015.

Table 3.4.1: Summary of Laboratory Testing

Investigation area	Test type	Test	No of tests	Testing laboratory	Test standard
Quay wall	Classification	Specific Gravity	5	SW	-
		Hydrometer	31	TS, SW, SC, NMTL	TMH A6, ASTM D422, ASTM D422, BS 1377
		Sieve analysis	36	TS, SW, SC, NMTL	TMH A1, ASTM D6913, TMH A1, BS 1377
		Atterberg limits	32	TS, SW, SC, NMTL	BS 1370, ASTM D4318, TMH A3, BS 1377
	Compressibility	Oedometer Tests	12	SU	BS 1377
		Triaxial UU Tests	14	TS, SU, NMTL	BS 1377
		Triaxial CIUC Tests	7	SU, NMTL	BS 1377
	Shear strength	Shear Vane Tests	22	SU	-
Dredge area		Hydrometer	11	TS	TMH A6
Classification	Sieve analysis	11	TS	TMH A1(a)	
	Organic content	11	TS	BS 1377	
Borrow Site	Classification	Sieve analysis	46	CGS	-
		Carbonate content	46	CGS	Carbonate bomb analysis

Note: TS - Thekwini Soils Laboratory cc, SW - GeoTegnniese Laboratorium Somerset-wes, SC - Soilco Materials Investigations (Pty) Limited, SU - Stellenbosch University, NMTL - National Materials Testing Laboratory LTD, CGS - Council for Geoscience

4.1 Classification Tests

Table 4.1.1 toTable 4.1.3 summarise the classification tests carried out for the Quay Wall, Dredge Area and borrow site.

Grain size distribution (or grading) was determined by sieve analysis and sedimentation analysis (Hydrometer method). Other classification tests included moisture content, Atterberg Limits, particle density, organic content, and carbonate content.



4.1.1 Quay Wall

Table 4.1.1: Summary of Classification Tests for the Quay Wall Area

BH No	Sample No.	Testing laboratory	Sample Details		Grading (BS 5930:2015 classification)					Atterberg Limits				Moisture content (%)	Specific Gravity	Soil Name BS EN 5930:2015
			Top CDP	Base CDP	Clay <0.002 mm (%)	Silt .002-0.063 mm (%)	Fine Sand 0.063-0.2 mm (%)	Medium Sand 0.2-0.63 mm (%)	Coarse Sand 0.63-2.0 mm (%)	Gravel >2mm (%)	LL (%)	PI (%)	Linear Shrinkage (%)			
BHL01	S01	SW	-16.28	-16.88	20.0	46.0	30.0	4.0	0.0	0.0	51	19	13	60	2.49	high plasticity, slightly sandy clayey SILT/silty CLAY
BHL01	S02	SW	-17.78	-18.38	20.0	47.0	33.0	0.0	0.0	0.0	69	28	15	91	2.42	high plasticity, slightly sandy clayey SILT/silty CLAY
BHL01	S03	SW	-21.78	-22.38	31.0	48.0	21.0	0.0	0.0	0.0	66	29	16	72	2.46	high plasticity, slightly sandy clayey SILT/silty CLAY
BHL01	S06	SW	-25.38	-25.98	45.0	43.0	12.0	0.0	0.0	0.0	57	24	14	57	2.75	high plasticity, slightly sandy clayey SILT/silty CLAY
BHL01	S07	SW	-26.48	-27.08	31.0	48.0	16.0	4.0	1.0	0.0	46	15	12	53	2.55	intermediate plasticity slightly sandy clayey SILT/silty CLAY
BHS04	S01	TS	-20.33	-20.98	49.9	42.7	5.8	0.9	0.4	0.0	55	24	13	53	-	high plasticity slightly sandy clayey SILT/silty CLAY
BHS05	S01	TS	-20.00	-20.60	50.5	28.9	12.5	7.1	0.7	0.3	50	21	12	56	-	intermediate/high plasticity slightly sandy clayey SILT/silty CLAY
BHS06	S02	TS	-22.30	-22.90	47.1	38.6	5.4	4.6	3.8	0.5	49	20	11	53	-	intermediate/high plasticity slightly gravelly slightly sandy clayey SILT/silty CLAY
BHS06	S03	TS	-26.80	-27.40	25.4	41.3	24.0	8.9	0.5	0.0	35	12	5	30	-	low/intermediate plasticity slightly sandy clayey SILT/silty CLAY
BD-BHM207	*5129	SC	-25.01	-25.46	-	5.0	94.0			1.0	NP	-	-	-	-	slightly silty SAND
BD-BHM207	*5130	SC	-32.51	-32.96	-	20.0	79.0			1.0	NP	-	-	-	-	silty SAND
BD-BHM208	*5131	SC	-20.62	-21.07	-	7.0	93.0			0.0	NP	-	-	-	-	silty SAND
BD-BHM208	*5132	SC	-26.62	-27.07	-	19.0	79.0			2.0	NP	-	-	-	-	silty SAND
BD-BHM209	1	NMTL	-17.51	-17.98	26.0	73.0	1.0			0.0	89	50	-	56.8	-	very high/extremely high plasticity slightly sandy clayey SILT/silty CLAY
BD-BHM209	*5141	SC	-17.98	-18.43	18.0	43.0	39.0			0.0	45	18	-	-	-	intermediate plasticity sandy clayey SILT/silty CLAY
BD-BHM209	*5142	SC	-31.88	-32.33	9.0	38.0	53.0			0.0	33	11	-	-	-	low plasticity sandy clayey SILT/silty CLAY
BD-BHM209	1	NMTL	-0.78	-1.25	26	73	1			0	89	50	-	1	-	very high/extremely high plasticity slightly sandy clayey SILT/silty CLAY
BHS03	S01	TS	-20.94	-21.51	39.0	55.5	4.9	0.7	0.0	0.0	35	21	11	62	-	low/intermediate plasticity slightly sandy clayey SILT/silty CLAY
BHS03	S02	TS	-22.94	-23.51	53.2	40.9	4.7	1.1	0.0	0.0	62	26	13	54	-	high plasticity slightly sandy clayey SILT/silty CLAY
BHS03	S03	TS	-24.94	-25.51	29.7	59.6	7.8	2.7	0.2	0.0	45	18	9	39	-	intermediate plasticity slightly sandy clayey SILT/silty CLAY
BHS03	S04	TS	-26.94	-27.51	21.7	53.3	18.6	6.0	0.3	0.0	41	16	8	-	-	intermediate plasticity slightly sandy clayey SILT/silty CLAY
BD-BHM210A	1	NMTL	-25.78	-27.28	14.0	74.0	12.0			0.0	71	47	-	44	-	very high plasticity slightly sandy clayey SILT/silty CLAY
BD-BHM210A	2	NMTL	-25.78	-27.28	10.0	67.0	18.0			5.0	61	39	-	36	-	high plasticity slightly sandy clayey SILT/silty CLAY
BD-BHM211	*5133	SC	-15.19	-15.64	20.0	52.0	27.0			1.0	47	20	-	-	-	intermediate plasticity slightly sandy clayey SILT/silty CLAY
BD-BHM211	*5134	SC	-22.59	-23.04	16.0	45.0	39.0			0.0	42	18	-	-	-	intermediate plasticity sandy clayey SILT/silty CLAY
BD-BHM211	*5135	SC	-24.09	-24.54	16.0	39.0	45.0			0.0	37	16	-	-	-	intermediate plasticity sandy clayey SILT/silty CLAY
BD-BHM211	*5136	SC	-25.59	-26.04	-	8.0	92.0			0.0	NP	-	-	-	-	silty SAND
BD-BHM211	1	NMTL	-27.27	-27.42	24.0	73.0	3.0			0.0	81	52	-	61.1	-	very high plasticity slightly sandy clayey SILT or silty CLAY
BD-BHM211	2	NMTL	-27.42	-27.61	14.0	82.0	4.0			0.0	76	50	-	55.2	-	very high plasticity slightly sandy clayey SILT or silty CLAY
BD-BHM211	3	NMTL	-27.61	-27.75	14.0	69.0	17.0			0.0	62	41	-	52.2	-	high plasticity slightly sandy clayey SILT or silty CLAY
BD-BHM211	4	NMTL	-27.75	-28.11	21.0	65.0	13.0			0.0	77	47	-	53.6	-	very high plasticity slightly sandy clayey SILT or silty CLAY
BD-BHM211	5	NMTL	-28.11	-28.29	26.0	65.0	9.0			0.0	86	57	-	63.5	-	very high plasticity slightly sandy clayey SILT or silty CLAY



Dredge Area

Table 4.1.2: Summary of Classification Tests for the Basin Dredge Area

BH No	Sample No.	Testing laboratory	Sample Details		Grading (BS 5930:2015 classification)					Organic content	Soil Name BS EN 5930:2015
			Top CDP	Base CDP	Clay <0.002 mm (%)	Silt .002 - 0.06 mm (%)	Fine sand 0.06 - 0.2 mm (%)	Medium Sand 0.2 - 0.6 mm (%)	Coarse Sand 0.6- 2.0 mm (%)		
BDS01	S04	TS	-18.57	-19.57	5.5	5.1	59.6	29.8	0.0	1.8	slightly organic clayey silty SAND
BDS02	S06	TS	-19.51	-20.21	5.8	3.8	52.2	38.0	0.2	1.8	slightly organic clayey silty SAND
BDS03	S05	TS	-16.15	-17.25	5.7	2.9	51.5	39.6	0.4	1.7	slightly organic clayey silty SAND
BDS03	S07	TS	-17.65	-18.45	41.2	31.2	20.1	7.1	0.4	11.6	organic slightly sandy clayey SILT/silty CLAY
BDS04	S02	TS	-14.66	-15.16	30.1	36.1	22.7	11.0	0.2	1.8	organic slightly sandy clayey SILT/silty CLAY
BDS04	S03	TS	-20.66	-21.11	5.8	6.7	30.0	50.7	6.9	1.8	slightly organic silty clayey SAND
BDS04	S08	TS	-16.66	-17.16	41.8	51.9	4.9	0.0	0.0	14.1	organic slightly sandy clayey SILT/silty CLAY
BDS04	S09	TS	-17.66	-19.16	46.5	46.8	5.8	0.7	0.1	13.5	organic slightly sandy clayey SILT/silty CLAY
BDS05	S01	TS	-18.61	-19.06	6.7	6.5	54.5	32.2	0.1	1.8	slightly organic clayey silty SAND
BDS08	S01	TS	-15.06	-18.51	6.9	2.6	32.7	50.9	0.0	1.1	slightly organic slightly clayey silty SAND
BDS10	S01	TS	-13.57	-19.02	7.3	5.4	51.6	35.2	0.2	1.0	slightly organic clayey silty SAND

4.1.2 Borrow Site

Table 4.1.3: Summary of Classification Tests for the Borrow Site

Sample number (n)= 16	Median (mm)	Mean (mm)	Sorting (mm)	Grading (BS 5930:2015 classification)			Carbonate	Soil Name BS EN 5930:2015
				*Clay & Silt <0.063 mm (%)	Sand 0.063 - 2mm (%)	Gravel >2mm (%)		
Min	0.213	0.227	0.25	0.97	99.03	0.00	8.85	calcareous slightly silty/clayey SAND
Max	0.444	0.474	0.62	1.75	96.90	1.35	23.47	calcareous slightly gravelly slightly silty/clayey SAND
Ave	0.304	0.322	0.45	1.34	98.17	0.49	12.77	calcareous slightly gravelly slightly silty/clayey SAND
Colour	Light olive - light reddish brown							
Sediment type	Well sorted medium-grained sand							
Sediment maturity	Subangular to subrounded							

* Termed "Mud" in Council for Geoscience Report

4.2 Compressibility Tests

The standard oedometer consolidation test was performed on selected undisturbed samples for the determination of the consolidation characteristics of the soils of low permeability. The two parameters required were:

- Coefficient of volume compressibility, M_v (Compressibility of soil)
- Coefficient of consolidation, C_v (Time related parameter)

The preconsolidation pressure (P_c) is estimated based on an interpretation of the consolidation test data. The Casagrande graphical method and the Work method were used to determine the preconsolidation pressure.

The coefficient of consolidation was not determined by oedometer testing. An estimation of the coefficient of consolidation may be made based on CPTu dissipation test results.

Table 4.2.1 presents the compressibility test results for the Quay Wall Area.



4.2.1 Quay Wall

Table 4.2.1: Summary of Oedometer Tests for the Quay Wall Area

BH no	Sample No.	Testing laboratory	Top CDP	Base CDP	Initial MC (%)	Bulk density (kg/m³)	Dry density (kg/m³)	Pc' (kPa)	Mv (100-200 kPa)) (m²/MN)	Mv (200-400 kPa) (m²/MN)	Initial void ratio (e₀)	Soil Name BS EN 5930:2015
BHL01	S01A	SU	-16.28	-16.88	48	1731	1168	117	0.604	0.288	1.31	high plasticity, slightly sandy clayey SILT/silty CLAY
BHL01	S01B	SU	-16.28	-16.88	61	1574	977	173	0.591	0.527	1.76	high plasticity, slightly sandy clayey SILT/silty CLAY
BHL01	S02A	SU	-17.78	-18.38	79	1362	760	108	0.450	0.520	2.55	high plasticity, slightly sandy clayey SILT/silty CLAY
BHL01	S02B	SU	-17.78	-18.38	93	1321	684	240	0.254	0.337	2.95	high plasticity, slightly sandy clayey SILT/silty CLAY
BHL01	S03	SU	-21.78	-22.38	66	1517	914	100	0.350	0.383	1.95	high plasticity, slightly sandy clayey SILT/silty CLAY
BHL01	S05A	SU	-24.28	-24.88	56	1703	1093	95	0.460	0.351	1.47	*high plasticity silty CLAY
BHL01	S05B	SU	-24.28	-24.88	48	1657	1119	100	0.429	0.338	1.41	*high plasticity silty CLAY
BHL01	S07A	SU	-26.48	-27.08	48	1677	1135	92	0.309	0.350	1.38	intermediate plasticity slightly sandy clayey SILT/silty CLAY
BHL01	S07B	SU	-26.48	-27.08	57	1638	1046	70	0.422	0.256	1.58	intermediate plasticity slightly sandy clayey SILT/silty CLAY
BHS03	S05	SU	-28.94	-29.50	45	1636	1132	125	0.275	0.321	1.39	*high plasticity silty CLAY
BHS03	S06A	SU	-30.94	-31.14	40	1767	1260	388	0.107	0.160	1.14	*high plasticity silty CLAY
BHS03	S06B	SU	-30.94	-31.14	41	1632	1155	358	0.155	0.114	1.34	*high plasticity silty CLAY

Note: * Field description

4.3 Shear Strength Tests

4.3.1 Quay wall

Shear strength tests were performed on undisturbed samples and on selected double tube core samples. Three types of shear strength tests were performed:

- Hand shear vane test
- Triaxial Unconsolidated Undrained Compression (UU) Test
- Triaxial Consolidated Isotopically Undrained Compression (CIUC) Test

The shear strength testing enabled the determination of both undrained and effective shear strength parameters. Table 4.3.1 presents the shear strength test results for the Quay Wall Area.



Table 4.3.1: Summary of Shear Strength Tests for the Quay Wall Area

BH no	Sample No.	Testing laboratory	Sample type	Test type	Top CDP	Base CDP	Soil Name BS EN 5930:2015	Ave Bulk density (kg/m3)	Ave Dry density (kg/m3)	Initial MC %	Av initial MC %	Cu (kPa) - Triaxial	Av Cu (kPa)	Cu (kPa) - Hand vane	Av Cu (kPa)	CIUC (Φ',c')
BHL01	S01	SU	Shelby	UU	-16.28	-16.88	high plasticity, slightly sandy clayey SILT/silty CLAY	-	-	61.11, 61.11, 48.11	57	30, 42, 27	33	28, 32, 32, 34	32	-
BHL01	S02	SU	Shelby	UU, HV	-17.78	-18.38	high plasticity, slightly sandy clayey SILT/silty CLAY	-	-	93.0, 93.0, 79.17	88	42, 34, 46	41	32, 36, 34, 36	35	-
BHL01	S03	SU	Shelby	UU, HV	-21.78	-22.38	high plasticity, slightly sandy clayey SILT/silty CLAY	-	-	-	-	43, 42	43	36, 38, 38, 39	38	-
BHL01	S05	SU	Shelby	CIUC, HV	-24.28	-24.88	*high plasticity silty CLAY	-	-	48.02, 55.82	52	-	-	38, 38, 39, 41, 40	39	Yes
BHL01	S07	SU	Shelby	UU, HV	-24.28	-24.88	intermediate plasticity slightly sandy clayey SILT/silty CLAY	-	-	56.53, 56.53, 47.72	54	13, 9, 13	12	7, 9, 6, 9	8	-
BHS04	S01	TS	Shelby	UU	-20.33	-20.98	high plasticity slightly sandy clayey SILT/silty CLAY	1697	1111	51.1, 52.9, 54.0	53	105, 144, 181	143	-	-	-
BHS05	S01	TS	Shelby	UU	-20.00	-20.60	intermediate/high plasticity slightly sandy clayey SILT/silty CLAY	1688	1079	56.0, 56.2, 57.1	56	65, 102, 119	95	-	-	-
BHS06	S02	TS	Shelby	UU	-22.30	-22.90	intermediate/high plasticity slightly gravelly slightly sandy clayey SILT/silty CLAY	1674	1097	52.0, 52.2, 53.6	53	167, 184, 252	201	-	-	-
BHS06	S03	TS	Shelby	UU	-26.80	-27.40	low/intermediate plasticity slightly sandy clayey SILT/silty CLAY	1951	1471	33.1, 32.0, 23.4	30	374, 407, 524	435	-	-	-
BD-BHM209	1	NMTL	Shelby	CIUC	-17.51	-17.98	very high/extremely high plasticity slightly sandy clayey SILT/silty CLAY	1581	1010	56.8	57	-	-	-	-	Yes
BD-BHM209	2	NMTL	Shelby	CIUC	-17.51	-17.98	*high plasticity silty CLAY	1651	1110	47.77	48	-	-	-	-	Yes
BD-BHM209	3	NMTL	Shelby	CIUC	-17.51	-17.98	*high plasticity silty CLAY	1581	980	60.65	61	-	-	-	-	Yes
BHS03	S01	TS	Shelby	UU	-20.94	-21.51	low/intermediate plasticity slightly sandy clayey SILT/silty CLAY	1589	981	62.1	62	185	185	-	-	-
BHS03	S02	TS	Shelby	UU	-22.94	-23.51	high plasticity slightly sandy clayey SILT/silty CLAY	1685	1096	53.6	54	415	415	-	-	-
BHS03	S03	TS	Shelby	UU	-24.94	-25.51	intermediate plasticity slightly sandy clayey SILT/silty CLAY	1806	1303	38.4, 39.5, 38.1	39	129, 146, 151	142	-	-	-
BHS03	S05	SU	Shelby	UU	-28.94	-29.51	*high plasticity silty CLAY	-	-	36.07, 36.04	36	47, 57	52	-	-	-
BD-BHM210A	-	NMTL	TW	UU	-25.97	-26.17	very high plasticity slightly sandy clayey SILT/silty CLAY	1721	-	44.2	44	46.8	47	-	-	-
BD-BHM210A	-	NMTL	TW	UU	-26.77	-26.96	high plasticity slightly sandy clayey SILT/silty CLAY	1741	-	36	36	57.9	58	-	-	-
BD-BHM211	1	NMTL	TW	CIUC	-27.27	-27.42	very high plasticity slightly sandy clayey SILT or silty CLAY	1661	1000	66.87	67	-	-	-	-	Yes
BD-BHM211	2	NMTL	TW	CIUC	-27.42	-27.61	very high plasticity slightly sandy clayey SILT or silty CLAY	1661	1070	55.19	55	-	-	-	-	Yes
BD-BHM211	3	NMTL	TW	CIUC	-27.61	-27.75	high plasticity slightly sandy clayey SILT or silty CLAY	1751	1150	52.18	52	-	-	-	-	Yes
BD-BHM211	4	NMTL	TW	CIUC	-27.75	-28.11	very high plasticity slightly sandy clayey SILT or silty CLAY	1761	1150	53.42	53	-	-	-	-	Yes

Note: *Field description, TW - samples retrieved from double tube coring, UU - Unconsolidated Undrained Triaxial Compression Test, HV - Shear Vane, CIUC - Consolidated-Undrained Triaxial Compression Test



5.0 GENERAL GEOTECHNICAL CONDITIONS

5.1 Quay Wall

The geology along the new quay wall line has been divided into seven zones based on the changing geotechnical conditions. A brief description of each zone is provided in the following subsections. The seven geotechnical zones are shown in drawings 1370-CO-180-DWG-0001-01 to 03 as well as Figure 5.1-1.

5.1.1 Geology between berth chainage 960 m and 1220 m (Zone 1)

Zone 1 is characterised by an upper unit comprising predominately clay. This unit is approximately 17 m thick, starting at seabed level (-12.72 MCDP), and terminating at approximately -30 m CDP. Underlying the silty clay is a sand unit with minor alternating horizons of gravel and clayey sand. This sand unit varies in thickness from 13 m to 36 m and has accumulated in a paleo channel cuts through the Cretaceous bedrock. Bedrock comprises very soft to hard siltstone and varies in depth from -65 m CDP in the east, shallowing to -32 m CDP in a westerly direction.

Typical uncorrected SPT N values in the upper silty clay unit range between 0 and 46 with an average of 16. Typical uncorrected cone resistance values range between 3 – 4 MPa, with an average value of 3.5 MPa.

Typical uncorrected SPT values in the lower sand unit range between 13 and 60 with an average of 35, although there is considerable localised variation.

5.1.2 Geology between berth chainage 750 m and 960 m (Zone 2)

Zone 2 is characterised by a thick unit of sand, with isolated lenses of silty clays in the upper 16 m. The total thickness of this upper unit is 21m, starting at seabed level and terminating at -30 m CDP. Underlying these sands is a dense to very dense sand with isolated boulders of very soft to hard siltstone. The thickness of this unit varies from 0 m to 35 m as the bedrock profile dips steeply to the west. The bedrock level varies from -28 m CDP to > -68 m CDP.

Typical uncorrected SPT N values in the upper and lower sand units range from 15 to 62 with an average of 34. Typical uncorrected cone resistance values in the upper sand unit range between < 1 – 30 MPa, with an average value of 7.5 MPa becoming 20 MPa with depth.

5.1.3 Geology between berth chainage 430 m and 750 m (Zone 3)

Zone 3 is characterised by an upper unit comprising clays interbedded with silty sands. This unit is approximately 13 to 18 m thick, starting at seabed level and terminating at approximately -26 to -30 m CDP. Underlying the silty clay is a sand. This sand unit varies in thickness from 8 m to 24 m. Bedrock comprises very soft to hard siltstone and varies in depth from -67 m CDP in the east, shallowing to -41 m CDP in a westerly direction.

Typical uncorrected SPT N values in the upper silty clay unit range between 0 and 17 with an average of 7. Typical uncorrected cone resistance values range between 1.5 – 5 MPa. Thin interbeds of silty clays result in cone resistance values < 1MPa locally.

Typical uncorrected SPT N values in the lower sand unit range between 10 and 29 with refusal at depth.

5.1.4 Geology between berth chainage 170 m and 430 m (Zone 4)

Zone 4 is characterised by an upper unit comprising silty clay. This unit is approximately 10 m thick, starting at seabed level and terminating at a maximum depth of -28 CDP. Underlying the silty clay is a sand unit comprising sand with minor interbeds of clayey sand and gravels. This sand unit varies in thickness from 6 m to 10 m. A continuous unit of residual sand, 2m thick, overlies bedrock which varies in depth from -32 m CDP in the west to -42 m CDP in the east.

Typical uncorrected SPT N values in the upper silty clay unit range between 0 and 19 with an average of 7. Typical uncorrected cone resistance values range between < 1 – 2 MPa with an average of 1 MPa. CPTM07 is the exception with an average cone resistance of 2 MPa.

Typical uncorrected SPT N values in the lower sand unit range between 6 and 53, with an average of 24; two instances of refusal were recorded.



5.1.5 Geology between berth chainage 40 m and 170 m (Zone 5)

The existing return wall defines the eastern boundary between zone 5 and zone 4. Land level is at approximately + 3.7 m CDP. This zone is characterised by an upper unit comprising medium dense to dense sand. This unit is approximately 14 m thick, terminating at approximately -10 m CDP. Underlying the sand is a clay unit similar in consistency to zone 4, comprising silty clay. Underlying the silty clay is a sand unit fine sand. This sand unit is 10 m thick, thinning to the east. A continuous unit of residual sand, 1 m thick, overlies bedrock which varies in depth from -34 m CDP in the east to -38 m CDP in the west.

Typical uncorrected SPT N values in the upper sand unit range between 1 and refusal with an average of 27. Typical uncorrected cone resistance values range between 4 – >12 MPa with an average of 10 MPa.

Typical uncorrected SPT N values in the middle silty clay unit range between 0 and 1 with an average of 1. Typical uncorrected cone resistance values are 1MPa.

Typical uncorrected SPT N values in the lower sand unit range between 11 and 70 with an average of 43.

5.1.6 Geology between berth chainage 0 m to 40 m and return wall chainage 0 to 72 m (Zone 6)

As in zone 5, zone 6 is characterised by an upper unit comprising medium dense to dense sand. This unit is approximately 16 m thick, terminating at approximately -12 m CDP. Underlying the sand is a silty clay unit which extends to bedrock at -37 m CDP. The silty clay has a maximum thickness of 25 m. The extent of the silty clay and absence of the lower sand unit are what distinguishes zone 6 from zone 5. Bedrock comprises very soft siltstone.

Typical uncorrected SPT N values in the upper sand unit range between 10 and 50 with an average of 29. Typical uncorrected cone resistance values range between 3 – >12 MPa with an average of 10 MPa.

Typical uncorrected SPT N values in the middle silty clay unit range between 0 and 1 with an average of 1. Typical uncorrected cone resistance values are 1MPa.

Typical uncorrected SPT N values in the lower clay unit range between 0 and 9 with an average of 1. Typical uncorrected cone resistance values range between 0.8 MPa and 1 MPa.

5.1.7 Geology between return wall chainage 72 m and 221 m (Zone 7)

Zone 7 is characterised by an upper unit comprising sand. This unit is approximately 14 m thick, terminating at approximately -10 m CDP. Underlying the sand is a clay unit similar in consistency to zone 6,. Underlying the silty clay is a sand unit. This sand unit is 10 m thick and thins towards zone 6. A continuous unit of residual sand, 1 m thick, overlies bedrock which varies in depth from -32 m CDP to -34 m CDP.

Typical uncorrected SPT N values in the upper sand unit range between 1 and 50 with an average of 26.

Typical uncorrected SPT N values in the middle silty clay unit range between 0 and 5 with an average of 2.

Typical uncorrected SPT N values in the lower sand unit range between 0 and 50 with an average of 22.

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Geological Long-Section A-A'

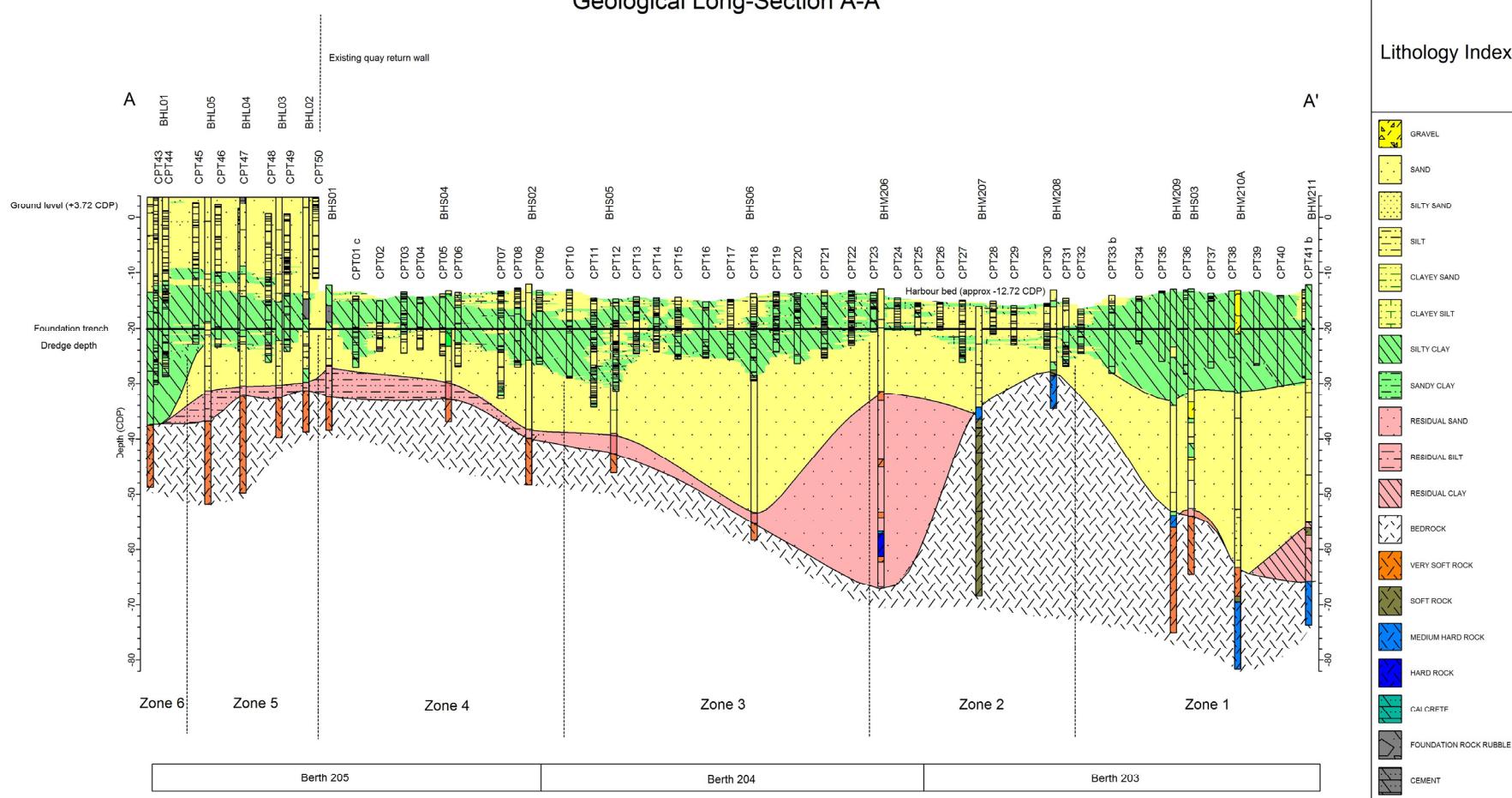


Figure 5.1-1: Quay Wall Geological Long Section



5.2 Dredge Area

The soils in the dredge area comprise predominantly fine sands with local deposits of silty clay, and a thin surficial layer of transported silty clay. Drawing 1370-CO-180-DWG-0003-02 shows the distribution and thickness of these units. Section B-B which runs east-west shows the sandy zone lensing out towards the turning circle as the profile becomes predominantly clay-rich with smaller volumes of sand.

The lateral and vertical distributions of sand and clay are shown on drawings 1370-CO-180-DWG-0003-02 and are derived from the results of widely spaced CPTu probes and exploratory boreholes. Due to the distance between sampling points and the local variability of the soil profile, variations from the distributions shown on the drawings can be expected.

From the shallow sampling conducted by the CSIR on the central sandbank as well as more recent exploratory boreholes at Berth 205 extension, the sandbank is characterised by an upper unit of fine to medium grained sands approximately 8 m thick. Underlying the sands is a silty clay unit approximately 10 m thick which is in turn underlain by sand to bedrock.

5.3 Borrow Site

The bathymetric survey conducted at the offshore borrow site shows two distinct features namely the northern and southern sand mounds. The northern mound encompasses an area of approximately 1.5 km² and is approximately 10 m high. The southern mound encompasses an area of 1.12 km² with an average height of 3 m. Apart from the distinct sand mounds present at site, the bathymetry is found to be gently undulating with submarine slopes steepening gently to the east. Rocky reef outcrops can be found scattered sparsely across the site. In general sea floor can be found at around -20 m CDP for the majority of the site with a submarine slope dropping down to approximately -31 m CDP towards the east.

The sand sediments comprise well graded medium slightly calcareous sands. The thickness of this unit ranges from 0 m in areas where rocky outcrop is present to 9.5 m. The average sediment thickness is 3.6 m.

5.4 Lot 10 Casting Yard

CPTu's probing at the Lot 10 casting yard reveals a uniform soil profile across site. The profile comprises an upper predominantly † sand unit approximately 10 m thick. The sand unit overlies a predominantly silty clay. CPTU coverage is limited with depth, but it appears the silty clays are underlain by dense sands at approximately -19 m CDP.

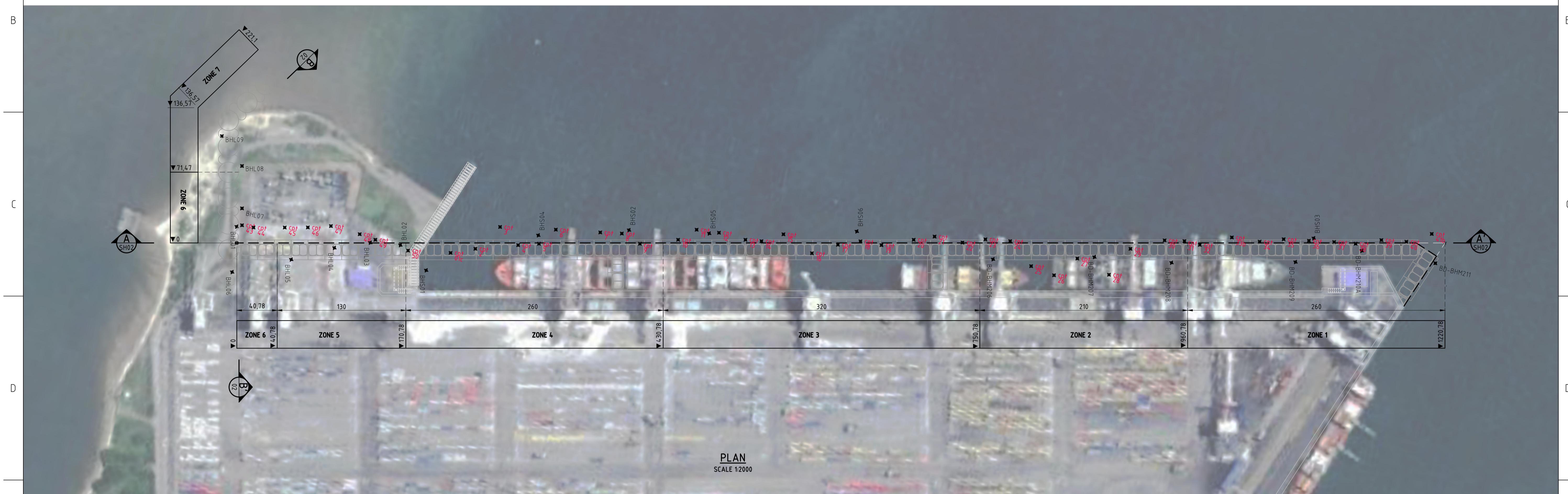
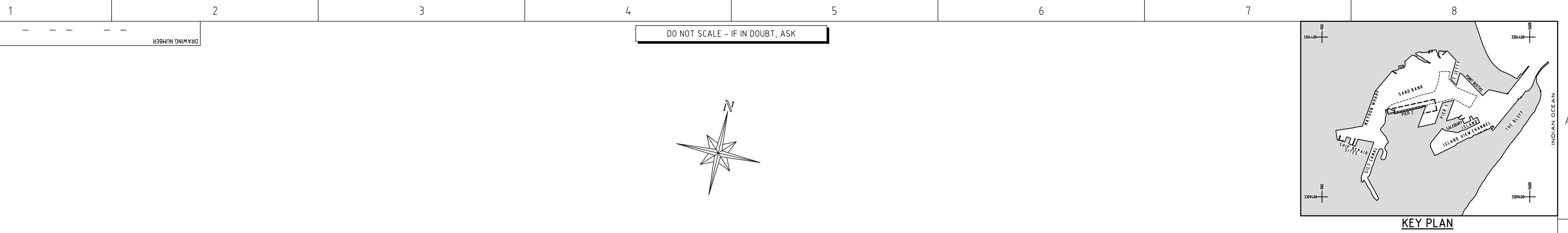
Typical uncorrected cone resistance values in the upper sand unit range between 8 – 12 MPa Typical uncorrected cone resistance values in the lower clay unit range between 2 – 4 MPa.



ANNEXURES



ANNEXURE 1: DRAWINGS



1 2 3 4 5 6 7 8

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1370-CO-180-C-DWG-0002-02 GEOLOGICAL LONG SECTION WITH SPT RESULTS
1370-CO-180-C-DWG-0002-01 GEOLOGICAL LONG SECTION - RETURN QUAY
1370-CO-180-C-DWG-0001-03 RELATIVE DENSITY AND CONSISTENCY SECTION
1370-CO-180-C-DWG-0001-02 GEOLOGICAL LONG SECTION - QUAY WALL

DRAWING NO. REFERENCE

REFERENCE DRAWINGS

PROJECT NO	DIS CODE	AREA	DISP	TYPE	DRG NO	sheet	status	REV
1370	C0	180	C	DWG	0001	01	P	0

EPCM CONSULTANT: TCP

ORIGINATOR: ZAA

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ENG. COORD.				CHECKED	AS		16/05/31
ENG. MANAGER				ENG. COORD.	MC		16/05/31
AREA MANAGER				DISCIP. ENG.			
PROJECT MGR.				ENG. MANAGER			
DIVISION				AREA MANAGER			

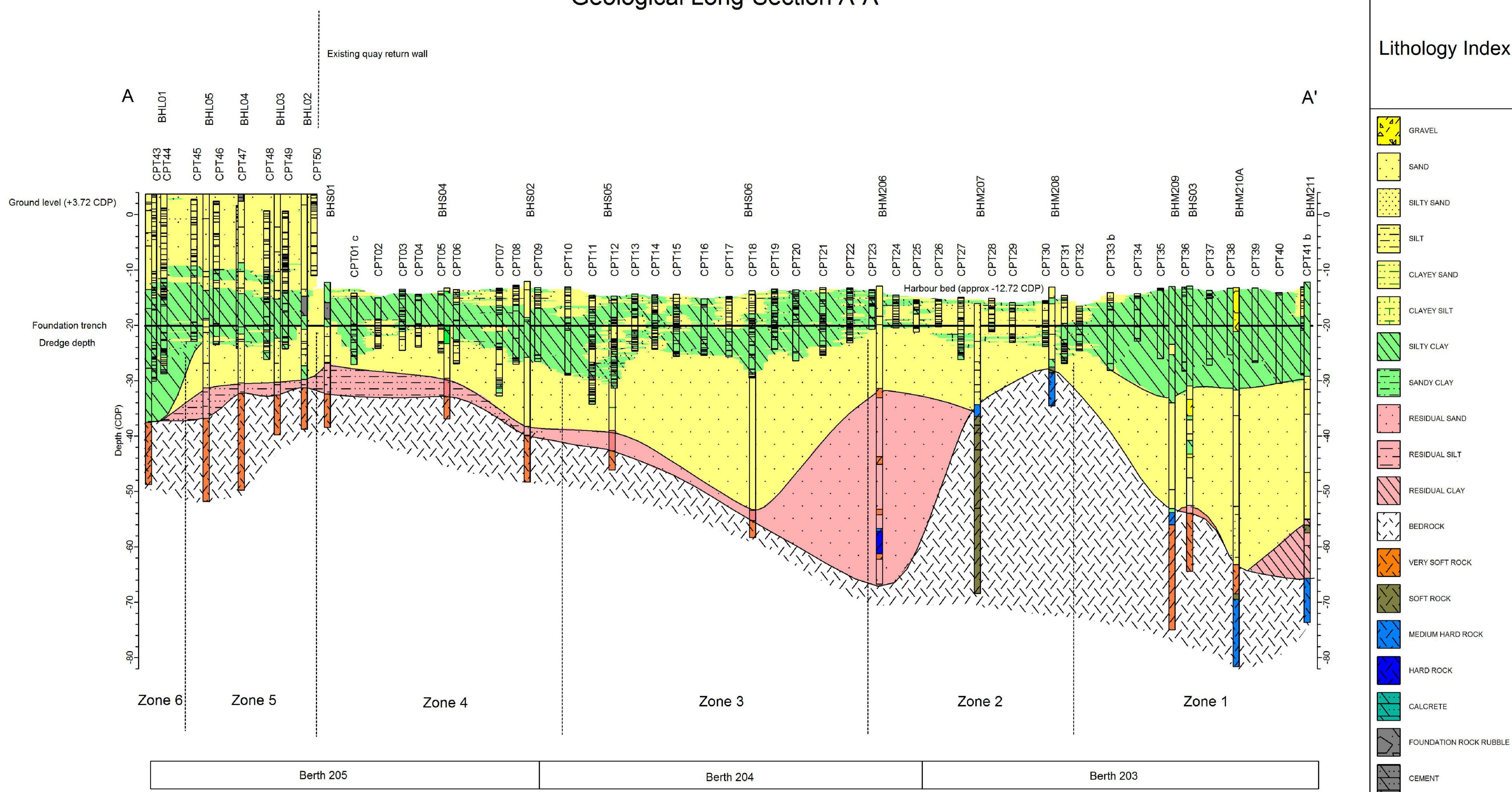
TRANSNER

DCT BERTH 203 to 205 RECONSTRUCTION
DEEPENING AND LENGTHENING
GEOTECHNICAL INVESTIGATION
CPTU AND BOREHOLE POSITIONS
QUAY WALL AND RETURN QUAY

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PROJECT NUMBER	DV	FBS	DIS	TYPE	DRG. NO.	SHT.	REV.	ID
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Geological Long-Section A-A'

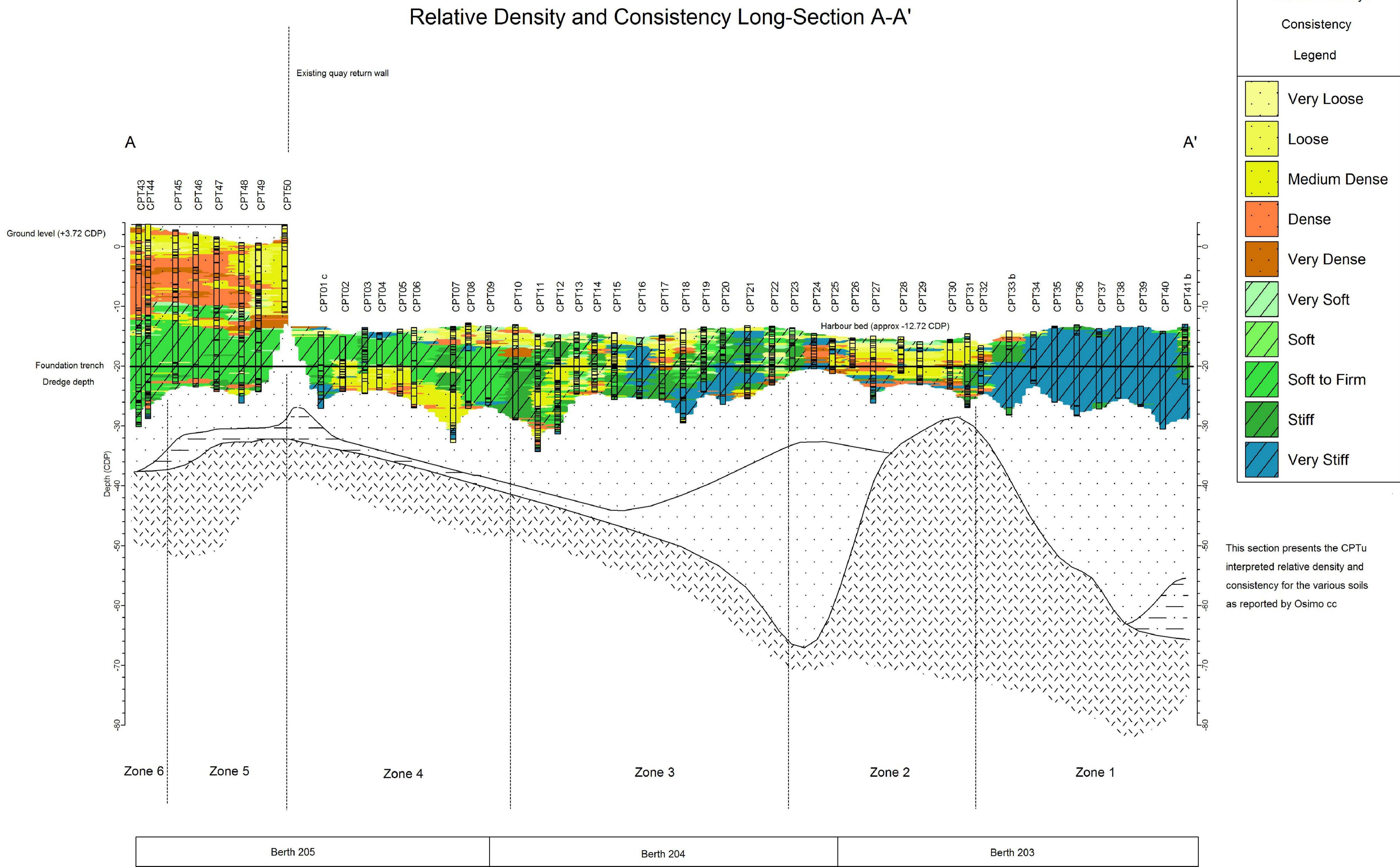


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						PROJECT MGR.						ENG. MANAGER					
						DIVISION						AREA MANAGER					
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TRANSNET

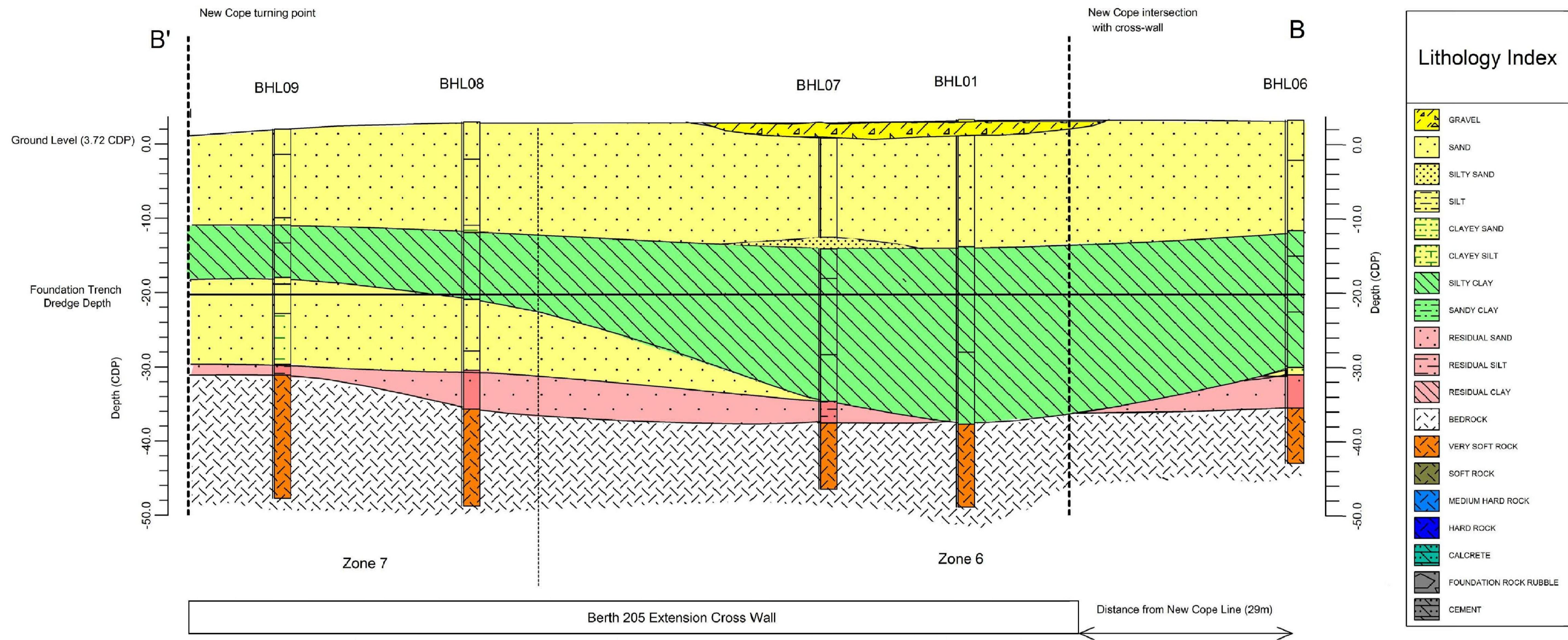
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DRAWING NO.	REFERENCE	PO Box 26546 HOUT BAY 7872 Tel: +2721 791 9100 EMAIL : zaaepna@zaepna.com	31 Melkhout Crescent HOUT BAY 7806 Fax: +2721 790 4470 www.zaaepna.com	TITLE NAME SIGNATURE DATE	TITLE NAME SIGNATURE DATE	DCT BERTH 203 to 205 RECONSTRUCTION	
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				PROJECT MGR.	ENG. MANAGER		
				DIVISION	AREA MANAGER		
				PR.ENG. / PR.TECH.			
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Generalised Geological Long-Section B-B'



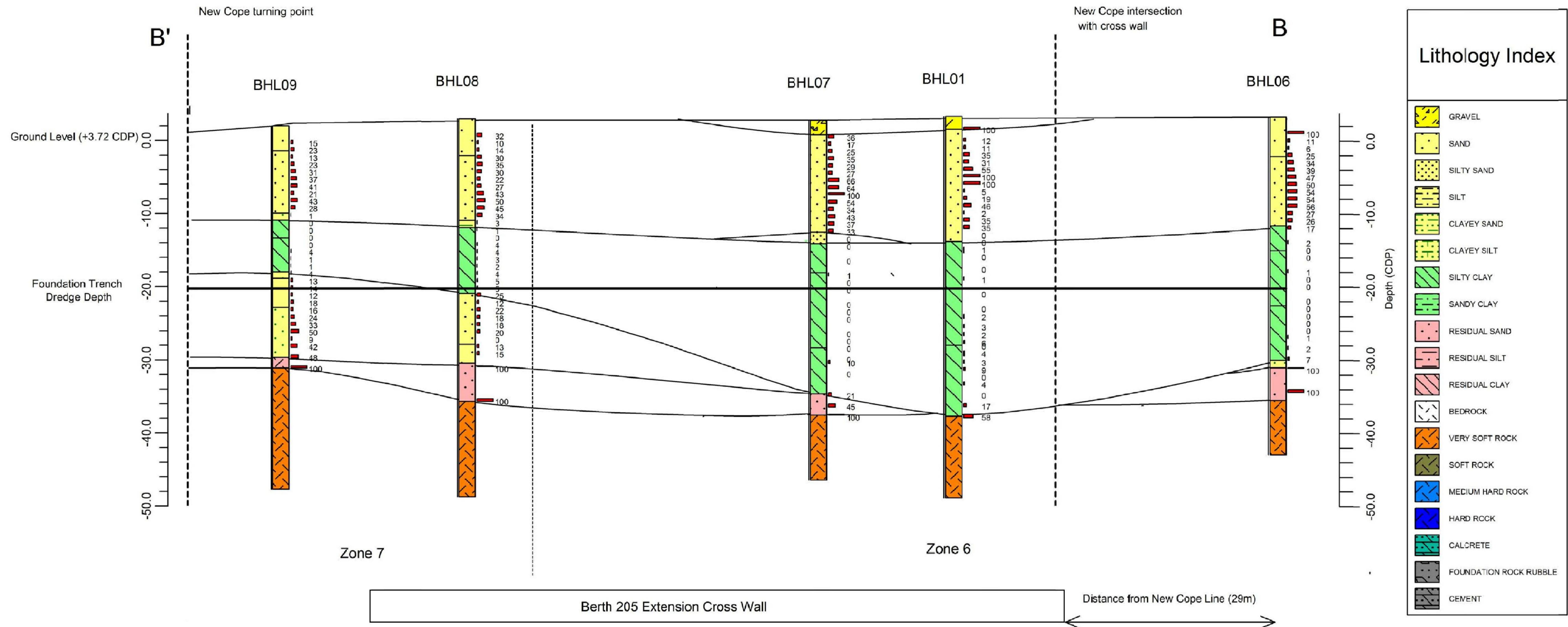
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Engineers	ZAA ENGINEERING PROJECTS & NAVAL ARCHITECTURE (Pty) Ltd	EPCM CONSULTANT: TCP	ORIGINATOR: ZAA	TRANSNET			
PO Box 26546 HOUT BAY 7872 Tel: +2721 791 9100 EMAIL : zaaepna@zaepna.com	31 Melkhou Crescent HOUT BAY 7806 Fax: +2721 790 4470 www.zaaepna.com	ISSUED FOR INFORMATION NO.	AS MC JZ 16.05.31	TITLE NAME SIGNATURE DATE LEAD DES. ENG. DRAWN AS 16.05.31 ENG. COORD. CHECKED MC 16.05.31 ENG. MANAGER ENG. COORD JZ 16.05.31 AREA MANAGER DISCIP. ENG. PROJECT MGR. ENG. MANAGER DIVISION AREA MANAGER	PR.ENG. / PR.TECH. NAME J. ZIETSMA 16.05.31 SIGNATURE	DCT BERTH 203 to 205 RECONSTRUCTION DEEPENING AND LENGTHENING	
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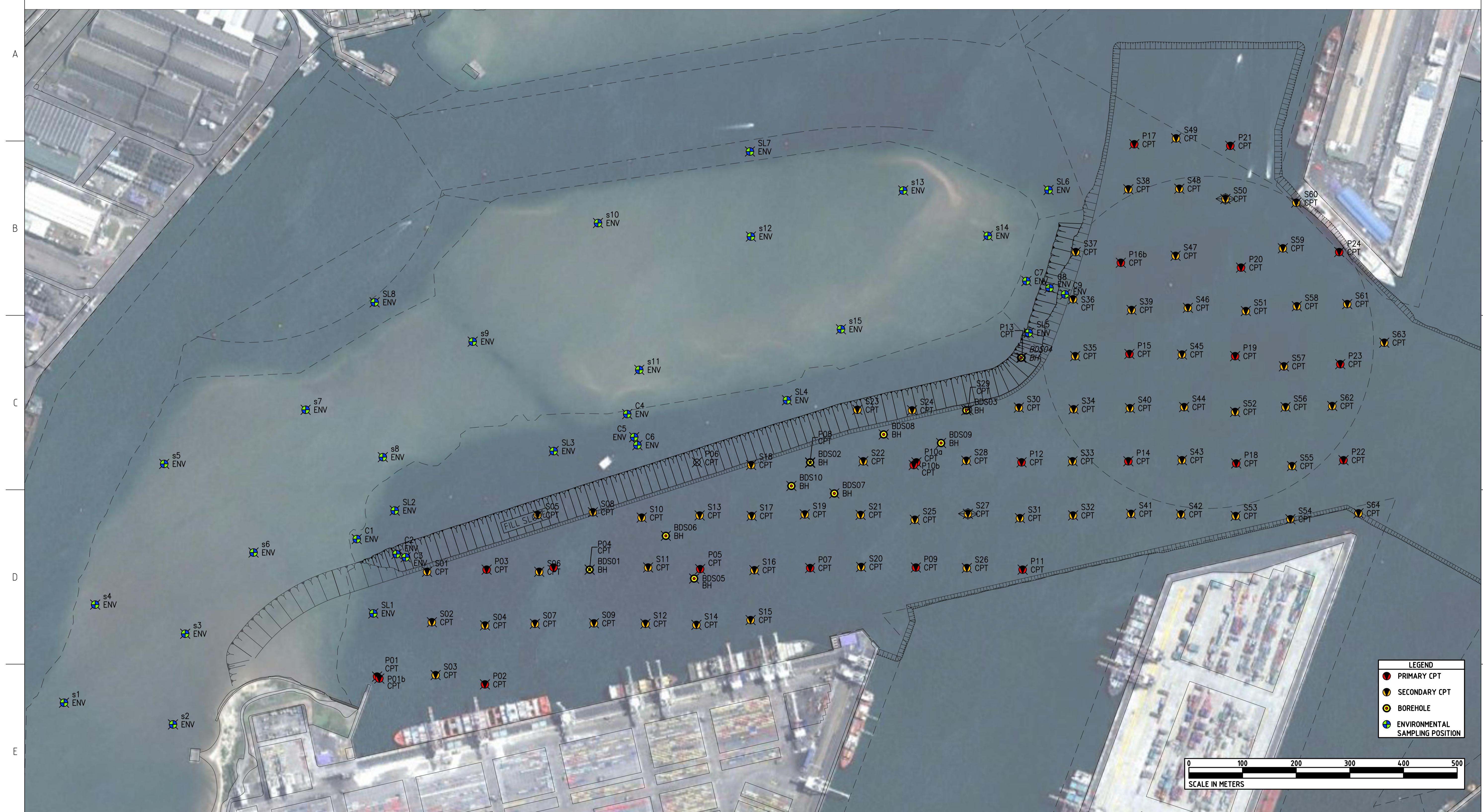
Generalised Geological Long-Section B-B' with SPT N Values



Note:
This section presents
the SPT results for the
various soils. SPT N
values are uncorrected

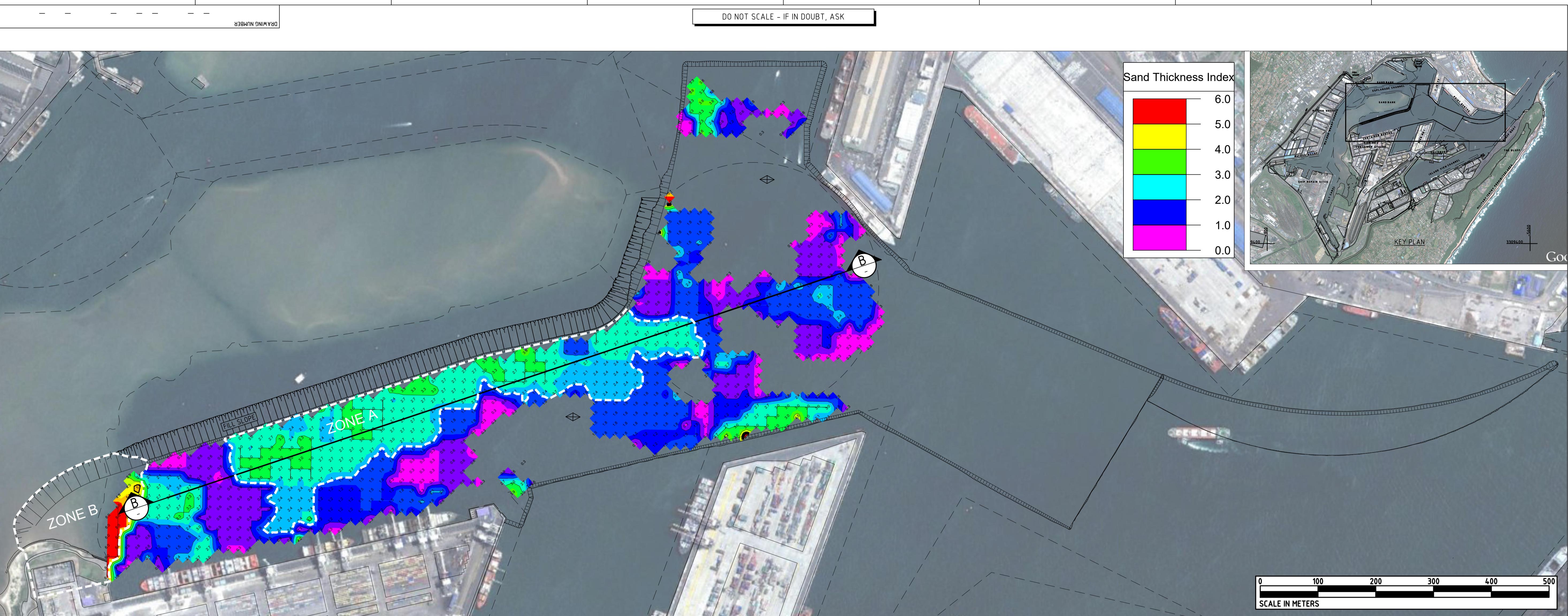
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PO Box 26546 HOUT BAY 7872 Tel: +2721 791 9100 EMAIL : zaaepna@zaaepna.com												POD ISSUED FOR INFORMATION NO.	DESCRIPTION	BY	CHK'D APP'D	DATE					PR.ENG. / PR.TECH.									
31 Melkhout Crescent HOUT BAY 7806 Fax: +2721 790 4470 www.zaaepna.com												REVISIONS / ISSUE AUTHORIZATION									SIGNATURE	16/05/31	DCT BERTH 203 to 205 RECONSTRUCTION DEEPENING AND LENGTHENING							
1370-CO-180-C-DWG-0001-01 CPTU AND BOREHOLE POSITIONS DRAWING NO. REFERENCE												PROJECT NO.	DIS CODE	AREA	DISP.	TYPE	DRG NO.	Sheet	Status	Rev	REG. NUMBER	760293	DIS	TYPE	DRG. NO.	SHT.	REV. ID			
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DO NOT SCALE - IF IN DOUBT, ASK



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J. ZIETSMAN		DATE			
		16 05 31			



Engineers	 PO Box 26546 HOOT BAY 7872 Tel: +2721 791 9100 EMAIL : zaaepna@zaaepna.com										EPCM CONSULTANT: TCP ORIGINATOR: ZAA TITLE NAME SIGNATURE DATE LEAD DES. ENG. ENG. COORD. ENG. MANAGER AREA MANAGER PROJECT MGR. DIVISION DISCIP. ENG. ENG. MANAGER AREA MANAGER										TRANSNET DCT BERTH 203 to 205 RECONSTRUCTION DEEPENING AND LENGTHENING GEOTECHNICAL INVESTIGATION GEOLOGICAL LONG SECTION BASIN			
31 Melkhout Crescent HOOT BAY 7806 Fax: +2721 790 4470 www.zaaepna.com	NO.	DESCRIPTION	AS	MC	JZ	16.05.31	BY	CHK'D	APP'D	DATE	PR.ENG. / PR.TECH.	NAME J. ZIETSMAN	DATE											
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1370-C0180-C-DWG-0003-01 CPTU AND BOREHOLE POSITIONS

DRAWING NO. REFERENCE

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ANNEXURE 2: EXPLORATORY BOREHOLE LOGS

- 2.1 EXPLANATORY NOTE ON BOREHOLE LOGS AND SAMPLING**
- 2.2 FAIRBROTHER GEOTECHNICAL METHOD STATEMENT**
- 2.3 BOREHOLE LOGS – QUAY WALL**
- 2.4 BOREHOLE LOGS – BASIN DREDGE AREA**



2.1: EXPLANATORY NOTE ON BOREHOLE LOGS AND SAMPLING



EXPLANATION OF BOREHOLE LOGS

This explanation describes the information provided in each of the twelve columns on the borehole logs. The explanations are applicable to ZAA's borehole logs.

An explanation of the borehole logs that form part of the FEL 2 study can be found in Annexure 5.1 of this Report. TCP Documentation No. H500272-1-221-H-RPT-0002-MJ.

The borehole naming convention is as follows:

- BD - BHM series boreholes – Marine boreholes drilled as part of Moore Spence and Johnes's FEL 2 investigation.
- BHL or BHS – Land and marine boreholes drilled as part of ZAA's FEL 3 geotechnical investigation.
- BDS – Marine dredge area boreholes drilled as part of ZAA's FEL 3 geotechnical investigation.

ELEV. (CDP)

This column indicates the elevation relative to the chart datum in the Port of Durban (CDP).

DEPTH OF THE RUN ELEV. (CDP)

The drill run depth recorded in the core boxes has been converted to chart datum in this column.

MATERIAL/CORE RECOVERY (%)

The material recovery is the measured material recovery per drill run expressed as a percentage. The core recovery columns represents the measured core recovery per drill run expressed as a percentage.

RQD (%)

The RQD (Rock Quality Designation) for rock core is presented in this column. The RQD is defined as the total cumulative lengths of all pieces of core greater than 100mm in length expressed as a percentage of the length of the drill run. Where only soil was recovered, no RQD is indicated.

FF (J/M)

The fracture frequency is the number of joints per meter per drill run. The fracture frequency is only measured in rock.

METHOD AND SAMPLE

The method of drilling and sample recovery is indicated in this column. The following abbreviated are used:

- WB - Wash boring
- SPT - Standard Penetration Test.
- Shelby – 60 mm (inner diameter) pressure tube sample
- NDW4 – 55 mm diameter, split double tube core barrel
- TNW.DT - TNW 60 mm diameter, double tube core barrel

SPT VALUE, 'N'

SPT testing was performed in accordance with ASTM D1586.

The field (uncorrected) SPT 'N'-values are recorded in this column. N=R indicates SPT refusal. Refusal was recorded if more than twenty one blows were measured per 75mm of penetration depth.



SPT 'N' BAR GRAPH

The 'N' value is shown as a bar graph.

LITHOLOGY

A combination of symbols and colours are used to differentiate thirteen different soil and rock types.

GENERAL DESCRIPTION

The soil and rock recovered is described according to standard South African practise using the methods described by Jennings, Brink and Williams (Soil Profiling for Civil Engineering in South Africa).

The depth of each soil and rock layer is shown relative to CDP.

REMARKS

Additional remarks, generally on small variations in soil type are given in these columns. 2.

DEPTH (MBGL- METERS BELOW GROUND LEVEL) OR DEPTH (MBPL – METERS BELOW PLATFORM LEVEL)

For land boreholes (BHL01 to BHL09), the depths in this column are given in meters below ground level (mbgl) at the drill positions.

For marine boreholes (BHS01 to BHS03 & BD-BHM206 to BD-BHM211 & BDS01 to BDS01 to BDS10), the depth is given in meters below platform level.

In both cases, the drill run depths in the core boxes are relative to these depths.

SAMPLE STORAGE AND PREPARATION

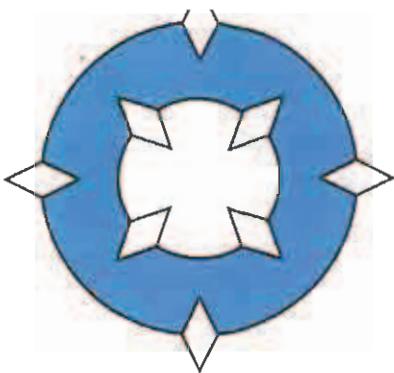
Cohesive samples contained in Shelby tubes were sealed with wax upon extraction from the borehole.

Core and wash bore samples were placed into airtight plastic bags. In both instances samples were labelled with a permanent marker indicating sample name and depth.

The samples are stored in labelled core boxes..



2.2: FAIRBROTHER GEOTECHNICAL METHOD STATEMENT



FAIRBROTHER

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MARINE BASED GEOTECHNICAL DRILLING AND CONE PENETROMETER TEST. PIER 2 DURBAN HARBOR

METHOD STATEMENT

REVISION: 001

This method statement describes the procedures and equipment used for marine based geotechnical drilling and cone penetrometer tests on a self-erecting work platform.

1. SCOPE OF WORK:

- Marine based drilling of geotechnical borehole and cone penetrometer tests.

2. AREA OF WORK:

- CPTu sampling will take place at 25 metre intervals along a line at 25metre in front (seaward) of Pier 2 in Durban Harbour.

3. EQUIPMENT TO BE USED:

- Gouritz-F SEWP (Self-Erecting Work platform) with power pack.
- CPTu cylinder
- HD300 (Rotary Drill Rig)
- Rotary coring tools (N-size)
- Standard Penetration Test Equipment
- OSIMO CPTu equipment
- Safety equipment and personal protective equipment

4. ESTABLISHMENT OF PLANT:

- Transporting equipment to site.
- Offloading of equipment with rear mounted crane.
- Construction of self-erecting work platform.
- Lifting of Gouritz-F with mobile crane into harbor.
- Move equipment to designated area.

5. PREPARATION OF EQUIPMENT:

- The Gouritz-F platform will be placed alongside the quay wall at a position convenient to the operations management.
- The Gouritz-F platform will be towed to sampling positions in a sequence determined by operations management so as to minimize any disruption to shipping.
- The mobile crane will be used to lower the four legs into place. Each leg will be mechanically locked until the hydraulic system is in place and commissioned.
- All equipment will be lifted and fitted using the mobile crane.
- One crew boat with an independent skipper will be employed.
- The Gouritz-F will be towed using two lines each at least 30m in length.
- The drilling supervisor will control the speed of the Gouritz-F and in contact with the skipper by means of a two-way radio.
- Once the Gouritz-F is in position, at the first approved sampling location, it will be lifted by using the hydraulically operated legs and that the platform is at least 3m above chart datum.
- Locking pins will be inserted into the appropriate holes in the legs.
- Once all four legs are in place and secure, CPTu sampling operations will commence.

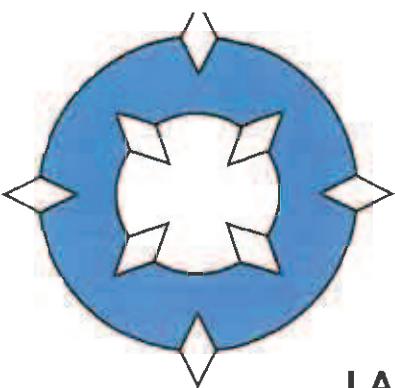
6. OPERATIONS METHOD:

- **CONE PENETROMETER TESTS (39 number)**
 - The boom will be vertically positioned.
 - CPTu Casings will be lowered into the water until it reached the ocean bed.
 - The CPTu probes will be lowered into the casing until it reached the ocean bed.
 - The CPTu casings will be connected to the hydraulic unit.
 - The probe (with the casings) will be hydraulically pressed to refusal depth.
 - Electronic readings will be taken continuously and presented to the consultant at completion of the test.
- **BOREHOLE DRILLING (1 number)**
 - The drill will be vertically positioned at the selected site (CPT profile determined by site Geologist).
 - The drill will be fitted with the appropriate drilling tools: (N-size)
 - SPT sampling will commence at 1metre depth intervals to the required depth at bedrock
 - Undisturbed Shelby samples will be taken at depth intervals specified by the site Geologist.
 - The drill tools will be extracted from the hole.
 - All samples will be marked, documented and packed in core boxes
- **STANDARD PENETRATION TESTING**
 - The Raymond spoon will be fitted to the drilling rods.
 - The SPT- hammer is self-tripping and drops a 63.5kg weight through 750mm.
 - Sampling will be conducted at 1.0 metre depth intervals through to bedrock depth.
 - On removal, the split tube will be opened and logged by the site Geologist. Thereafter, the sample will be plastic wrapped.
 - All samples will be marked, documented and packed in core boxes.

7. DE ESTABLISMENT OF PLANT:

- The legs of the Gouritz-F will be lowered.
- Tow the Gouritz-F with vessel to the quay.
- Remove legs from the leg guides with the mobile crane and place on the quay.
- Lifting the power pack, HD300 and equipment with the mobile crane.
- Dismantle the Gouritz-F.
- Loading of equipment with rear mounted crane
- Transporting equipment from site.

QA043



FAIRBROTHER

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LAND BASED GEOTECHNICAL DRILLING AT PIER 2 DURBAN HARBOR

METHOD STATEMENT

REVISION: 001

This method statement describes the procedures and equipment used for Geotechnical Rotary drilling.

1. SCOPE OF WORK:

- Drilling of two (2) geotechnical boreholes into required areas

2. AREA OF WORK:

- Drilling will take place Durban Harbor; (Pier 2 Container Terminal).

3. EQUIPMENT TO BE USED:

- HD300 (Rotary drill rig)
- Rotary coring tools (N-size)
- SPT sampler.
- Shelby sample tubes (63mm diameter).
- Safety equipment and personal protective equipment

4. ESTABLISHMENT OF PLANT:

- Transporting HD300 and equipment to site.
- Offloading of equipment with Rear mounted crane.
- Move equipment to designated area.

5. PREPARATION OF PLANT:

- All holes will be vertically drilled.
- All holes will be pre-set out.

6. OPERATIONS METHOD:

- The drill will be placed on the first drilling position with the boom vertical.
- The drill will be fitted with the appropriate drilling tools (P-size)
- SPT sampling will commence at 1.0m depth intervals.
- Wash boring will be conducted between SPT sample depths.
- Shelby undisturbed samples will be taken in cohesive horizons as per site geologist instructions.
- Upon hole completion at bedrock (2m penetration into), the drill tools will be extracted from the hole and the drill will be moved to the next drilling position.
- All samples will be marked, documented and packed in core boxes.

7. DE ESTABLISHING OF PLANT:

- Loading of equipment with Rear mounted crane
- Transporting equipment from site.

- **SPT Sample.**

- Using a Raymond Split-spoon connected onto the rod lowered into hole using rig winch.
- Connect drop weight onto drill rods and lift with rig winch.
- Ensure that the weight is locked with a d-shackle before lifting.
- Guide weight safely into position.
- Fasten weight on rods.
- Mark SPT measurements with chalk on the rods.
- The measurements must be 75mm intervals at a total penetration length of 450mm.
- Lifting weight using the winch.
- A 63.5kg monkey falling through 760mm.
- The drop weight shall be free-falling with one central guide.
- Recording number of blows per 75mm intervals up to 450mm or refusal.
- Remove Weight.
- Extraction of Raymond-spoon by using the winch.
- Strip Raymond-spoon to recover SPT sample.
- SPT sample will be place into clear plastic, sealed off and place into sample boxes and the depth will be clearly marked on the core blocks.

- The above mentioned method will be continued until competent soils are encountered.

- **Using of core barrels (TNW, NWD4) in competent soils.**

- When rock is encountered a core barrel will be used to drill and extract samples.
- Connect rods to core barrel and lift using the rig winch.
- Lower drill rods with diamond core bit and core barrel to bottom of hole.
- Use stilsens the lock the rod to prevent the rod from falling into the drilled hole during the next connection.
- When reaching bottom of hole connect to the rod inside rotation unit.
- Remove all tools from Rods and rotation.
- Start-up mono pump and check water circulation.
- Start-up drilling process.
- Drill 1.5m and detached rods from rotation unit.
- Extract the rods using the rig winch from the hole and use stilsens to loosen the rods.
- Place rods at safe area.
- Strip core barrel using stilsens.
- Remove sample from core barrel and ensure no contamination of sample.
- Place sample in clear plastic, sealed off and place into sample box and the depth will be clearly marked on the core blocks.
- Continue above process until required depth, instructed by the client is reached.

App.	Date	Rev.	Revision Details	Safe Work Instruction	Ref. Number:	SWI 007
	10/11	0	New Format		Compiled By:	J Theron
					Approved By:	A Meerburg
					Date:	28/10/11
					Page No:	2 of 3

QA 051 Safe Work Instruction

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SAFE WORK INSTRUCTION

Subject: Diamond Core Drilling.
Number: SWI007

				Ref/Resp
1.0	What is it?			
	<ul style="list-style-type: none"> The instruction to diamond core drilling. 			
2.0	Why is it performed?			
	<ul style="list-style-type: none"> To ensure the safety of operators and assistants during diamond core drilling. 			
3.0	When is it performed?			
	<ul style="list-style-type: none"> When samples of soil material are required by clients at designated areas. 			
4.0	How is it performed?			
	<ul style="list-style-type: none"> Loading and offloading of drill rigs and equipment. <ul style="list-style-type: none"> Safe work instruction to be followed and adhere to. Setup of drill rigs and equipment. <ul style="list-style-type: none"> Attached derrick (Boom) by using crane truck. Stabilize rigs by using cables, pens and turn buckles. Inspection on drill rig and grease all required areas. Ensure that the rig is fitted onto a drip pan to prevent any soil pollution. Dig sump for water circulation and use plastic sheet or use the specified circulation tank. Attached mono pump onto drill rig. Attached hoses to mono pump and water swivel at the rotation unit. Fill sump or circulation tank with clean water and additional 1000L tank. Start-up rig. Fit starter NX Casing. Check water circulation and add drill mix. Drilling of Borehole for sampling. <ul style="list-style-type: none"> Fit 1m interval NX casing to drill rig rotation. Start drilling into required marked position. Start-up water circulation at Mono pump. Ensure that the water and drill mix always circulate from the hole into the sump or circulation tank to prevent the casing and rods from getting stuck or damaged inside hole. When soil is soft a wash sample will be taken and a SPT will be carried out. Loosen casing-head with stilsen and remove from casing. 		Truck Driver (Competent) Operator	

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					Approved By:	A Meerburg
					Date:	28/10/11
					Page No:	1 of 3

QA 051 Safe Work Instruction

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- Core Orientation (IF REQUIRED)**
 - Insert plasticized clay into a core orientator.
 - Lowered into bottom of borehole using the rig winch with caution.
 - Ensure that the orientator reached bottom of hole.
 - Extract Orientator from the bore hole with rig winch cable.
 - Detached orientator from cable.
 - Place orientator with plasticized clay in cool area to prevent the clay from deforming.
- Completion of hole**
 - After required depth of client is reached NX casing must be removed using the rig winch.
 - Loosen casing using stilsens.
 - Place all casing at safe flat area.
 - Clean sump and remove plastic liner where applicable.
 - Clear work area and ensure all equipment and rubbish is cleared.
- Loading of equipment.**
 - Loading of all equipment onto truck and moved to next specified area.

Safety requirements:

Hazard	Risk	Control
Slipping conditions when working	Body Injury	Clean and stable operating area.
Falling equipment	Head Injury	Hard hats to be worn
Moving parts	Body injury	Clear from rotation and moving parts
Slipping stilsens	Hand and body injury	Checklist of equipment and using gloves
Moving casings and sleeves	Falling onto workers causing body injury	Competent Operators and equipment inspections
Lifting heavy equipment	Back and body injury when equipment fall	Using of winch and straps to guide rods and casings



App.	Date	Rev.	Revision Details	Safe Work Instruction	Ref. Number:	SWI 007
	10/11	0	New Format	 FAIRBROTHER	Compiled By:	J Theron
					Approved By:	A Meerburg
					Date:	28/10/11
					Page No:	3 of 3

QA 051 Safe Work Instruction

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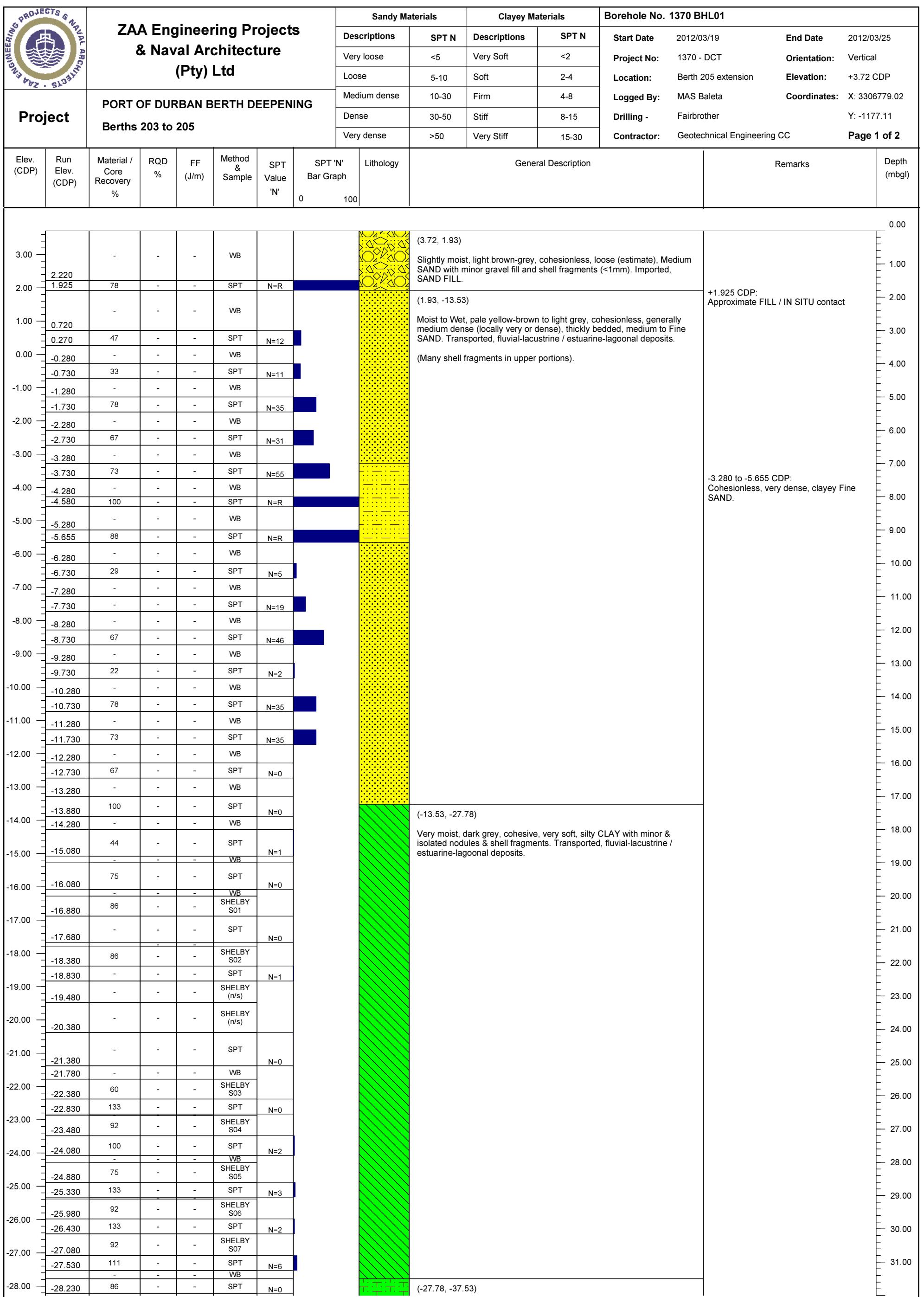
2.3: BOREHOLE LOGS – QUAY WALL



EXPLORATORY BOREHOLE LOGS – QUAY WALL

BH	Location	End	Start depth	End depth	y	x
-	-	-	CDP	CDP	-	-
BHL01	Land	2012/03/25	3.720	-48.780	-1177.11	3306779.02
BHL02	Land	2012/07/18	3.671	-38.780	-1331.30	3306765.10
BHL03	Land	2012/07/12	3.482	-39.780	-1303.90	3306770.90
BHL04	Land	2012/06/25	3.417	-49.780	-1265.50	3306776.90
BHL05	Land	2012/06/18	3.457	-51.780	-1233.80	3306797.40
BHL06	Land	2012/07/07	3.630	-42.780	-1172.00	3306824.20
BHL07	Land	2012/07/01	3.180	-45.780	-1168.00	3306760.40
BHL08	Land	2012/08/13	3.380	-48.280	-1151.00	3306715.00
BHL09	Land	2012/08/18	2.410	-46.280	-1132.00	3306693.00
BHS01	Marine	2012/07/22	2.595	-38.405	-1357.87	3306773.03
BHS02	Marine	2012/07/27	2.689	-48.311	-1554.20	3306694.60
BHS03	Marine	2012/07/15	3.060	-64.440	-2230.81	3306549.66
BHS04	Marine	2012/10/19	2.666	-36.934	-1466.00	3306720.00
BHS05	Marine	2012/10/16	2.500	-46.100	-1634.00	3306680.00
BHS06	Marine	2012/10/14	2.700	-58.300	-1778.59	3306654.11
BD-BHM-206	Marine	2008/08/12	*-12.900	-65.810	-1919.43	3306642.01
BD-BHM-207	Marine	2008/07/03	*-16.010	-67.400	-2018.85	3306617.19
BD-BHM-208	Marine	2008/07/02	*-13.118	-33.668	-2096.11	3306605.56
BD-BHM-209	Marine	2008/07/04	*-13.010	-74.110	-2218.11	3306577.19
BD-BHM-210A	Marine	2008/09/25	*-13.205	-80.705	-2280.97	3306551.30
BD-BHM-211	Marine	2008/07/21	*-12.187	-73.727	-2356.11	3306547.08

Note: * Indicates sea floor depth in CDP



		ZAA Engineering Projects & Naval Architecture (Pty) Ltd						Sandy Materials		Clayey Materials		Borehole No. 1370 BHL01						
								Descriptions		SPT N	Descriptions		SPT N	Start Date	2012/03/19	End Date	2012/03/25	
Project PORT OF DURBAN BERTH DEEPENING Berths 203 to 205								Very loose		<5	Very Soft		<2	Project No:	1370 - DCT	Orientation:	Vertical	
								Loose		5-10	Soft		2-4	Location:	Berth 205 extension	Elevation:	+3.72 CDP	
								Medium dense		10-30	Firm		4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306779.02	
								Dense		30-50	Stiff		8-15	Drilling -	Fairbrother	Y:	-1177.11	
								Very dense		>50	Very Stiff		15-30	Contractor:	Geotechnical Engineering CC	Page 2 of 2		
Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	0	100	Lithology	General Description			Remarks	Depth (mbgl)			
-28.780	100	-	-	TNW											32.00			
-29.230	71	-	-	SPT	N=4										33.00			
-29.780	80	-	-	TNW											34.00			
-30.230	100	-	-	SPT	N=3										35.00			
-30.780	-	-	-	WB											36.00			
-31.230	-	-	-	SPT	N=9										37.00			
-31.780	-	-	-	WB											38.00			
-32.230	58	-	-	SPT	N=0										39.00			
-32.780	-	-	-	WB											40.00			
-33.230	111	-	-	SPT	N=4										41.00			
-34.280	95	-	-	TNW											42.00			
-34.730	122	-	-	SPT	N=0										43.00			
-35.280	-	-	-	WB											44.00			
-36.230	100	-	-	SPT	N=17										45.00			
-37.280	-	-	-	WB											46.00			
-37.730	89	-	-	SPT	N=58										47.00			
-38.280	-	-	-	WB											48.00			
-39.780	99 / 83	77	5	NWD4											49.00			
-41.280	79 / 79	63	5	TNW.DT											50.00			
-42.780	62 / 60	47	5	NWD4											51.00			
-44.280	51 / 46	37	5	TNW.DT											52.00			
-45.780	65 / 43	31	4	NWD4														
-47.280	93 / 89	77	3	TNW.DT														
-48.780	85 / 85	85	3	NWD4														



1370-BHL01: Box 1 of 6



1370-BHL01: Box 2 of 6



1370-BHL01: Box 3 of 6



1370-BHL01: Box 4 of 6



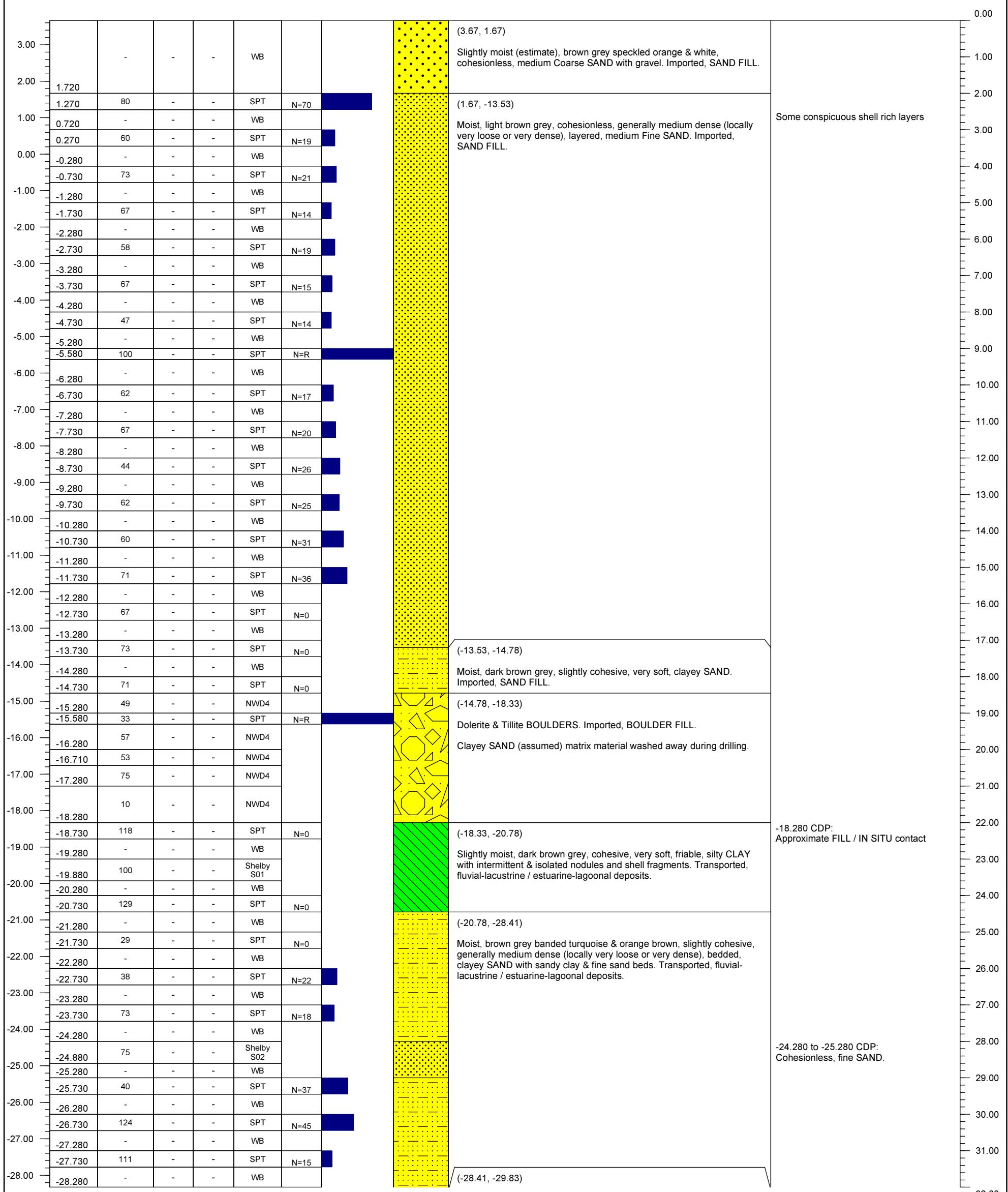
1370-BHL01: Box 5 of 6



1370-BHL01: Box 6 of 6

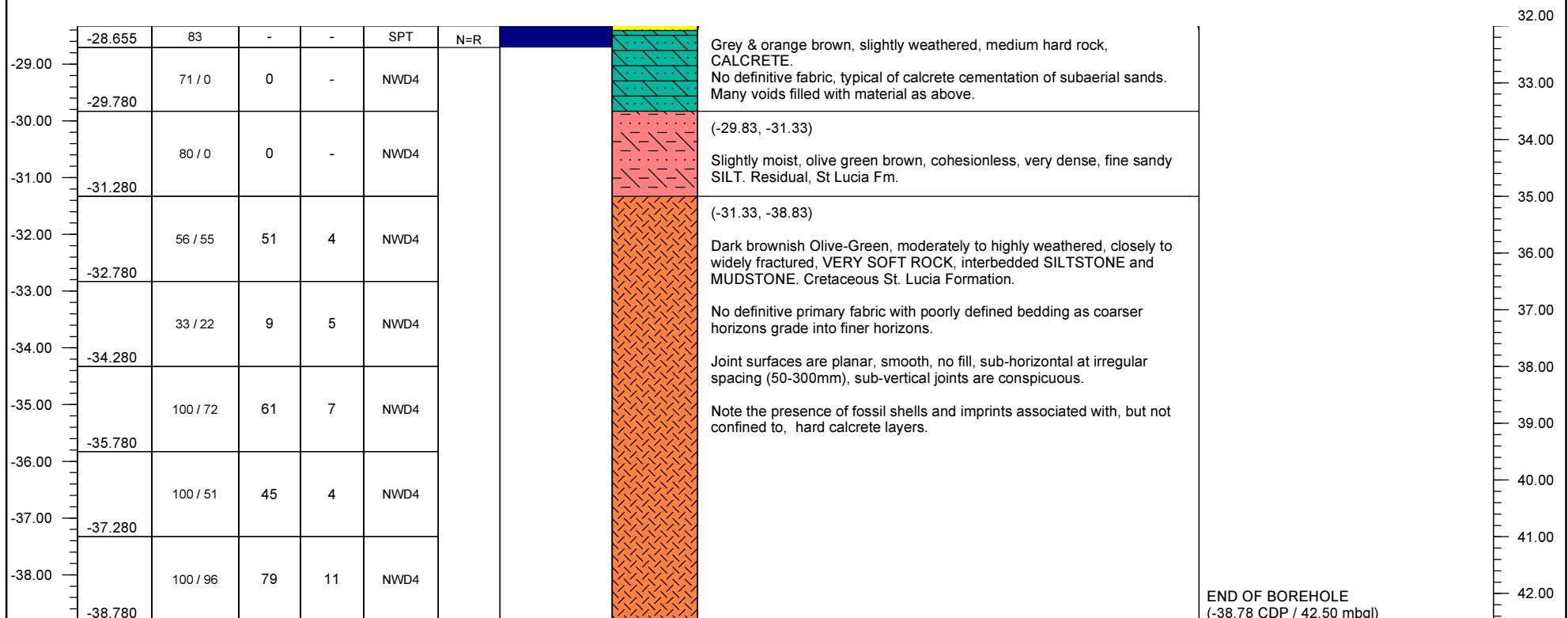
		ZAA Engineering Projects & Naval Architecture (Pty) Ltd							Sandy Materials		Clayey Materials		Borehole No. 1370 BHL02					
									Descriptions		SPT N	Descriptions		SPT N	Start Date	2012/07/14	End Date	2012/07/18
Project		PORT OF DURBAN BERTH DEEPENING Berths 203 to 205							Very loose		<5	Very Soft		<2	Project No:	1370 - DCT	Orientation:	Vertical
									Loose		5-10	Soft		2-4	Location:	Berth 205 extension	Elevation:	+3.671 CDP
									Medium dense		10-30	Firm		4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306765.10
									Dense		30-50	Stiff		8-15	Drilling -	Fairbrother	Y:	-1331.30
									Very dense		>50	Very Stiff		15-30	Contractor:	Geotechnical Engineering CC	Page 1 of 2	

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description				Remarks	Depth (mbgl)
0	100	0	100											0.00

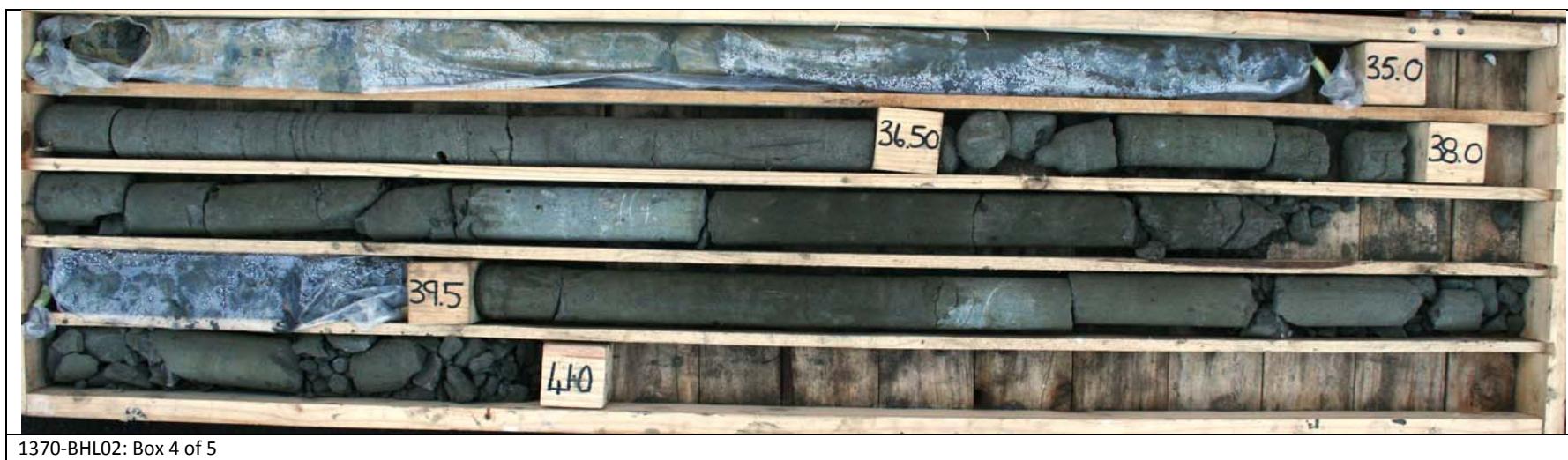


 ZAA Engineering Projects & Naval Architecture (Pty) Ltd	ZAA Engineering Projects & Naval Architecture (Pty) Ltd				Sandy Materials		Clayey Materials		Borehole No. 1370 BHL02			
	Descriptions	SPT N	Descriptions	SPT N	Start Date	2012/07/14	End Date	2012/07/18				
	Very loose	<5	Very Soft	<2	Project No:	1370 - DCT		Orientation: Vertical				
	Loose	5-10	Soft	2-4		Location: Berth 205 extension		Elevation: +3.671 CDP				
	Medium dense	10-30	Firm	4-8		Logged By: MAS Baleta		Coordinates: X: 3306765.10				
	Dense	30-50	Stiff	8-15	Drilling -	Fairbrother		Y: -1331.30				
	Very dense	>50	Very Stiff	15-30		Contractor: Geotechnical Engineering CC		Page 2 of 2				

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description			Remarks	Depth (mbgl)
							0	100					

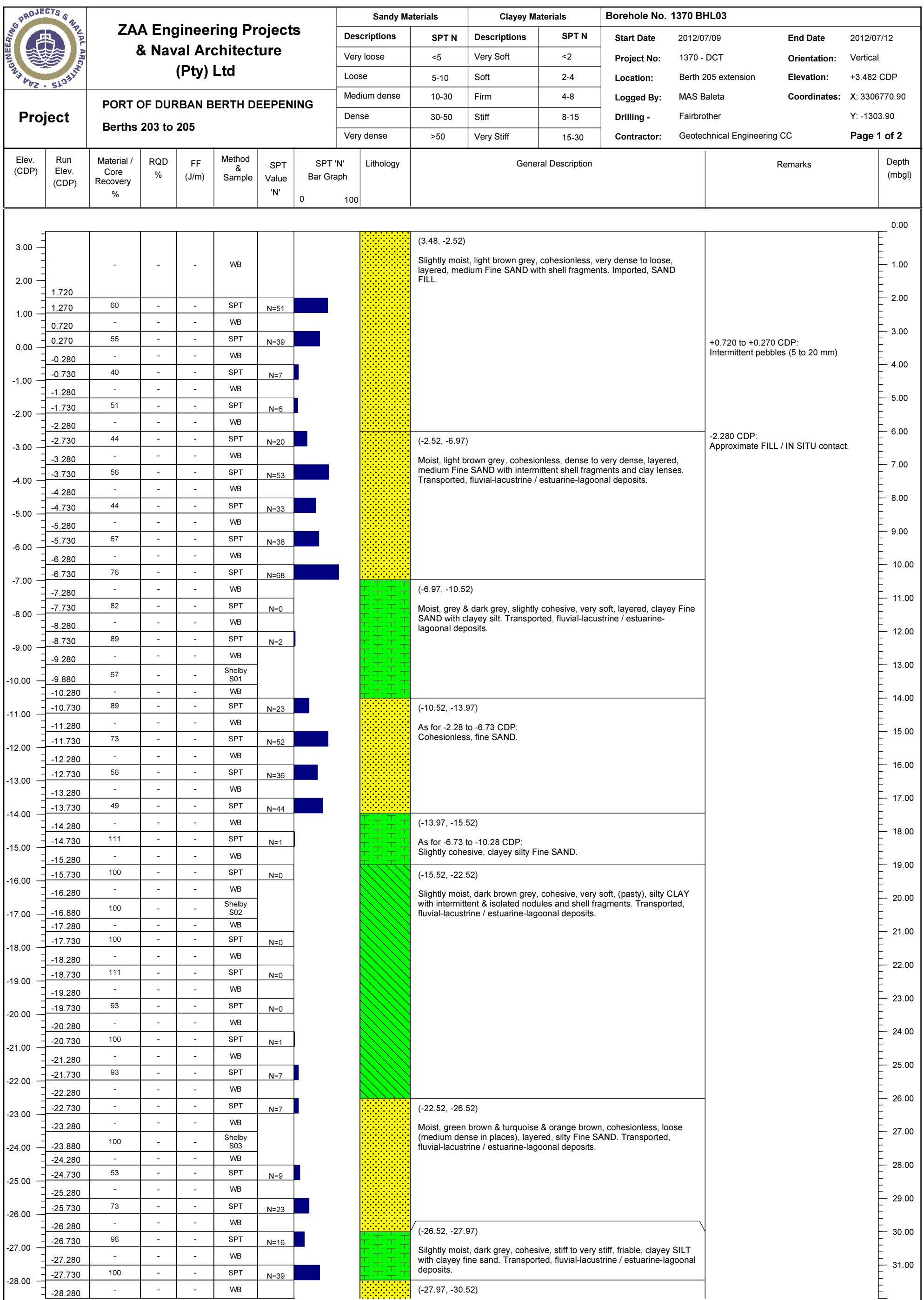








1370-BHL02: Box 5 of 5



		ZAA Engineering Projects & Naval Architecture (Pty) Ltd PORT OF DURBAN BERTH DEEPENING Berths 203 to 205						Sandy Materials		Clayey Materials		Borehole No. 1370 BHL03																																																																																																																																																																																								
								Descriptions		SPT N	Descriptions		Start Date	2012/07/09	End Date	2012/07/12																																																																																																																																																																																				
Project								Very loose		<5	Very Soft		Location:	Berth 205 extension	Orientation:	Vertical																																																																																																																																																																																				
								Loose		5-10	Soft		Logged By:	MAS Baleta	Coordinates:	+3.482 CDP X: 3306770.90																																																																																																																																																																																				
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1370-BHL03: Box 1 of 4



1370-BHL03: Box 2 of 4

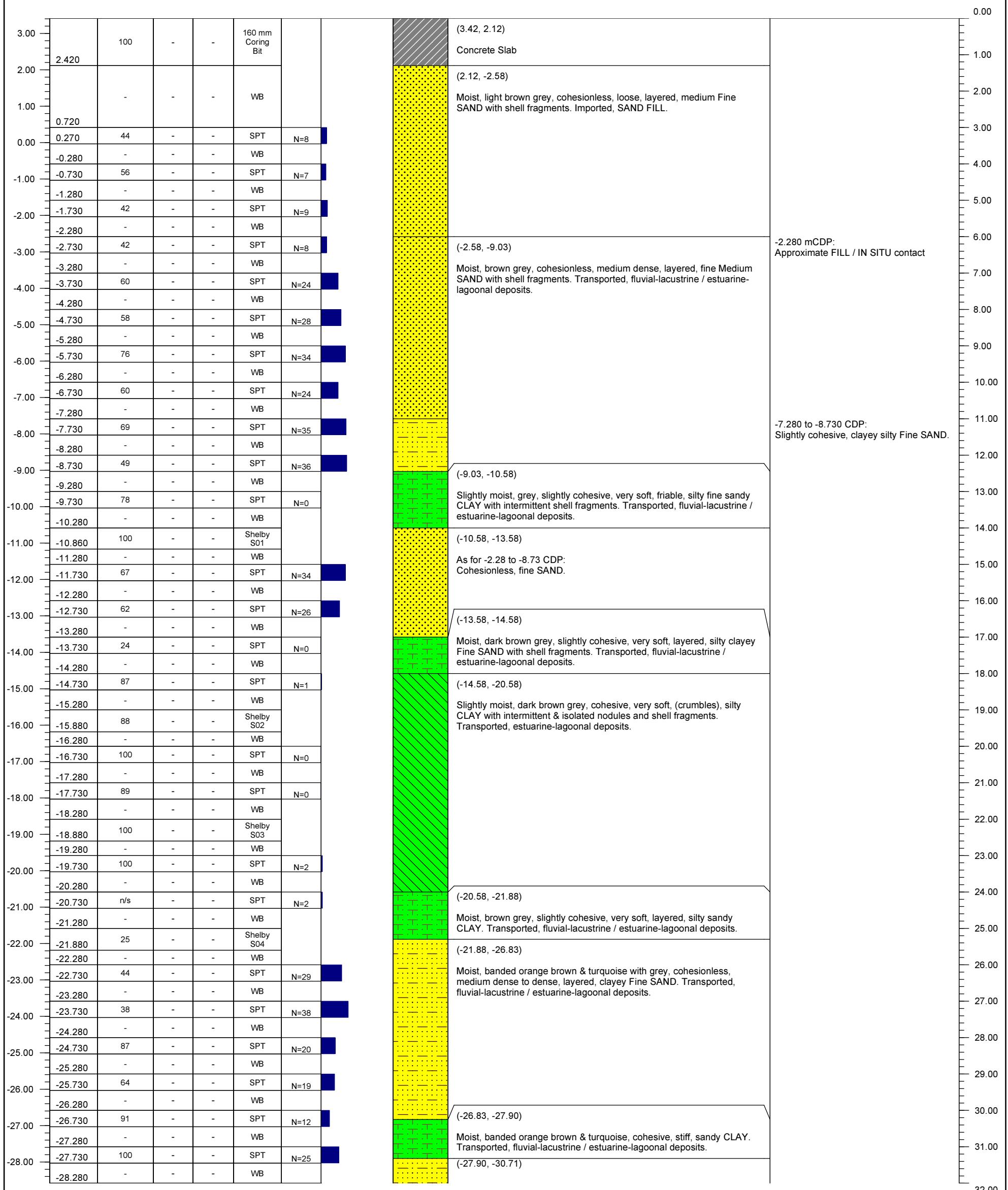


1370-BHL03: Box 3 of 4



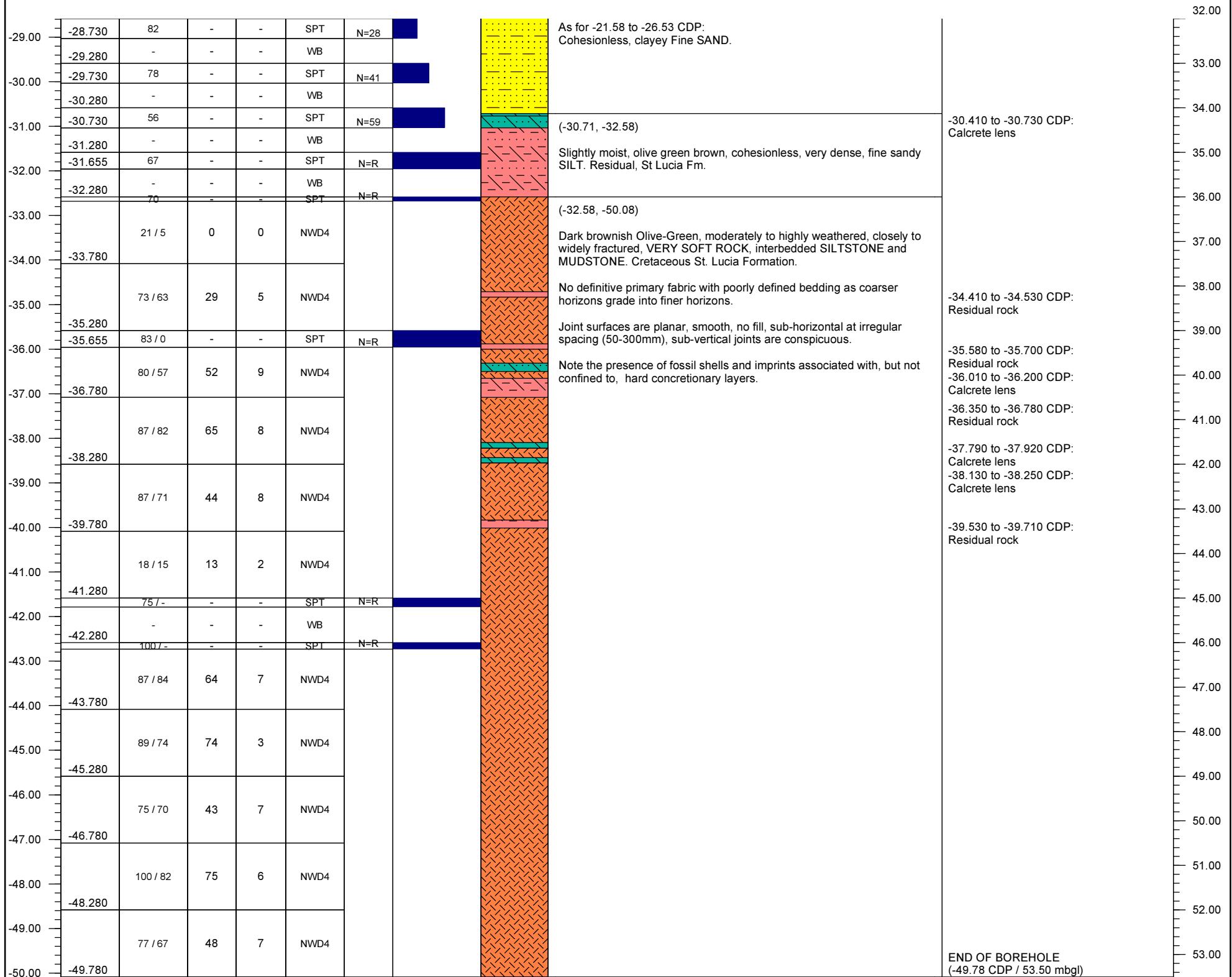
1370-BHL03: Box 4 of 4

 ZAA Engineering Projects & Naval Architecture (Pty) Ltd								Sandy Materials		Clayey Materials		Borehole No. 1370 BHL04			
Project PORT OF DURBAN BERTH DEEPENING Berths 203 to 205								Descriptions	SPT N	Descriptions	SPT N	Start Date	2012/06/19	End Date	2012/06/25
								Very loose	<5	Very Soft	<2	Project No:	1370 - DCT	Orientation:	Vertical
								Loose	5-10	Soft	2-4	Location:	Berth 205 extension	Elevation:	+3.417 CDP
								Medium dense	10-30	Firm	4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306776.90
								Dense	30-50	Stiff	8-15	Drilling -	Fairbrother	Y:	-1265.50
							Very dense	>50	Very Stiff	15-30	Contractor:	Geotechnical Engineering CC	Page 1 of 2		
Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	0	100	Lithology	General Description			Remarks	Depth (mbgl)



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Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description	Remarks	Depth (mbgl)
0	100										

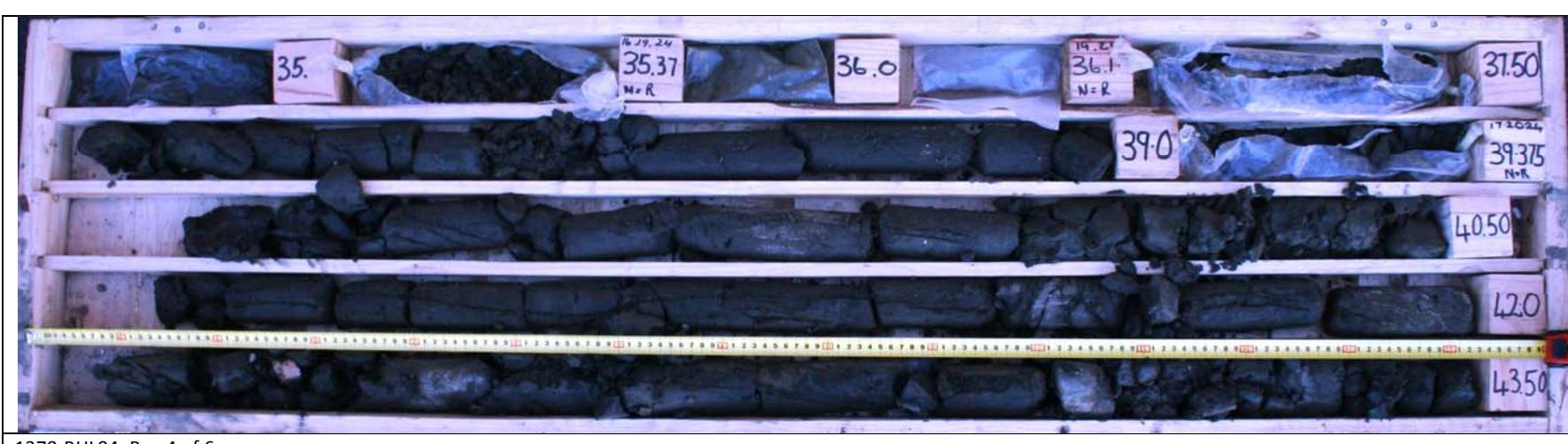


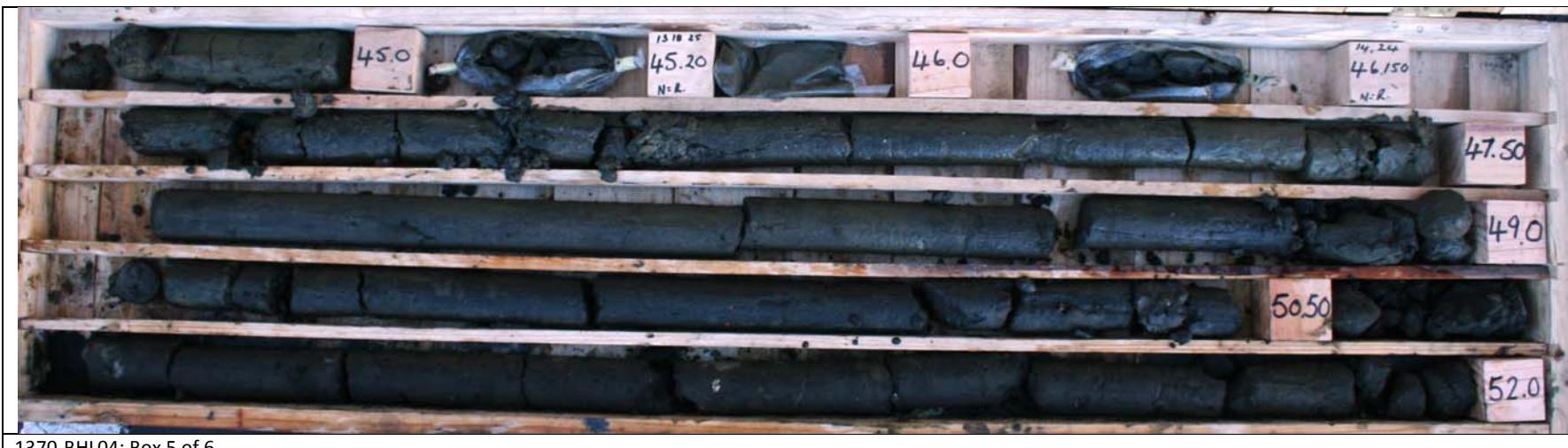


1370-BHL04: Box 1 of 6



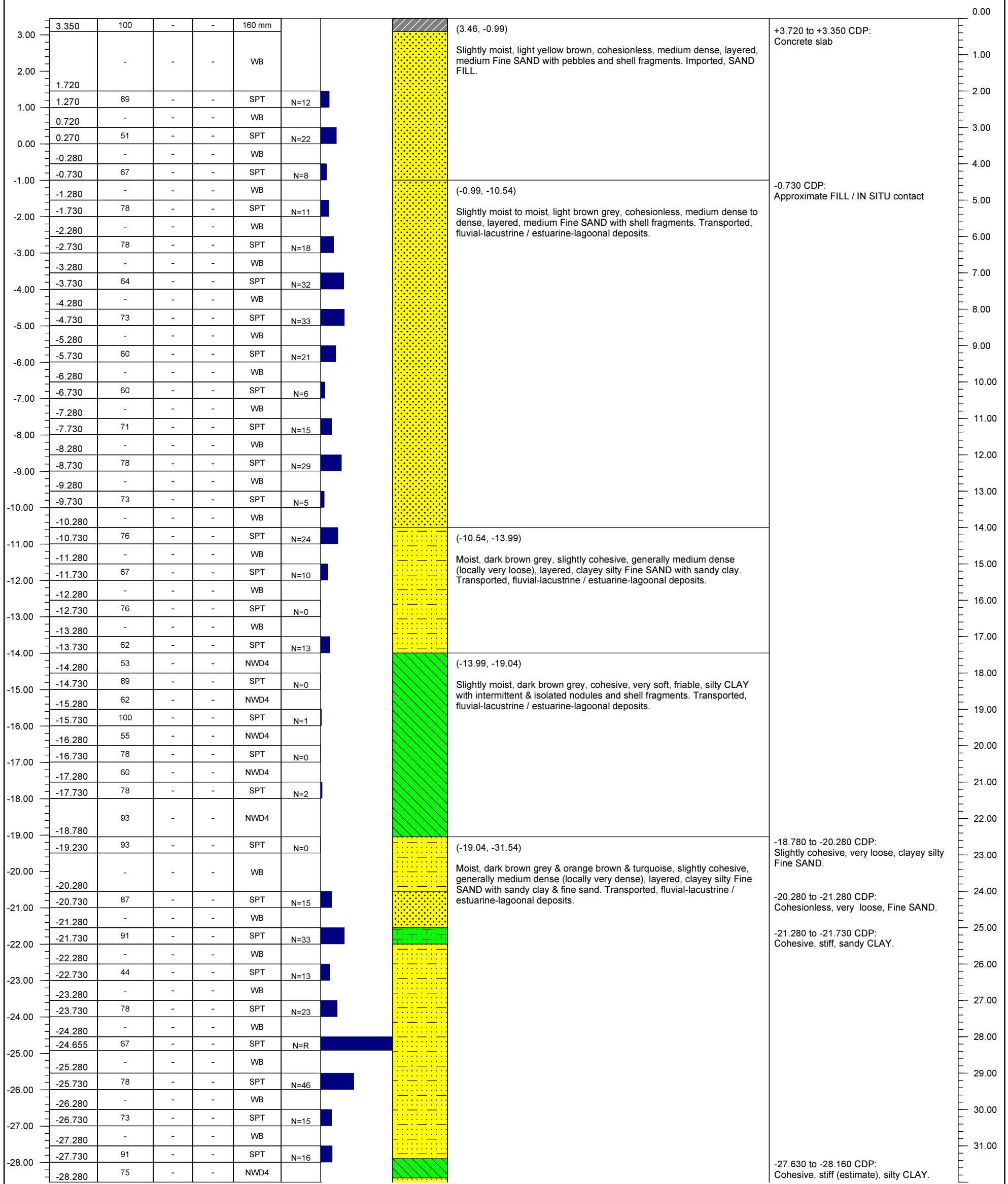
1370-BHL04: Box 2 of 6





 ZAA Engineering Projects & Naval Architecture (Pty) Ltd							Sandy Materials		Clayey Materials		Borehole No. 1370 BHL05			
Project		PORT OF DURBAN BERTH DEEPENING Berths 203 to 205				Descriptions		SPT N	Descriptions	SPT N	Start Date	2012/06/09	End Date	2012/06/18
						Very loose	<5		Very Soft	<2	Project No:	1370 - DCT	Orientation:	Vertical
						Loose	5-10		Soft	2-4	Location:	Berth 205 extension	Elevation:	+3.457 CDP
						Medium dense	10-30		Firm	4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306797.40
						Dense	30-50		Stiff	8-15	Drilling -	Fairbrother	Y:	-1233.80
						Very dense	>50		Very Stiff	15-30	Contractor:	Geotechnical Engineering CC	Page 1 of 2	

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description			Remarks	Depth (mbgl)
0	100	-	-	160 mm	WB	N=12	0	100					0.00



 ZAA Engineering Projects & Naval Architecture (Pty) Ltd		Sandy Materials				Clayey Materials		Borehole No. 1370 BHL05			
		Descriptions		SPT N	Descriptions		SPT N	Start Date	2012/06/09	End Date	2012/06/18
		Very loose	<5		Very Soft	<2		Project No:	1370 - DCT	Orientation:	Vertical
		Loose	5-10		Soft	2-4		Location:	Berth 205 extension	Elevation:	+3.457 CDP
		Medium dense	10-30		Firm	4-8		Logged By:	MAS Baleta	Coordinates:	X: 3306797.40
		Dense	30-50		Stiff	8-15		Drilling -	Fairbrother		Y: -1233.80
		Very dense	>50		Very Stiff	15-30		Contractor:	Geotechnical Engineering CC		Page 2 of 2
Project	PORT OF DURBAN BERTH DEEPENING Berths 203 to 205				General Description				Remarks		Depth (mbgl)
Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology			

The figure is a geological log diagram for a borehole. It features a vertical axis on the left representing depth in meters, ranging from -28.730 at the top to -51.780 at the bottom. A horizontal scale bar at the top indicates distance in meters, with values 0, 50, and 100. The central part of the diagram contains a series of rectangular boxes, each representing a sample interval. Each box includes four numerical values (e.g., 53, 44, 56, 67) and a letter code (e.g., WB, SPT, NWD4). To the right of these boxes is a vertical column of text providing detailed geological information for specific intervals:

- 28.730 to -30.280 m:** SPT N=12, WB.
- 30.280 to -31.280 m:** WB, SPT N=21, WB, SPT N=R.
- 31.280 to -32.00 m:** (-31.54, -37.04). Slightly moist, olive green brown, cohesionless, very dense, fine sandy SILT. Residual, St Lucia Fm.
- 32.00 to -33.080 m:** NWD4, SPT N=R.
- 33.080 to -34.280 m:** SPT N=R.
- 34.280 to -35.280 m:** NWD4, SPT N=25.
- 35.280 to -36.780 m:** NWD4, SPT N=R.
- 36.780 to -38.280 m:** (-37.04, -52.04). Dark brownish Olive-Green, moderately to highly weathered, closely to widely fractured, VERY SOFT ROCK, interbedded SILTSTONE and MUDSTONE. Cretaceous St. Lucia Formation.
- 38.280 to -39.480 m:** NWD4, SPT N=R.
- 39.480 to -39.780 m:** 31 / 13, 6, 3, NWD4, Shelby.
- 39.780 to -41.280 m:** 100 / -, - , - , NWD4.
- 41.280 to -42.280 m:** 60 / 43, 29, 11, NWD4.
- 42.280 to -43.480 m:** 97 / 65, 56, 8, NWD4.
- 43.480 to -44.280 m:** 100 / 100, 4, 6, NWD4.
- 44.280 to -45.780 m:** 104 / 89, 78, 15, NWD4.
- 45.780 to -47.280 m:** 84 / 79, 47, 11, NWD4.
- 47.280 to -48.780 m:** 70 / 68, 65, 5, NWD4.
- 48.780 to -50.280 m:** 80 / 77, 61, 7, NWD4.
- 50.280 to -51.780 m:** 100 / 100, 81, 7, NWD4.

On the far right, there are additional geological notes corresponding to specific depth ranges:

- 32.200 to -32.430 CDP:** Calcrete lens.
- 35.500 to -35.710 CDP:** Calcrete lens.
- 37.910 to -38.020 CDP:** Calcrete lens.
- 41.930 to -42.280 CDP:** Residual rock.
- 50.100 to -50.280 CDP:** Calcrete lens.
- 50.980 to -51.330 CDP:** Calcrete lens.

END OF BOREHOLE
(-51.78 CDP / 55.50 mbal)



1370-BHL05: Box 1 of 7



1370-BHL05: Box 2 of 7



1370-BHL05: Box 3 of 7



1370-BHL05: Box 4 of 7



1370-BHL05: Box 5 of 7



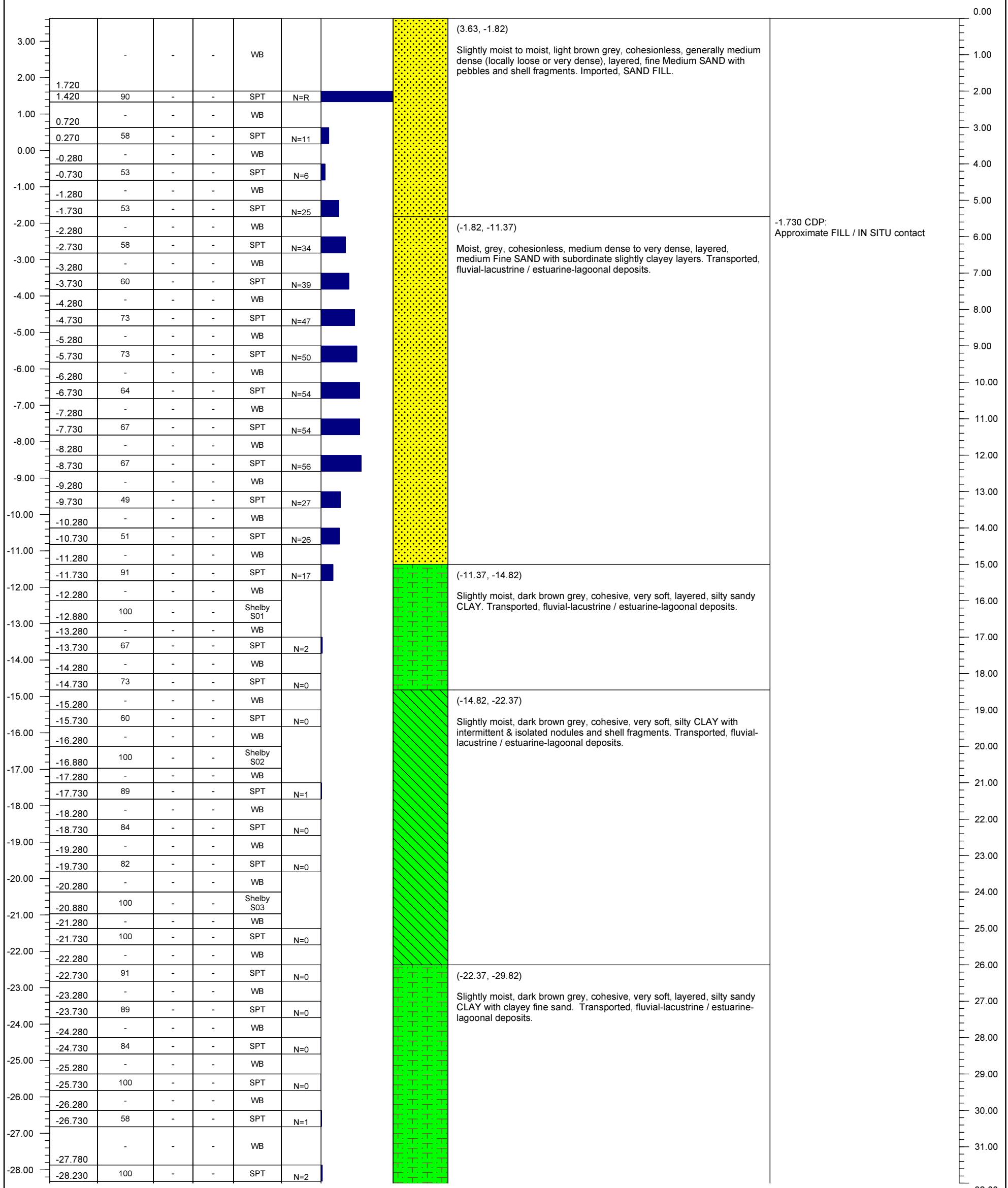
1370-BHL05: Box 6 of 7



1370-BHL05: Box 7 of 7

		ZAA Engineering Projects & Naval Architecture (Pty) Ltd						Sandy Materials		Clayey Materials		Borehole No. 1370 BHL06					
								Descriptions		SPT N	Descriptions		SPT N	Start Date	2012/07/02	End Date	2012/07/07
Project		PORT OF DURBAN BERTH DEEPENING Berths 203 to 205						Very loose		<5	Very Soft		<2	Project No:	1370 - DCT	Orientation:	Vertical
								Loose		5-10	Soft		2-4	Location:	Berth 205 extension	Elevation:	+3.630 CDP
								Medium dense		10-30	Firm		4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306824.20
								Dense		30-50	Stiff		8-15	Drilling -	Fairbrother	Y:	-1172.00
								Very dense		>50	Very Stiff		15-30	Contractor:	Geotechnical Engineering CC	Page 1 of 2	

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description			Remarks	Depth (mbgl)
0	100	0	100										0.00



		ZAA Engineering Projects & Naval Architecture (Pty) Ltd PORT OF DURBAN BERTH DEEPENING Berths 203 to 205						Sandy Materials		Clayey Materials		Borehole No. 1370 BHL06			
								Descriptions		SPT N	Descriptions		Start Date	2012/07/02	End Date
Project								Very loose		<5	Very Soft		<2		
								Loose		5-10	Soft		2-4		
								Medium dense		10-30	Firm		4-8		
								Dense		30-50	Stiff		8-15		
								Very dense		>50	Very Stiff		15-30		
Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	0	100	Lithology	General Description			Remarks	Depth (mbgl)
-29.00	-29.280	-	-	-	WB										32.00
-29.730	78	-	-	-	SPT	N=7					(-29.82, -30.87)				33.00
-30.780	-	-	-	-	WB						Moist, dark brown grey, slightly cohesive, loose, clayey SAND with some shell fragments. Transported, fluvial-lacustrine / estuarine-lagoonal deposits.				34.00
-31.00	67	-	-	-	SPT	N=R					(-30.87, -35.37)				35.00
-32.280	62	12	8	NWD4							Slightly moist, olive green brown, cohesionless, very dense, fine sandy SILT. Residual, St Lucia Fm.				36.00
-33.00	7	-	-	NWD4											37.00
-34.00	69	-	-	SPT	N=R										38.00
-35.00	-	-	-	WB											39.00
-35.280	73 / 65	35	11	NWD4							(-35.37, -42.87)				40.00
-36.780	80 / 77	72	7	NWD4							Dark brownish Olive-Green, moderately to highly weathered, closely to widely fractured, VERY SOFT ROCK, interbedded SILTSTONE and MUDSTONE. Cretaceous St. Lucia Formation.				41.00
-38.280	100 / 83	63	8	NWD4							No definitive primary fabric with poorly defined bedding as coarser horizons grade into finer horizons.				42.00
-39.780	100 / 99	89	10	NWD4							Joint surfaces are planar, smooth, no fill, sub-horizontal at irregular spacing (50-300mm), sub-vertical joints are conspicuous.				43.00
-41.280	100 / 89	73	11	NWD4							Note the presence of fossil shells and imprints associated with, but not confined to, hard concretionary layers.				44.00
-42.780											END OF BOREHOLE (-42.78 CDP / 46.50 mbgl)				45.00
															46.00



1370-BHL06: Box 1 of 5



1370-BHL06: Box 2 of 5



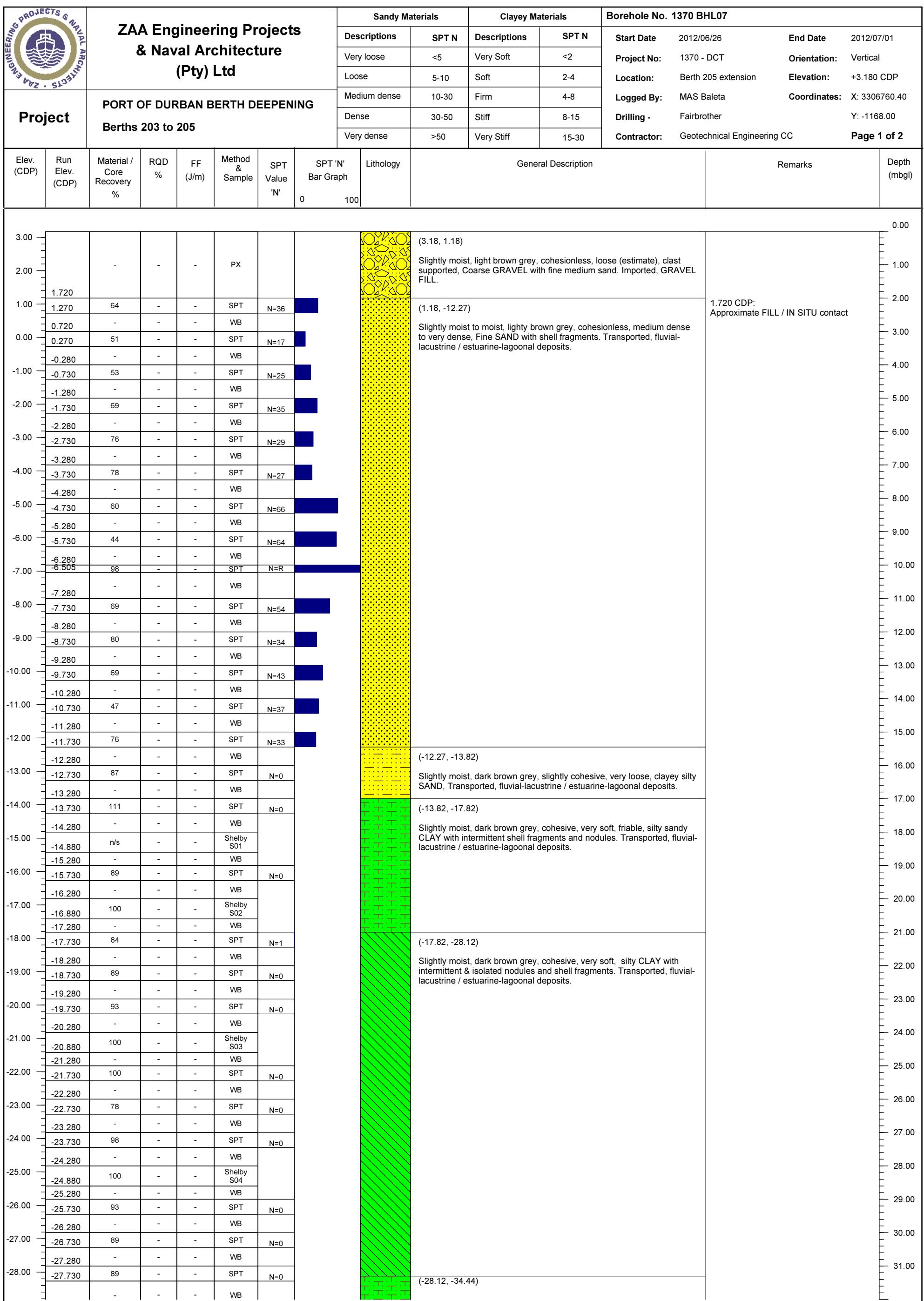
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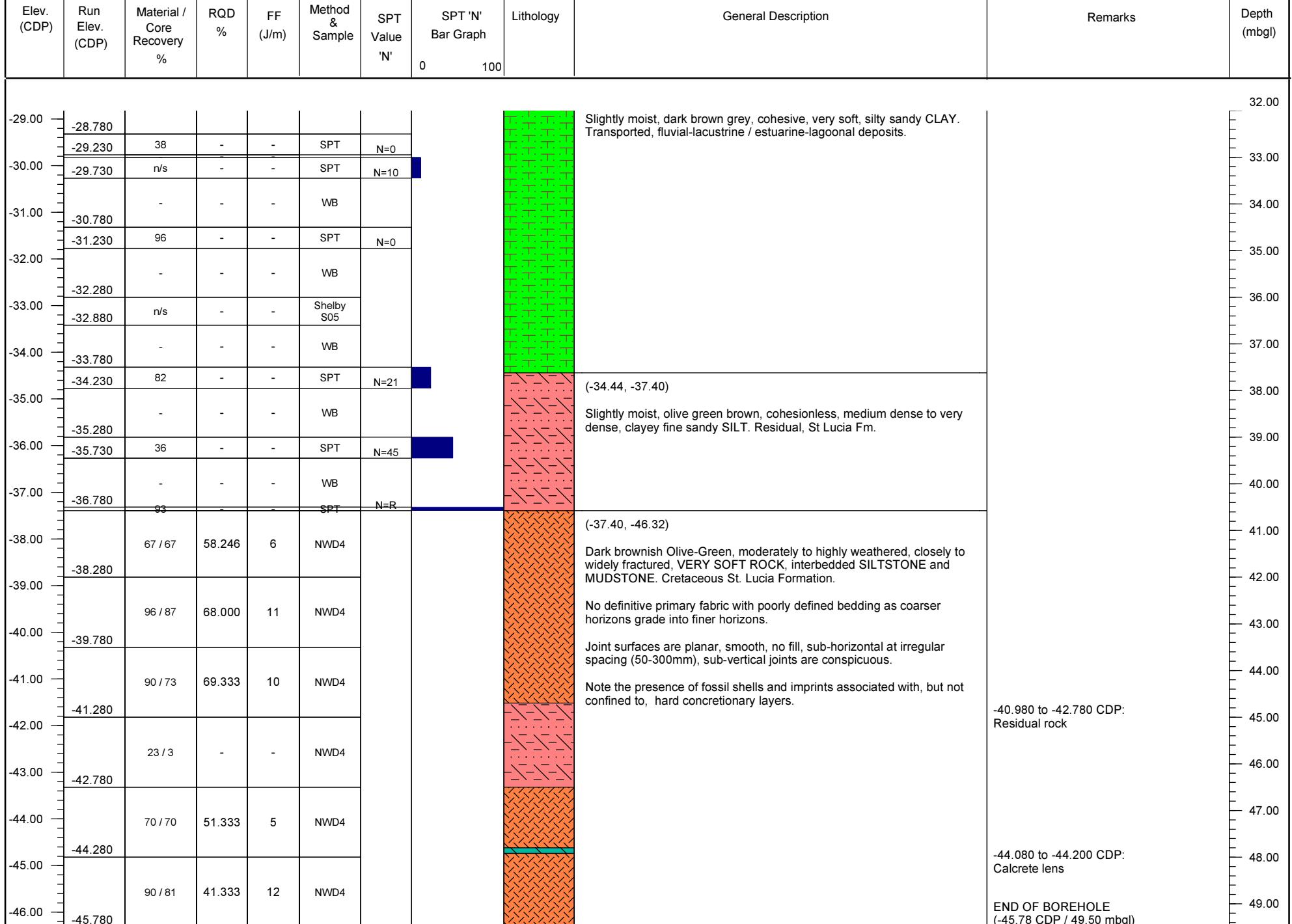
1370-BHL06: Box 4 of 5



1370-BHL06: Box 5 of 5



		ZAA Engineering Projects & Naval Architecture (Pty) Ltd PORT OF DURBAN BERTH DEEPENING Berths 203 to 205						Sandy Materials		Clayey Materials		Borehole No. 1370 BHL07				
								Descriptions		SPT N	Descriptions		Start Date	2012/06/26	End Date	2012/07/01
Project								Very loose		<5	Very Soft		Location:	1370 - DCT	Orientation:	Vertical
								Loose		5-10	Soft		Elevation:	+3.180 CDP	Coordinates:	X: 3306760.40
								Medium dense		10-30	Firm		Logged By:	MAS Baleta	Contractor:	Geotechnical Engineering CC
								Dense		30-50	Stiff		Drilling -	Fairbrother	Y: -1168.00	Page 2 of 2
								Very dense		>50	Very Stiff		Contractor:	Geotechnical Engineering CC	Page 2 of 2	





1370-BHL07: Box 1 of 5



1370-BHL07: Box 2 of 5



1370-BHL07: Box 3 of 5



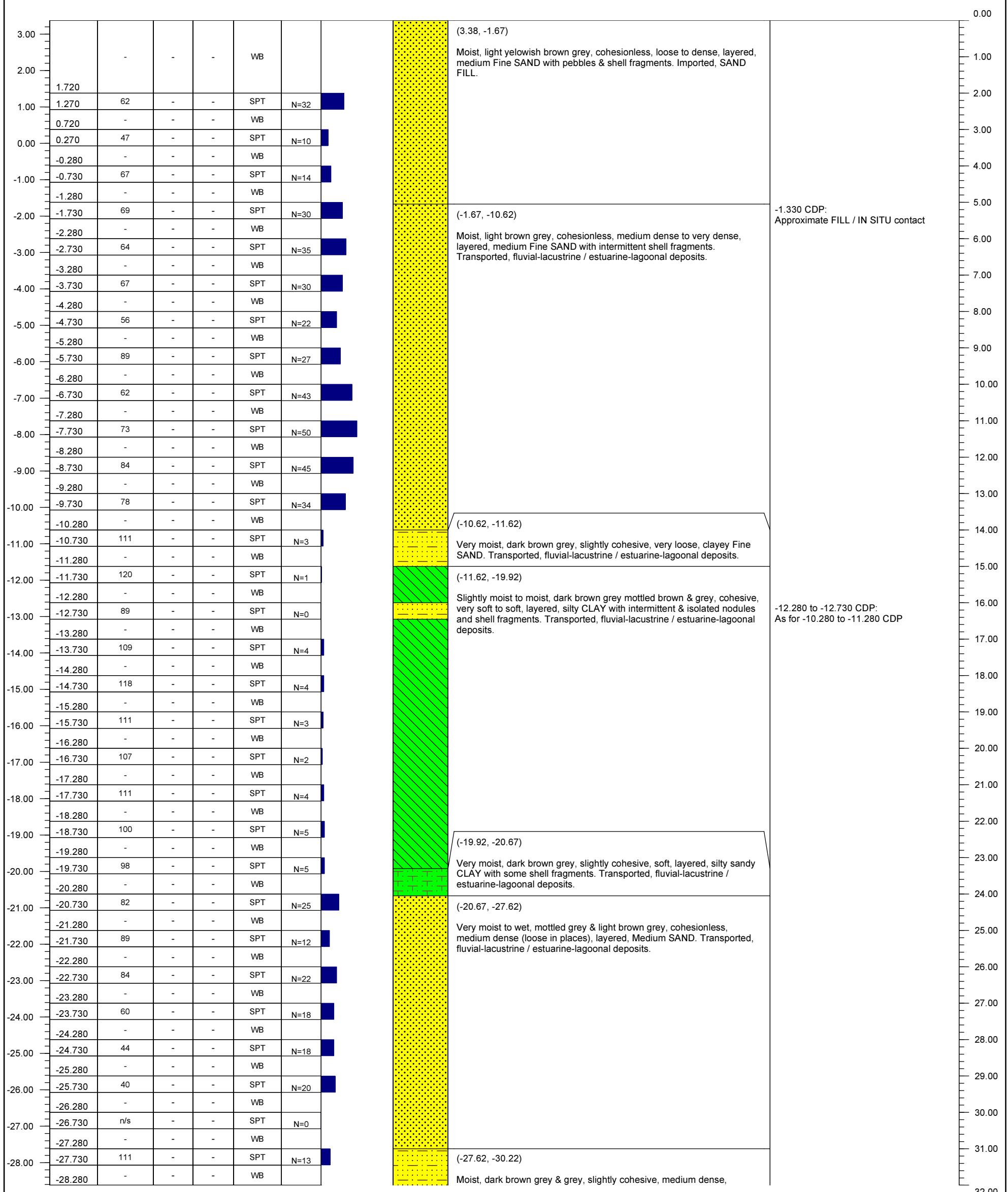
1370-BHL07: Box 4 of 5



1370-BHL07: Box 5 of 5

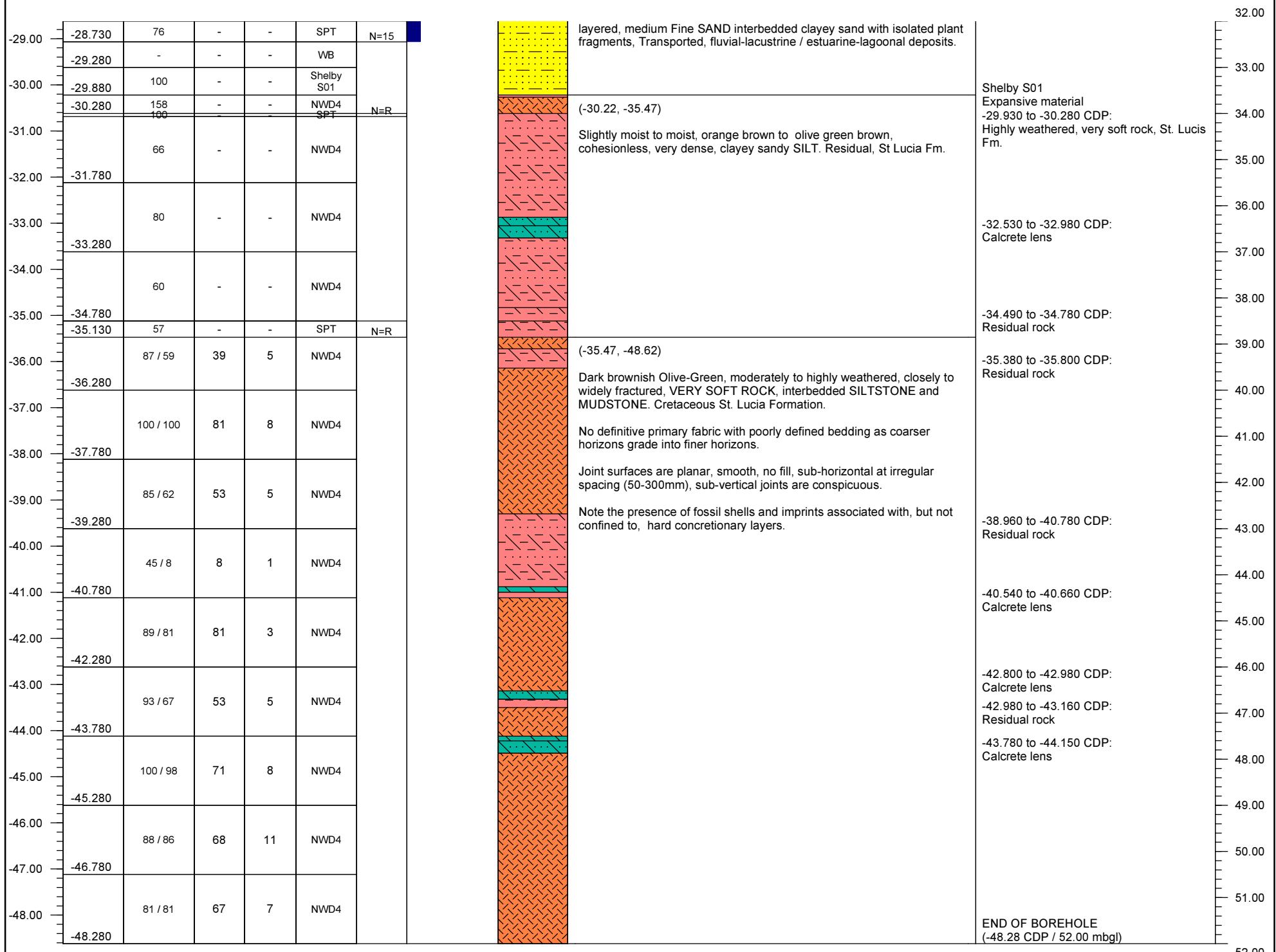
 ZAA Engineering Projects & Naval Architecture (Pty) Ltd		Project PORT OF DURBAN BERTH DEEPENING Berths 203 to 205		Sandy Materials		Clayey Materials		Borehole No. 1370 BHL08			
				Descriptions	SPT N	Descriptions	SPT N	Start Date	2012/07/24	End Date	2012/08/13
				Very loose	<5	Very Soft	<2	Project No:	1370 - DCT	Orientation:	Vertical
				Loose	5-10	Soft	2-4	Location:	Berth 205 extension	Elevation:	+3.380 CDP
				Medium dense	10-30	Firm	4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306715.00
				Dense	30-50	Stiff	8-15	Drilling -	Fairbrother		Y: -1151.00
				Very dense	>50	Very Stiff	15-30	Contractor:	Geotechnical Engineering CC	Page 1 of 2	

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description	Remarks	Depth (mbgl)
0	100										



		ZAA Engineering Projects & Naval Architecture (Pty) Ltd PORT OF DURBAN BERTH DEEPENING Berths 203 to 205						Sandy Materials		Clayey Materials		Borehole No. 1370 BHL08				
								Descriptions		SPT N	Descriptions		Start Date	2012/07/24 <th>End Date</th> <td>2012/08/13</td>	End Date	2012/08/13
Project								Very loose		<5	Very Soft		Project No:	1370 - DCT	Orientation:	Vertical
								Loose		5-10	Soft		Location:	Berth 205 extension	Elevation:	+3.380 CDP
								Medium dense		10-30	Firm		Logged By:	MAS Baleta	Coordinates:	X: 3306715.00
								Dense		30-50	Stiff		Drilling -	Fairbrother	Y:	-1151.00
								Very dense		>50	Very Stiff		Contractor:	Geotechnical Engineering CC	Page 2 of 2	

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description			Remarks	Depth (mbgl)
						0	100						







1370-BHL08: Box 3 of 6



1370-BHL08: Box 4 of 6



1370-BHL08: Box 5 of 6



1370-BHL08: Box 6 of 6



ZAA Engineering Projects & Naval Architecture (Pty) Ltd

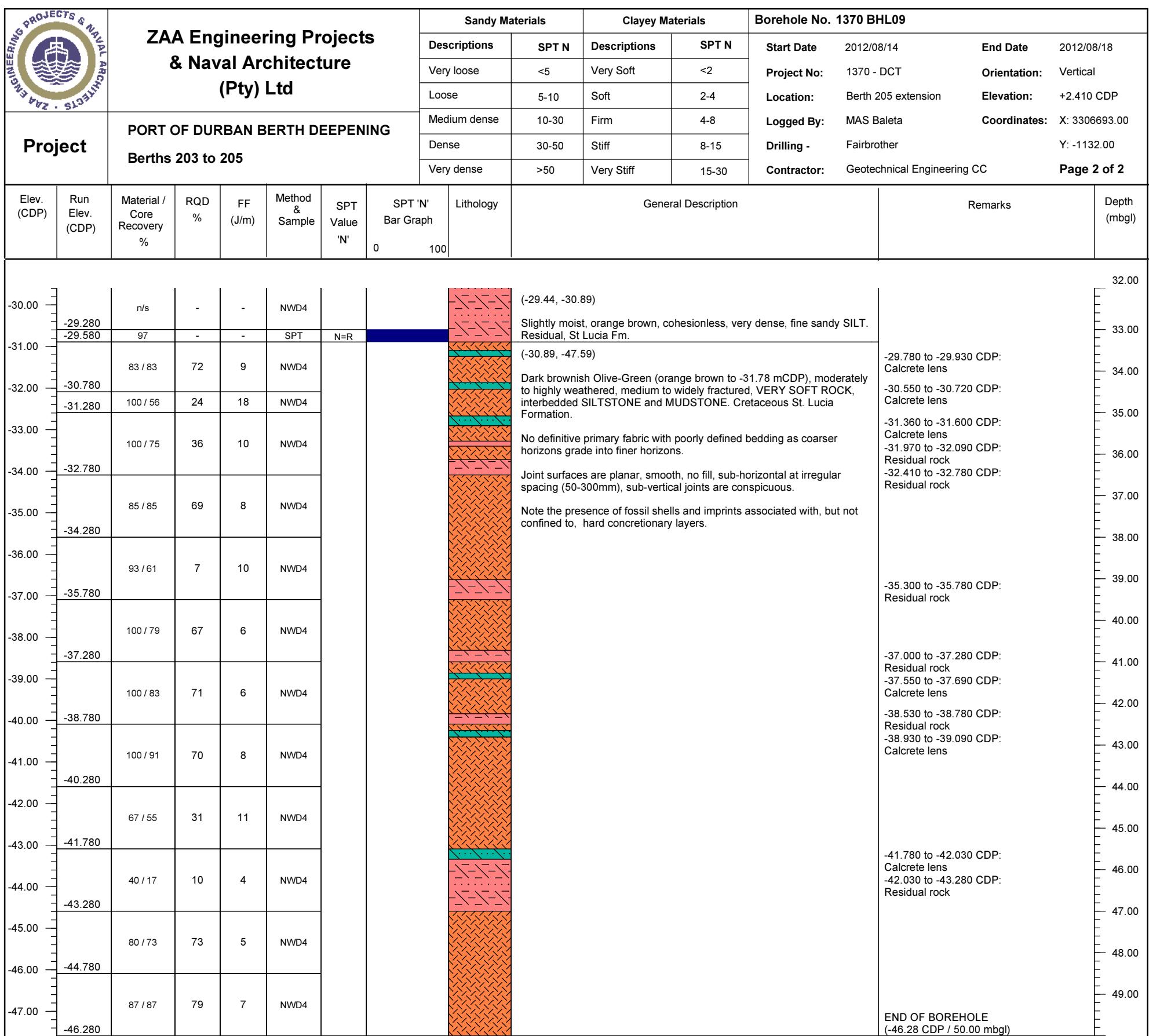
Project

PORT OF DURBAN BERTH DEEPENING

Berths 203 to 205

Sandy Materials		Clayey Materials		Borehole No. 1370 BHL09			
Descriptions	SPT N	Descriptions	SPT N	Start Date	2012/08/14	End Date	2012/08/18
Very loose	<5	Very Soft	<2	Project No:	1370 - DCT	Orientation:	Vertical
Loose	5-10	Soft	2-4	Location:	Berth 205 extension	Elevation:	+2.410 CDP
Medium dense	10-30	Firm	4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306693.00
Dense	30-50	Stiff	8-15	Drilling -	Fairbrother		Y: -1132.00
Very dense	>50	Very Stiff	15-30	Contractor:	Geotechnical Engineering CC	Page 1 of 2	
'l'	Lithology	General Description			Remarks		Depth

Borehole No. 1370 BHL09
Page 1 of 2





1370-BHL09: Box 1 of 5



1370-BHL09: Box 2 of 5



1370-BHL09: Box 3 of 5



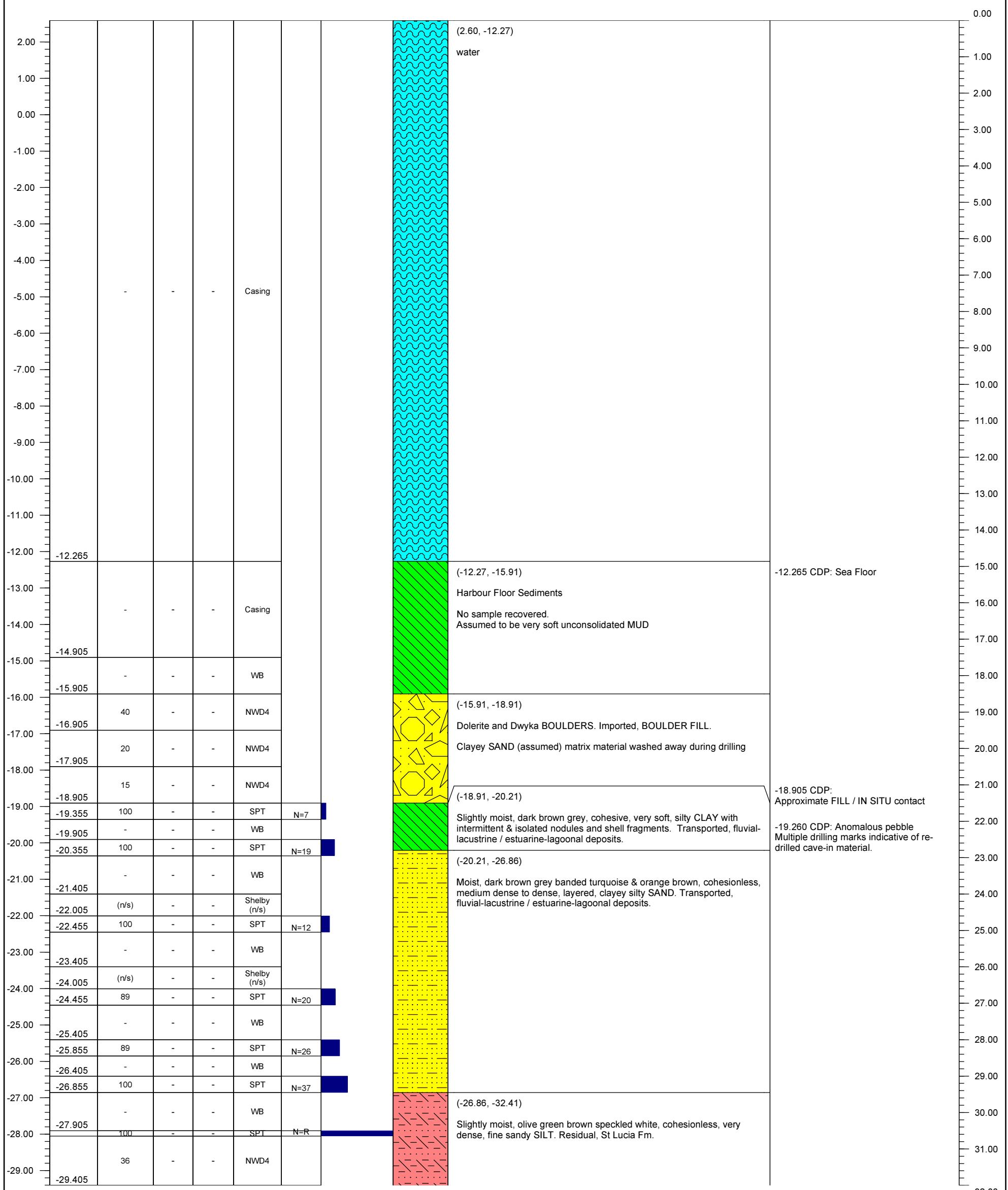
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1370-BHL09: Box 5 of 5

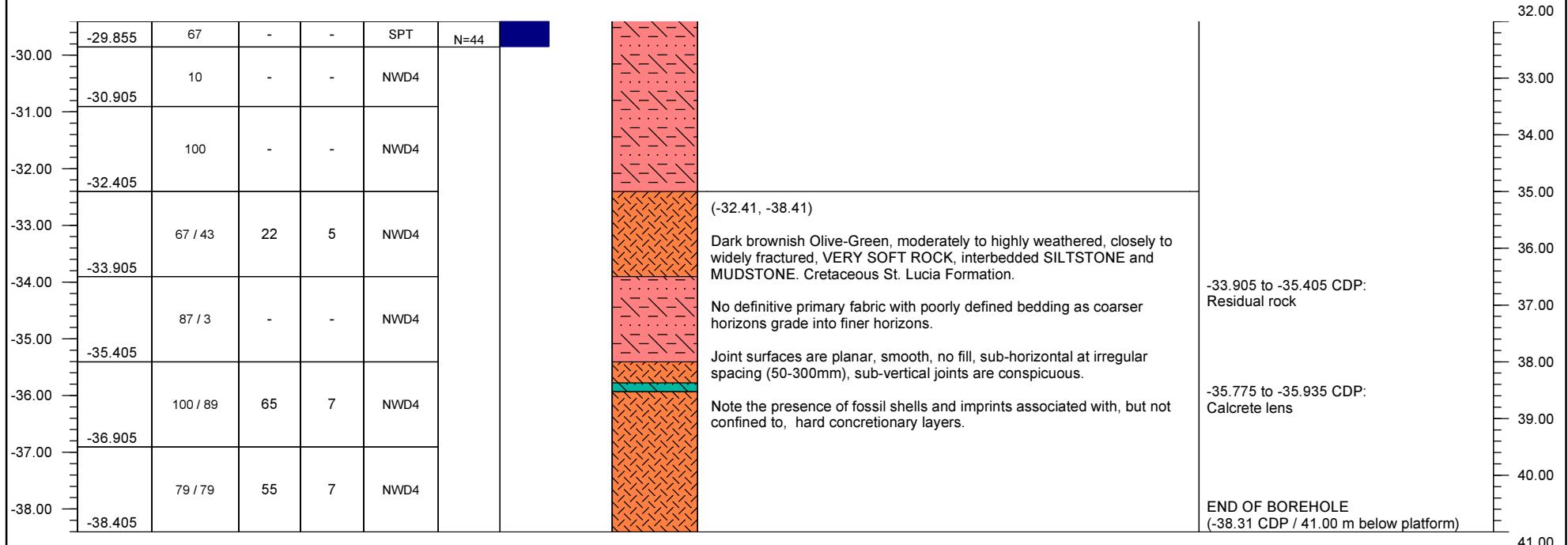
 ZAA Engineering Projects & Naval Architecture (Pty) Ltd	ZAA Engineering Projects & Naval Architecture (Pty) Ltd							Sandy Materials		Clayey Materials		Borehole No. 1370 BHS01			
								Descriptions		SPT N	Descriptions		Start Date	2012/07/18 <th>End Date</th> <td>2012/07/22</td>	End Date
	Project PORT OF DURBAN BERTH DEEPENING Berths 203 to 205							Very loose		<5	Very Soft		<2		
								Loose		5-10	Soft		2-4		
								Medium dense		10-30	Firm		4-8		
								Dense		30-50	Stiff		8-15		
								Very dense		>50	Very Stiff		15-30		

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description			Remarks	Depth (m below platform)
0	100												0.00



 ZAA Engineering Projects & Naval Architecture (Pty) Ltd	ZAA Engineering Projects & Naval Architecture (Pty) Ltd						Sandy Materials		Clayey Materials		Borehole No. 1370 BHS01							
							Descriptions		SPT N		Descriptions		SPT N		Start Date	2012/07/18	End Date	2012/07/22
	PORT OF DURBAN BERTH DEEPENING Berths 203 to 205						Very loose	<5	Very Soft	<2	Medium dense	10-30	Firm	4-8	Project No:	1370 - DCT	Orientation:	Vertical
							Loose	5-10	Soft	2-4	Dense	30-50	Stiff	8-15	Location:	Berth 205, Bollard 97	Elevation:	+2.595 CDP
							Very dense	>50	Very Stiff	15-30	Very dense	>50	Very Stiff	15-30	Logged By:	MAS Baleta	Coordinates:	X: 3306773.03
							Project	Drilling -	Fairbrother	Contractor:	Geotechnical Engineering CC	Page 2 of 2			Page 2 of 2			

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description			Depth (m below platform)
0	100	67	-	-	NWD4	N=44						100





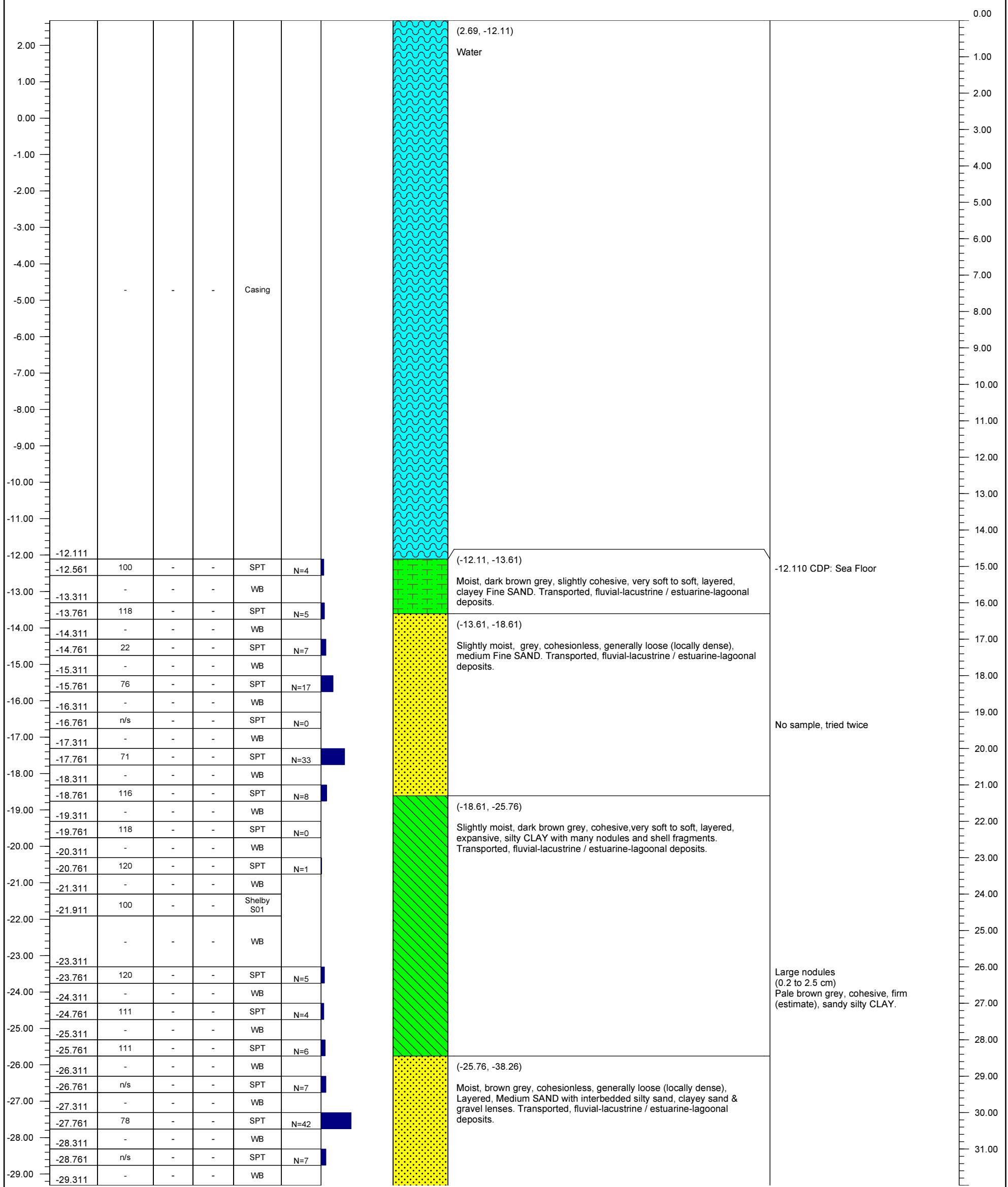
1370-BHS01: Box 1 of 2



1370-BHS01: Box 2 of 2

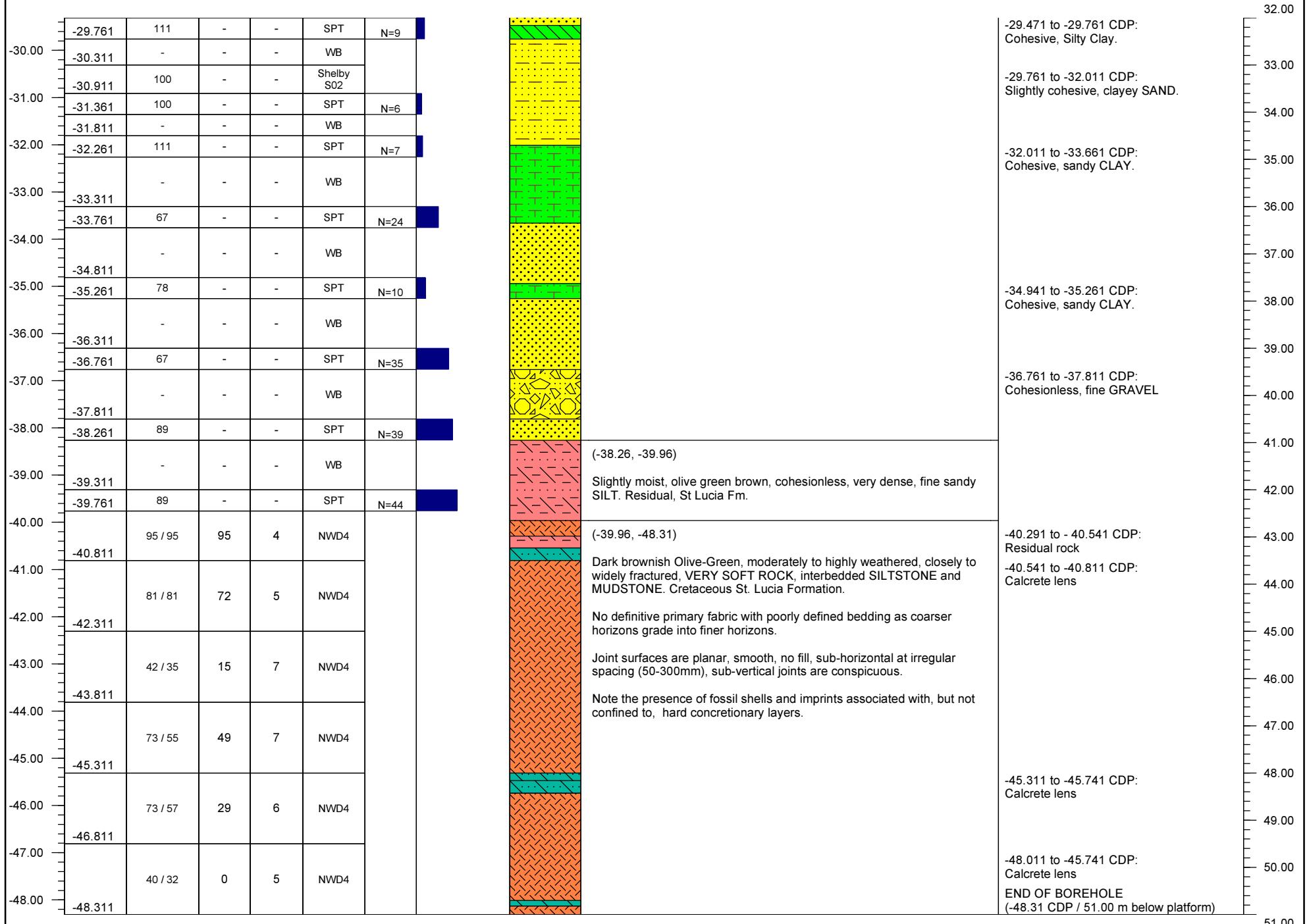
 ZAA Engineering Projects & Naval Architecture (Pty) Ltd	ZAA Engineering Projects & Naval Architecture (Pty) Ltd							Sandy Materials		Clayey Materials		Borehole No. 1370 BHS02			
								Descriptions	SPT N	Descriptions	SPT N	Start Date	2012/07/26 <th>End Date</th> <td>2012/07/27</td>	End Date	2012/07/27
								Very loose	<5	Very Soft	<2	Project No:	1370 - DCT	Orientation:	Vertical
								Loose	5-10	Soft	2-4	Location:	Berth 205, Bollard 82	Elevation:	+2.689 CDP
								Medium dense	10-30	Firm	4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306694.60
								Dense	30-50	Stiff	8-15	Drilling -	Fairbrother	Y:	-1554.20
								Very dense	>50	Very Stiff	15-30	Contractor:	Geotechnical Engineering CC	Page 1 of 2	

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description			Remarks	Depth (m below platform)
0	100	-	-	-	SPT	N=4	0	100					0.00



 ZAA Engineering Projects & Naval Architecture (Pty) Ltd	ZAA Engineering Projects & Naval Architecture (Pty) Ltd							Sandy Materials		Clayey Materials		Borehole No. 1370 BHS02			
								Descriptions	SPT N	Descriptions	SPT N	Start Date	2012/07/26	End Date	2012/07/27
								Very loose	<5	Very Soft	<2	Project No:	1370 - DCT	Orientation:	Vertical
								Loose	5-10	Soft	2-4	Location:	Berth 205, Bollard 82	Elevation:	+2.689 CDP
								Medium dense	10-30	Firm	4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306694.60
								Dense	30-50	Stiff	8-15	Drilling -	Fairbrother	Y:	-1554.20
								Very dense	>50	Very Stiff	15-30	Contractor:	Geotechnical Engineering CC	Page 2 of 2	

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description			Remarks	Depth (m below platform)
0	100	100	100	100	WB	N=9	0	100					32.00



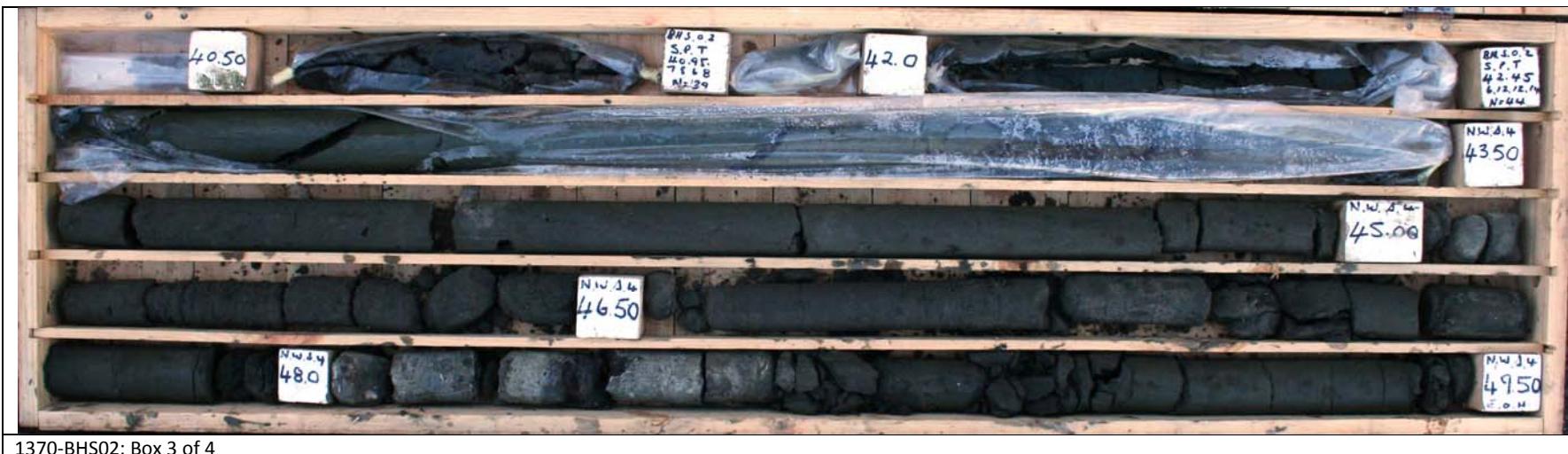
ZAA Engineering Projects & Naval Architecture (Pty) Ltd 31 Melkhout Crescent Hout Bay Cape Town 7806 T: +27 (0) 21 791 9100	ZAA Project Number: 1370 Client Project Number: M-2122830-408	Client: Transnet Project: PORT OF DURBAN BERTH DEEPENING Berths 203 to 205 Borehole No.: 1370 BHS02
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1370-BHS02: Box 1 of 4



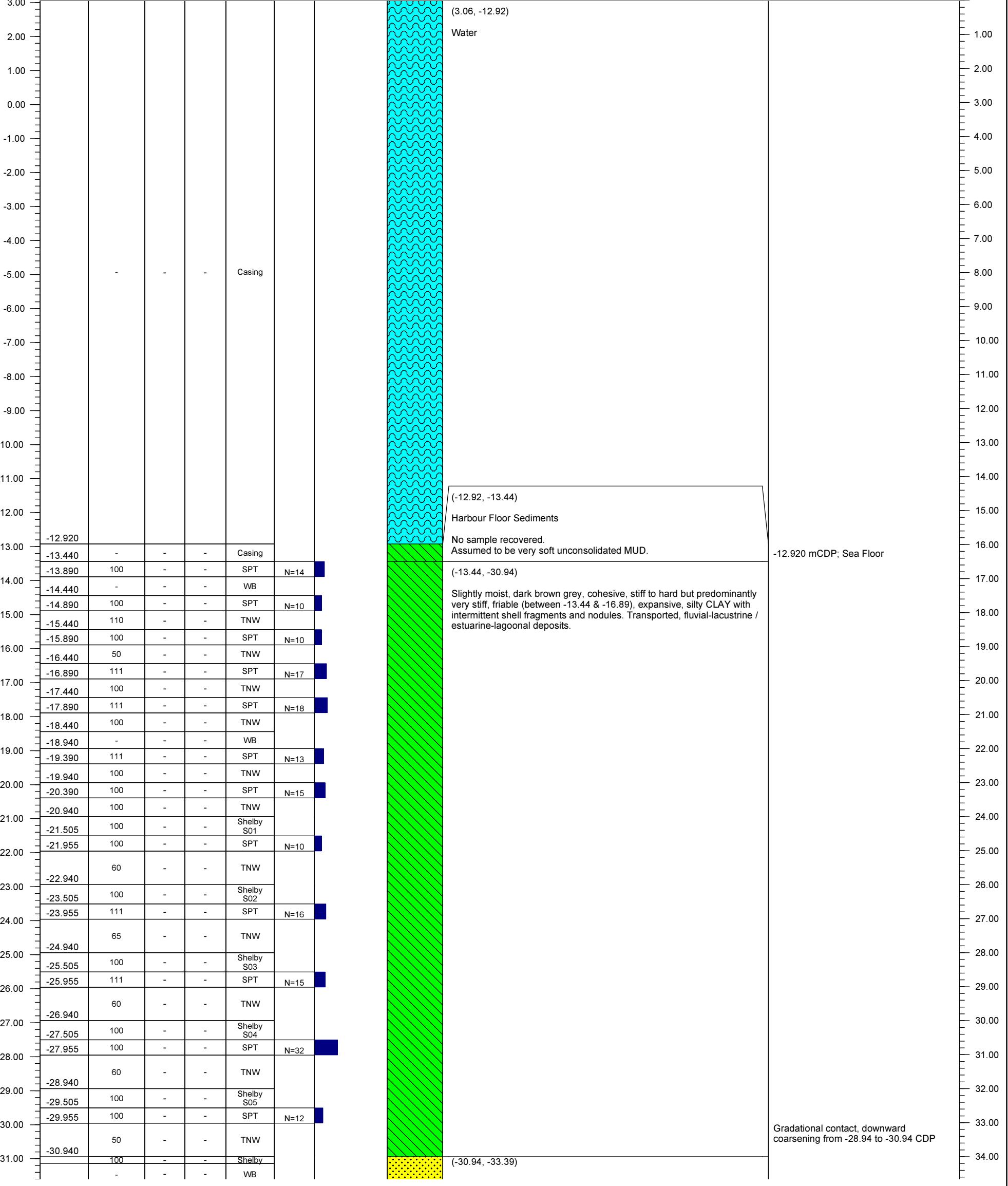
1370-BHS02: Box 2 of 4



1370-BHS02: Box 3 of 4



1370-BHS02: Box 4 of 4

		ZAA Engineering Projects & Naval Architecture (Pty) Ltd						Sandy Materials		Clayey Materials		Borehole No. 1370 BHS03								
								Descriptions		SPT N	Descriptions		SPT N	Start Date	2012/07/06	End Date	2012/07/15			
Project PORT OF DURBAN BERTH DEEPENING Berths 203 to 205								Very loose		<5	Very Soft		<2	Project No:	1370 - DCT	Orientation:	Vertical			
								Loose		5-10	Soft		2-4	Location:	Berth 203, Bollard 41	Elevation:	+3.060 CDP			
								Medium dense		10-30	Firm		4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306549.66			
								Dense		30-50	Stiff		8-15	Drilling -	Fairbrother	Y:	-2230.81			
								Very dense		>50	Very Stiff		15-30	Contractor:	Geotechnical Engineering CC	Page 1 of 2				
Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	0	100	Lithology	General Description			Remarks		Depth (m below platform)				
 <p>The borehole log diagram illustrates the subsurface profile from -12.920 m to -31.00 m CDP. The vertical axis on the right shows depth in meters, ranging from 0.00 at the top to 34.00 at the bottom. The horizontal axis at the bottom shows the borehole depth in meters, ranging from -12.920 to -31.00. The diagram features several distinct soil layers, each represented by a different pattern. A blue wavy pattern at the top represents water. Below it, a green diagonal-hatched pattern represents Harbour Floor Sediments. A yellow dotted pattern at the bottom represents a Shelby S04 sample. The diagram also includes SPT values and descriptions for each layer, such as 'Casing' at -13.440 m and 'Shelby S01' at -21.505 m.</p>																				
<p>(3.06, -12.92) Water</p> <p>(-12.92, -13.44) Harbour Floor Sediments No sample recovered. Assumed to be very soft unconsolidated MUD.</p> <p>(-13.44, -30.94) Slightly moist, dark brown grey, cohesive, stiff to hard but predominantly very stiff, friable (between -13.44 & -16.89), expansive, silty CLAY with intermittent shell fragments and nodules. Transported, fluvial-lacustrine / estuarine-lagoonal deposits.</p> <p>(-30.94, -33.39) Gradational contact, downward coarsening from -28.94 to -30.94 CDP</p>														-12.920 mCDP; Sea Floor						
<p>ZAA Engineering Projects & Naval Architecture (Pty) Ltd</p> <p>31 Melkhout Crescent Hout Bay Cape Town 7806 T: +27 (0) 21 791 9100</p> <p>ZAA Project Number: 1370</p> <p>Client Project Number: M-2122830-408</p> <p>Client: Transnet Project: PORT OF DURBAN BERTH DEEPENING Berths 203 to 205</p> <p>Borehole No.: 1370 BHS03</p>																				

		ZAA Engineering Projects & Naval Architecture (Pty) Ltd PORT OF DURBAN BERTH DEEPENING Berths 203 to 205						Sandy Materials		Clayey Materials		Borehole No. 1370 BHS03							
								Descriptions		SPT N	Descriptions		SPT N	Start Date	2012/07/06	End Date	2012/07/15		
Project	PORT OF DURBAN BERTH DEEPENING Berths 203 to 205							Very loose		<5	Very Soft		<2	Project No:	1370 - DCT	Orientation:	Vertical		
								Loose		5-10	Soft		2-4	Location:	Berth 203, Bollard 41	Elevation:	+3.060 CDP		
								Medium dense		10-30	Firm		4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306549.66		
								Dense		30-50	Stiff		8-15	Drilling -	Fairbrother	Y:	-2230.81		
								Very dense		>50	Very Stiff		15-30	Contractor:	Geotechnical Engineering CC	Page 2 of 2			
Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	0	100	Lithology	General Description			Remarks		Depth (m below platform)			
-31.940																			
-32.00	100	-	-	-	SPT	N=R					Slightly moist, dark brown grey, cohesionless, very dense, bedded, coarse to Medium SAND with subordinate clayey fine sand. Transported, fluvial-lacustrine / estuarine-lagoonal deposits.					35.00			
-32.940	-	-	-	-	WB						<p>-32.940 to -33.160 CDP: Clayey Fine SAND Wash bore drilling (WB) concentrates gravel sized material</p> <p>(-33.39, -36.32)</p> <p>-34.940 to -35.340 CDP: Medium SAND</p> <p>-36.190 to -36.320 CDP: Medium SAND</p> <p>Casing sink under own weight. Assume very loose conditions.</p>					36.00			
-33.390	89	-	-	-	SPT	N=50										37.00			
-34.640	-	-	-	-	Casing Sunk											38.00			
-34.940	-	-	-	-	WB											39.00			
-35.390	89	-	-	-	SPT	N=46										40.00			
-35.940	-	-	-	-	WB						<p>(-39.82, -40.77)</p> <p>Moist, brown & orange brown, cohesionless, very loose (assumed) to very dense, bedded, Fine SAND with clayey fine sand . Transported, fluvial-lacustrine / estuarine-lagoonal deposits.</p> <p>(-40.77, -41.940)</p> <p>Slightly moist, dark brown grey, cohesive, very stiff, friable, expansive, clayey fine sand to silty CLAY with nodules. Transported, fluvial-lacustrine / estuarine-lagoonal.</p> <p>(-43.24, -47.54)</p> <p>Slightly moist, brown grey with layers of orange brown & green brown, cohesionless, medium dense to very dense, layered, coarse Medium SAND with subordinate fine gravel & fine sand layers. Transported, fluvial-lacustrine / estuarine-lagoonal.</p> <p>(-47.54, -52.64)</p> <p>Moist, light brown grey, cohesionless, very dense, bedded, clayey fine sand & medium sand and Coarse SAND. Transported, fluvial-lacustrine / estuarine-lagoonal.</p> <p>Casing sink under own weight. Assume very loose conditions.</p>						41.00		
-36.390	60	-	-	-	SPT	N=21					42.00								
-36.940	-	-	-	-	WB						43.00								
-37.390	78	-	-	-	SPT	N=59					44.00								
-37.80	-	-	-	-	Casing Sunk						45.00								
-38.20	-	-	-	-	WB						<p>(-47.815, -48.00)</p> <p>Moist, light brown grey, cohesionless, very dense, bedded, clayey fine sand & medium sand and Coarse SAND. Transported, fluvial-lacustrine / estuarine-lagoonal.</p> <p>(-49.00, -50.940)</p> <p>Slightly moist, olive green brown, cohesionless, very dense, fine sandy SILT. Residual, St Lucia Fm.</p> <p>(-50.940, -51.165)</p> <p>Slightly moist, dark brown grey, cohesive, very stiff, friable, expansive, clayey fine sand to silty CLAY with nodules. Transported, fluvial-lacustrine / estuarine-lagoonal.</p> <p>(-51.165, -52.440)</p> <p>Slightly moist, olive green brown, cohesionless, very dense, fine sandy SILT. Residual, St Lucia Fm.</p> <p>(-52.440, -53.940)</p> <p>Dark brownish Olive-Green, moderately to highly weathered but predominantly highly weathered, closely to widely fractured, VERY SOFT ROCK, interbedded SILTSTONE and MUDSTONE. Cretaceous St. Lucia Formation.</p> <p>No definitive primary fabric with poorly defined bedding as coarser horizons grade into finer horizons.</p> <p>Joint surfaces are planar, smooth, no fill, sub-horizontal at irregular spacing (50-300mm), sub-vertical joints are conspicuous.</p> <p>Note the presence of fossil shells and imprints associated with, but not confined to, hard calcareous layers.</p> <p>Casing sink under own weight. Assume very loose conditions.</p>						46.00		
-38.60	-	-	-	-	WB						47.00								
-39.00	-	-	-	-	WB						48.00								
-39.40	-	-	-	-	WB						49.00								
-39.80	78	-	-	-	SPT	N=25					50.00								
-40.20	-	-	-	-	WB						<p>(-64.440, -65.80)</p> <p>(-65.80, -67.50)</p> <p>END OF BOREHOLE (-64.44 CDP / 67.50 m below platform)</p>						51.00		
-40.60	-	-	-	-	WB									52.00					
-41.00	-	-	-	-	WB												53.00		
-41.40	-	-	-	-	WB												54.00		
-41.80	-	-	-	-	WB												55.00		
-42.20	-	-	-	-	WB												56.00		
-42.60	-	-	-	-	WB												57.00		
-43.00	-	-	-	-	WB												58.00		
-43.40	-	-	-	-	WB												59.00		
-43.80	-	-	-	-	WB												60.00		
-44.20	-	-	-	-	WB												61.00		
-44.60	-	-	-	-	WB														



1370-BHS03: Box 1 of 7



1370-BHS03: Box 2 of 7



1370-BHS03: Box 3 of 7



1370-BHS03: Box 4 of 7



1370-BHS03: Box 5 of 7



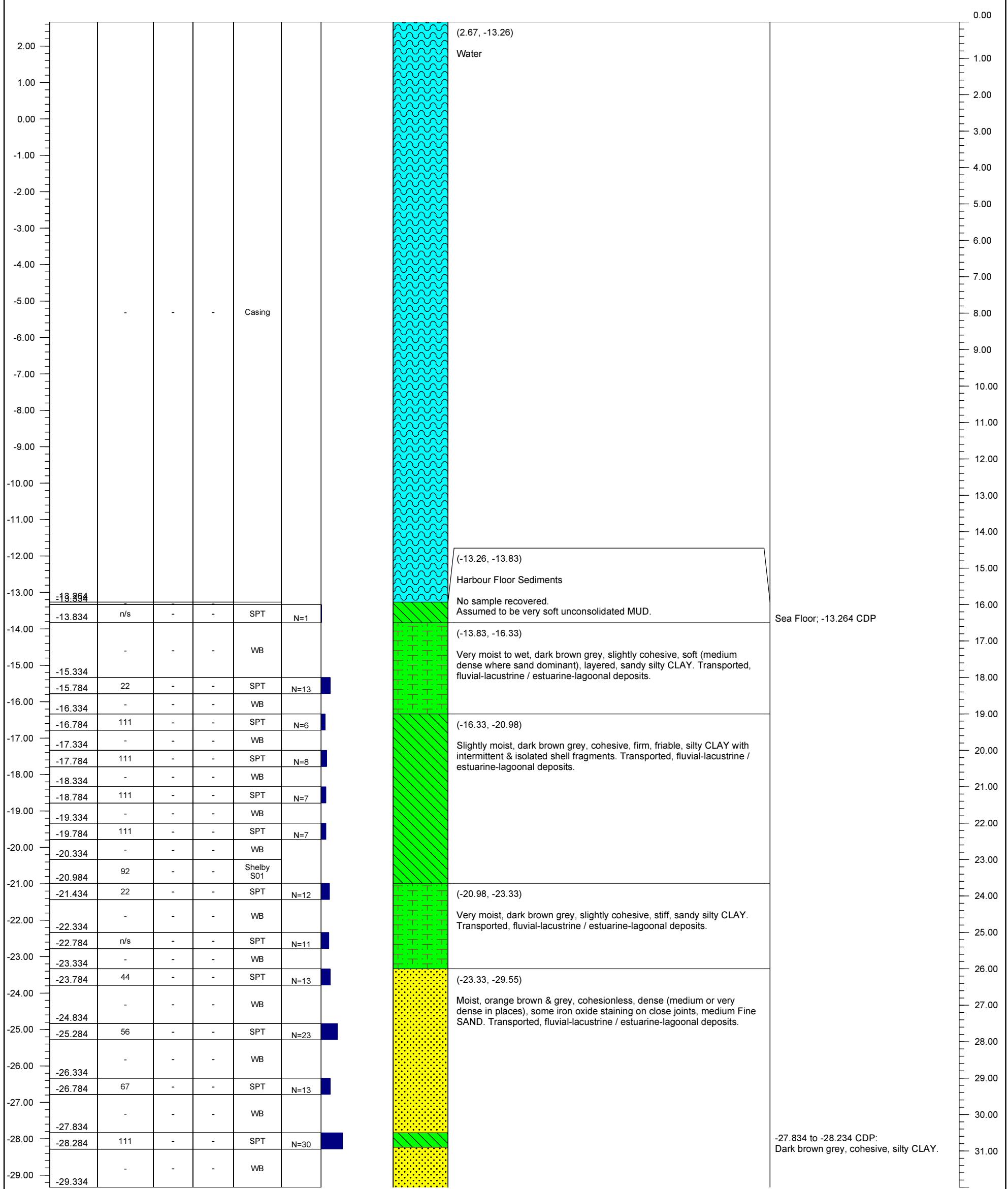
1370-BHS03: Box 6 of 7



1370-BHS03: Box 7 of 7

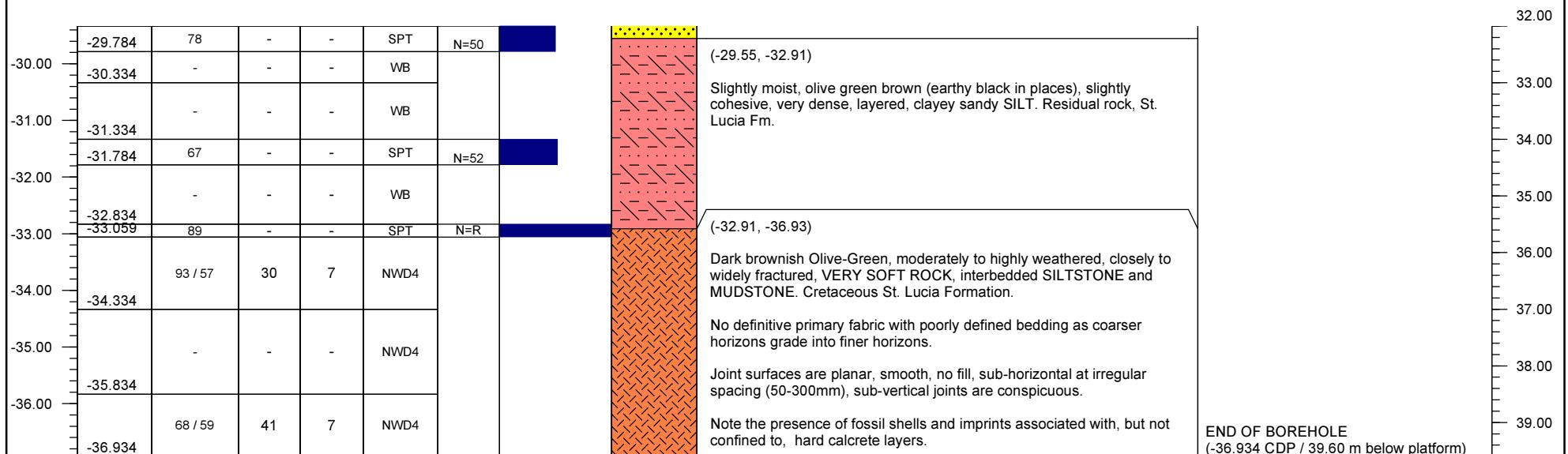
 ZAA Engineering Projects & Naval Architecture (Pty) Ltd	ZAA Engineering Projects & Naval Architecture (Pty) Ltd								Sandy Materials		Clayey Materials		Borehole No. 1370 BHS04			
	Descriptions		SPT N		Descriptions		SPT N		Start Date	2012/10/18 <th>End Date</th> <td>2012/10/19</td>	End Date	2012/10/19				
	Very loose	<5	Very Soft	<2	Medium dense	Firm	4-8	Project No:	1370 - DCT	Orientation:	Vertical					
	Loose	5-10	Soft	2-4				Location:	Berth 205, Bollard 86	Elevation:	+2.666 mCDP					
	Dense	30-50	Stiff	8-15				Logged By:	MAS Baleta	Coordinates:	X: 3306720.00					
	Very dense	>50	Very Stiff	15-30	Drilling -	Fairbrother	Y: -1466.00	Contractor:	Geotechnical Engineering CC	Page 1 of 2						

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description			Remarks		Depth (m below platform)
0	100													



		ZAA Engineering Projects & Naval Architecture (Pty) Ltd PORT OF DURBAN BERTH DEEPENING Berths 203 to 205						Sandy Materials		Clayey Materials		Borehole No. 1370 BHS04			
								Descriptions		SPT N	Descriptions		Start Date	2012/10/18	End Date
Project								Very loose		<5	Very Soft		<2		
								Loose		5-10	Soft		2-4		
								Medium dense		10-30	Firm		4-8		
								Dense		30-50	Stiff		8-15		
								Very dense		>50	Very Stiff		15-30		

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description			Remarks	Depth (m below platform)
0	100												

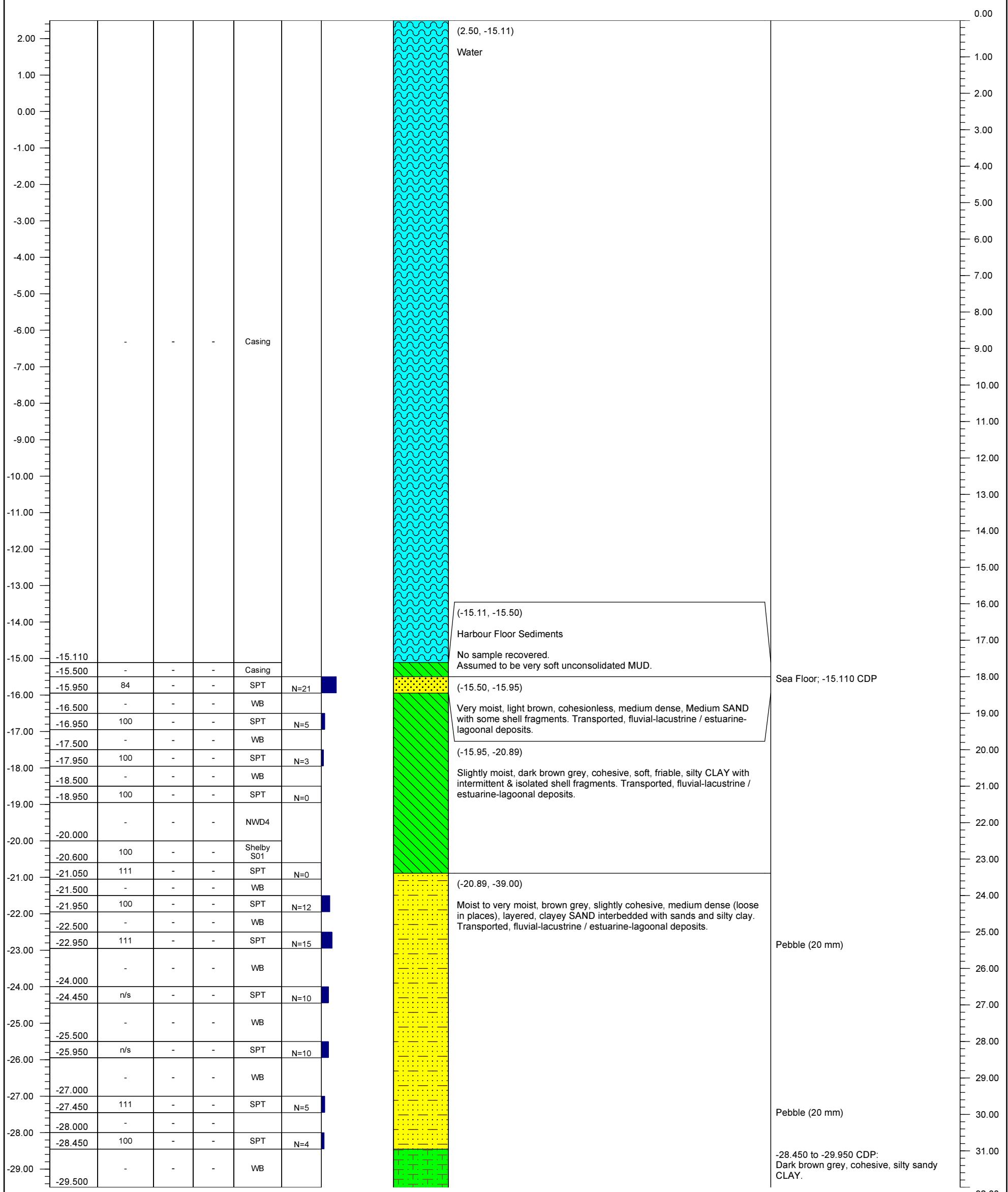


ZAA Engineering Projects & Naval Architecture (Pty) Ltd 31 Melkhout Crescent Hout Bay Cape Town 7806 T: +27 (0) 21 791 9100	ZAA Project Number: 1370 Client Project Number: M-2122830-408	Client: Transnet Project: PORT OF DURBAN BERTH DEEPENING Berths 203 to 205
		Borehole No.: 1370 BHS04

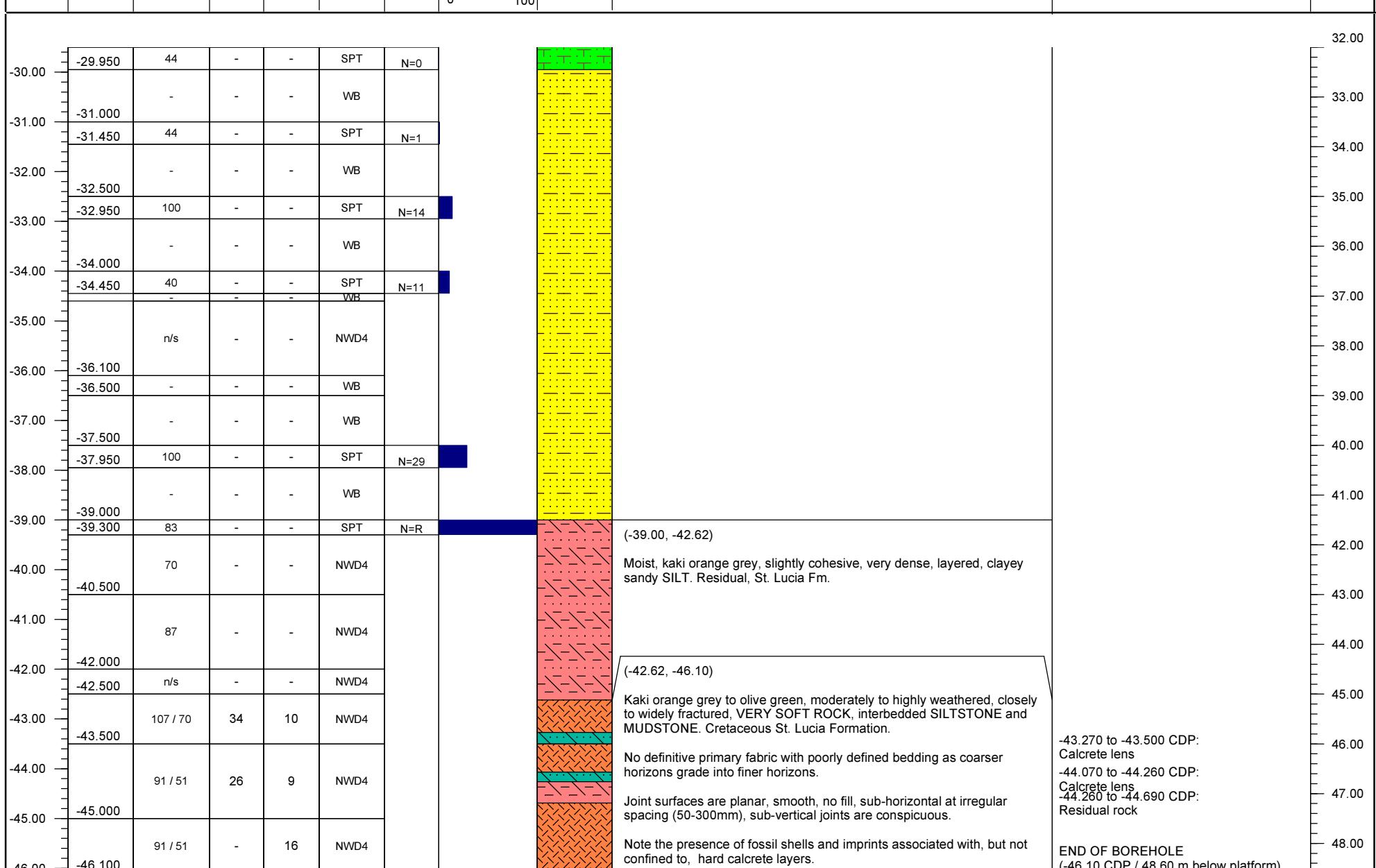


 ZAA Engineering Projects & Naval Architecture (Pty) Ltd	ZAA Engineering Projects & Naval Architecture (Pty) Ltd								Sandy Materials		Clayey Materials		Borehole No. 1370 BHS05			
	Descriptions		SPT N	Descriptions		SPT N	Start Date	2012/10/14 <th>End Date</th> <td>2012/10/16</td>	End Date	2012/10/16						
	Project PORT OF DURBAN BERTH DEEPENING Berths 203 to 205								Very loose	<5	Very Soft	<2	Project No:	1370 - DCT	Orientation:	Vertical
	Loose		5-10	Soft	2-4	Location:	Berth 204, Bollard 77	Elevation:	+2.500 mCDP							
	Medium dense		10-30	Firm	4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306680.00							
	Dense		30-50	Stiff	8-15	Drilling -	Fairbrother	Y:	-1634.00							
	Very dense		>50	Very Stiff	15-30	Contractor:	Geotechnical Engineering CC	Page 1 of 2								

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description			Remarks	Depth (m below platform)
0	100												0.00



		ZAA Engineering Projects & Naval Architecture (Pty) Ltd				Sandy Materials		Clayey Materials		Borehole No. 1370 BHS05			
						Descriptions	SPT N	Descriptions	SPT N	Start Date	2012/10/14	End Date	2012/10/16
Project	PORT OF DURBAN BERTH DEEPENING Berths 203 to 205					Very loose	<5	Very Soft	<2	Project No:	1370 - DCT	Orientation:	Vertical
						Loose	5-10	Soft	2-4	Location:	Berth 204, Bollard 77	Elevation:	+2.500 mCDP
						Medium dense	10-30	Firm	4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306680.00
						Dense	30-50	Stiff	8-15	Drilling -	Fairbrother	Y:	-1634.00
						Very dense	>50	Very Stiff	15-30	Contractor:	Geotechnical Engineering CC	Page 2 of 2	
Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description	Remarks	Depth (m below platform)		





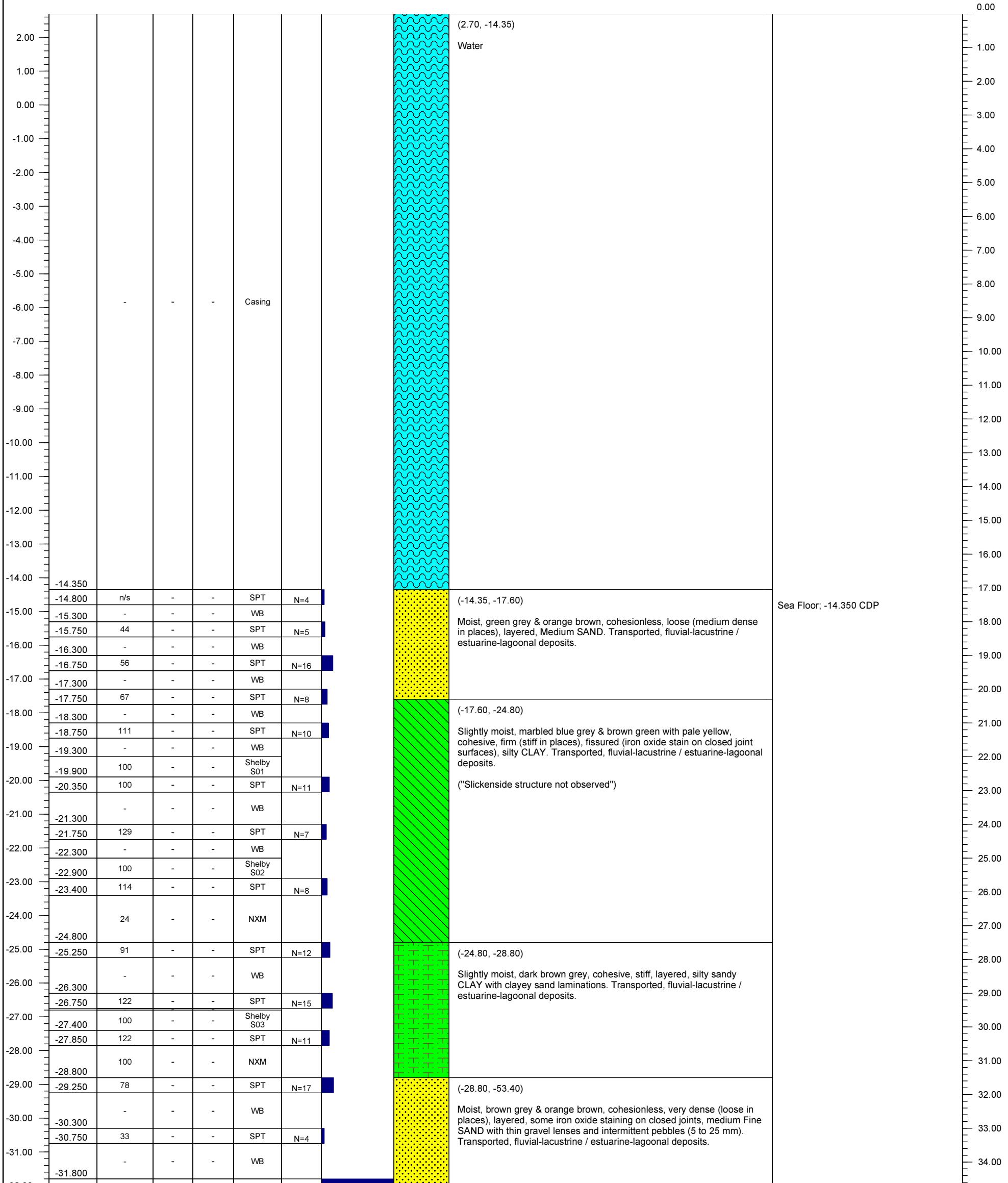
1370-BHS05: Box 1 of 3



1370-BHS05: Box 2 of 3

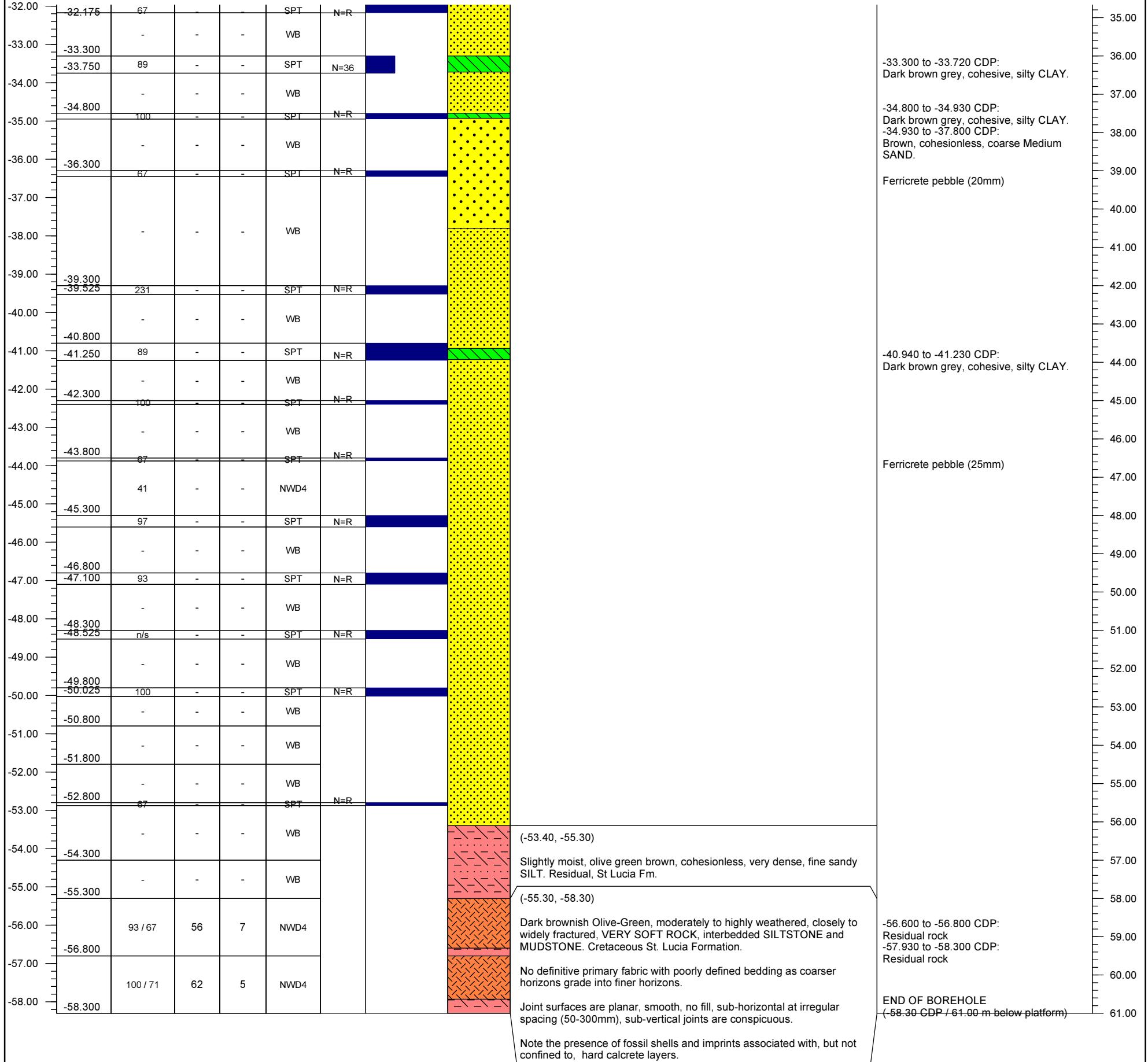


1370-BHS05: Box 3 of 3

		ZAA Engineering Projects & Naval Architecture (Pty) Ltd						Sandy Materials		Clayey Materials		Borehole No. 1370 BHS06					
								Descriptions		SPT N	Descriptions		SPT N	Start Date	2012/10/11	End Date	2012/10/14
Project PORT OF DURBAN BERTH DEEPENING Berths 203 to 205								Very loose		<5	Very Soft		<2	Project No:	1370 - DCT	Orientation:	Vertical
								Loose		5-10	Soft		2-4	Location:	Berth 204, Bollard 69	Elevation:	+2.700 mCDP
								Medium dense		10-30	Firm		4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306645.11
								Dense		30-50	Stiff		8-15	Drilling -	Fairbrother	Y:	-1778.59
								Very dense		>50	Very Stiff		15-30	Contractor:	Geotechnical Engineering CC	Page 1 of 2	
Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	0	100	Lithology	General Description			Remarks		Depth (m below platform)	
 <p>The borehole log diagram illustrates the subsurface profile. The vertical axis represents depth in meters (m CDP), ranging from -32.00 at the bottom to 0.00 at the top. The horizontal axis represents distance along the borehole. The diagram is divided into several distinct layers, each represented by a different color and texture pattern. Key features include: <ul style="list-style-type: none"> Top Layer (0.00 to ~-14.35 m): Labeled as 'Water' with coordinates (2.70, -14.35). Casing Layer (~-14.35 to ~-14.80 m): Labeled 'Casing'. Layer ~-14.80 to ~-15.30 m: SPT value N=4, labeled 'n/s'. Layer ~-15.30 to ~-15.75 m: SPT value N=5, labeled 'WB'. Layer ~-15.75 to ~-16.30 m: SPT value N=44, labeled 'WB'. Layer ~-16.30 to ~-16.75 m: SPT value N=56, labeled 'WB'. Layer ~-16.75 to ~-17.00 m: SPT value N=16, labeled 'WB'. Layer ~-17.00 to ~-17.75 m: SPT value N=8, labeled 'WB'. Layer ~-17.75 to ~-18.30 m: SPT value N=111, labeled 'WB'. Layer ~-18.30 to ~-18.75 m: SPT value N=10, labeled 'WB'. Layer ~-18.75 to ~-19.30 m: SPT value N=100, labeled 'WB'. Layer ~-19.30 to ~-19.90 m: SPT value N=100, labeled 'Shelby S01'. Layer ~-19.90 to ~-20.35 m: SPT value N=100, labeled 'WB'. Layer ~-20.35 to ~-21.30 m: SPT value N=100, labeled 'WB'. Layer ~-21.30 to ~-21.75 m: SPT value N=129, labeled 'WB'. Layer ~-21.75 to ~-22.30 m: SPT value N=7, labeled 'WB'. Layer ~-22.30 to ~-22.90 m: SPT value N=100, labeled 'Shelby S02'. Layer ~-22.90 to ~-23.40 m: SPT value N=114, labeled 'WB'. Layer ~-23.40 to ~-24.80 m: SPT value N=24, labeled 'NXM'. Layer ~-24.80 to ~-25.25 m: SPT value N=91, labeled 'WB'. Layer ~-25.25 to ~-26.30 m: SPT value N=12, labeled 'WB'. Layer ~-26.30 to ~-26.75 m: SPT value N=122, labeled 'WB'. Layer ~-26.75 to ~-27.40 m: SPT value N=122, labeled 'WB'. Layer ~-27.40 to ~-27.85 m: SPT value N=100, labeled 'Shelby S03'. Layer ~-27.85 to ~-28.80 m: SPT value N=122, labeled 'WB'. Layer ~-28.80 to ~-29.25 m: SPT value N=100, labeled 'NXM'. Layer ~-29.25 to ~-29.50 m: SPT value N=78, labeled 'WB'. Layer ~-29.50 to ~-30.30 m: SPT value N=17, labeled 'WB'. Layer ~-30.30 to ~-30.75 m: SPT value N=33, labeled 'WB'. Layer ~-30.75 to ~-31.80 m: SPT value N=4, labeled 'WB'. Bottom Layer (~-31.80 to -32.00 m): Labeled 'Sea Floor; -14.350 CDP'. The legend indicates the following patterns: <ul style="list-style-type: none"> Blue wavy pattern: (2.70, -14.35) Water Yellow dotted pattern: (-14.35, -17.60) Moist, green grey & orange brown, cohesionless, loose (medium dense in places), layered, Medium SAND. Transported, fluvial-lacustrine / estuarine-lagoonal deposits. Green diagonal hatching: (-17.60, -24.80) Slightly moist, marbled blue grey & brown green with pale yellow, cohesive, firm (stiff in places), fissured (iron oxide stain on closed joint surfaces), silty CLAY. Transported, fluvial-lacustrine / estuarine-lagoonal deposits. ("Slickenside structure not observed") Pink cross-hatching: (-24.80, -28.80) Slightly moist, dark brown grey, cohesive, stiff, layered, silty sandy CLAY with clayey sand laminations. Transported, fluvial-lacustrine / estuarine-lagoonal deposits. Yellow dotted pattern: (-28.80, -53.40) Moist, brown grey & orange brown, cohesionless, very dense (loose in places), layered, some iron oxide staining on closed joints, medium Fine SAND with thin gravel lenses and intermittent pebbles (5 to 25 mm). Transported, fluvial-lacustrine / estuarine-lagoonal deposits. </p>																	
ZAA Engineering Projects & Naval Architecture (Pty) Ltd 31 Melkhout Crescent Hout Bay Cape Town 7806 T: +27 (0) 21 791 9100	ZAA Project Number: 1370 Client Project Number: M-2122830-408	Client: Transnet Project: PORT OF DURBAN BERTH DEEPENING Berths 203 to 205 Borehole No.: 1370 BHS06															

 ZAA Engineering Projects & Naval Architecture (Pty) Ltd	ZAA Engineering Projects & Naval Architecture (Pty) Ltd						Sandy Materials		Clayey Materials		Borehole No. 1370 BHS06				
							Descriptions	SPT N	Descriptions	SPT N	Start Date	2012/10/11	End Date	2012/10/14	
							Very loose	<5	Very Soft	<2	Project No:	1370 - DCT	Orientation:	Vertical	
							Loose	5-10	Soft	2-4	Location:	Berth 204, Bollard 69	Elevation:	+2.700 mCDP	
							Medium dense	10-30	Firm	4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306645.11	
							Dense	30-50	Stiff	8-15	Drilling -	Fairbrother	Y:	-1778.59	
Project		PORT OF DURBAN BERTH DEEPENING Berths 203 to 205						Very dense	>50	Very Stiff	15-30	Contractor:	Geotechnical Engineering CC	Page 2 of 2	

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description			Depth (m below platform)
						0	100					





1370-BHS06: Box 1 of 4



1370-BHS06: Box 2 of 4



1370-BHS06: Box 3 of 4



1370-BHS06: Box 4 of 4

HOLE No: BD-BHM206
Sheet 1 of 6

JOB NUMBER: 07-395

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ -slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR-hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING		SR-soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ-close spacing	PLA-planar	
	MJ-medium spacing	UND-undulating	
	WJ-wide spacing	STE-stepped	
	VWJ-very wide spacg	IRR-irregular	

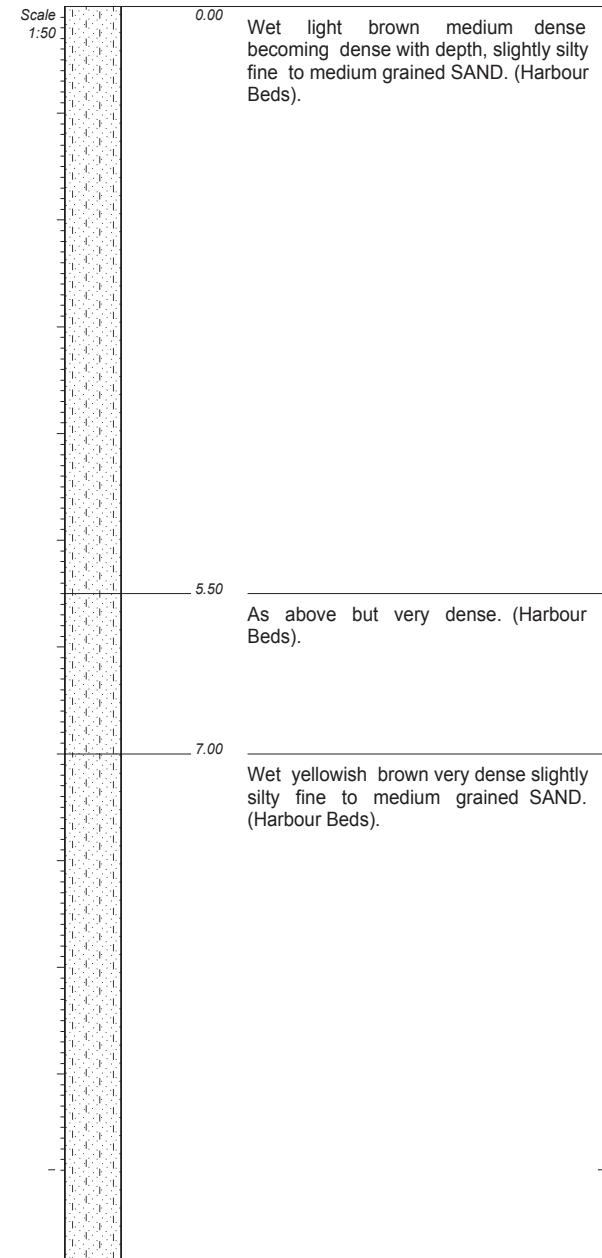


Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM206
Sheet 1 of 6

JOB NUMBER: 07-395

Drill Method and Size	Depth (m)	Material Recovery (%)	Core Recovery (%)	RQD (%)	Test Results	Set No.	Inclination (°)	Spac-ing	Joint shape	Rough-ness	Fill type	0	1	2	3	4	5	Elevation (m.a.m.s.l.)	DEPTH Scale 1:50
												Fill thick-ness (mm)	Fracture Frequency	Weathering code					
Wash Bore	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
SPT	1.50	100	-	-	N=31	-	-	-	-	-	-	-	-	-	-	-	-	-	
	1.95																		
Wash Bore	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
SPT	3.00	100	-	-	N=35	-	-	-	-	-	-	-	-	-	-	-	-	-	
	3.45																		
Wash Bore	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
SPT	4.50	100	-	-	N=34	-	-	-	-	-	-	-	-	-	-	-	-	-	
	4.95																		
Wash Bore	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
SPT	6.00	100	-	-	N=Ref 64	-	-	-	-	-	-	-	-	-	-	-	-	-	
	6.45																		
Wash Bore	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
Wash Bore	7.55	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
SPT	8.00	100	-	-	N=Ref 21	-	-	-	-	-	-	-	-	-	-	-	-	-	
	8.22																		
Wash Bore	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
SPT	9.50	100	-	-	N=Ref 41	-	-	-	-	-	-	-	-	-	-	-	-	-	
	9.78																		
Wash Bore	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	



HOLE No: BD-BHM206
Sheet 2 of 6

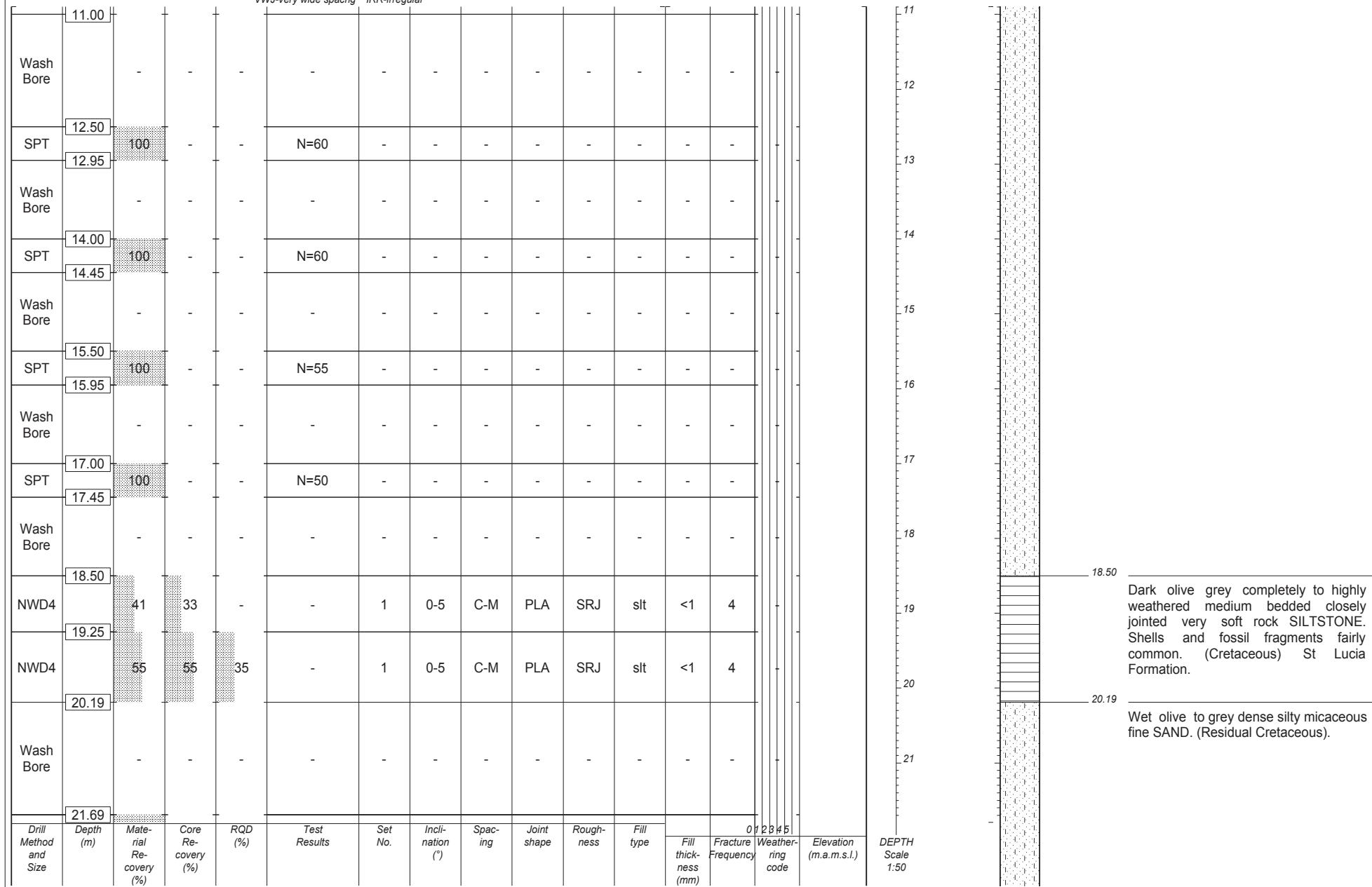
JOB NUMBER: 07-395

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ -slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR-hard rock
CF -cleaved	VCJ -very close spacg	CUR-curvilinear	MHR-medium hard rock
SF -schistose	CJ -close spacing	PLA-planar	SR-soft rock
GF -gneissose	MJ -medium spacing	UND-undulating	VSR-very soft rock
LF -laminated	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	



Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM206
Sheet 2 of 6
JOB NUMBER: 07-395



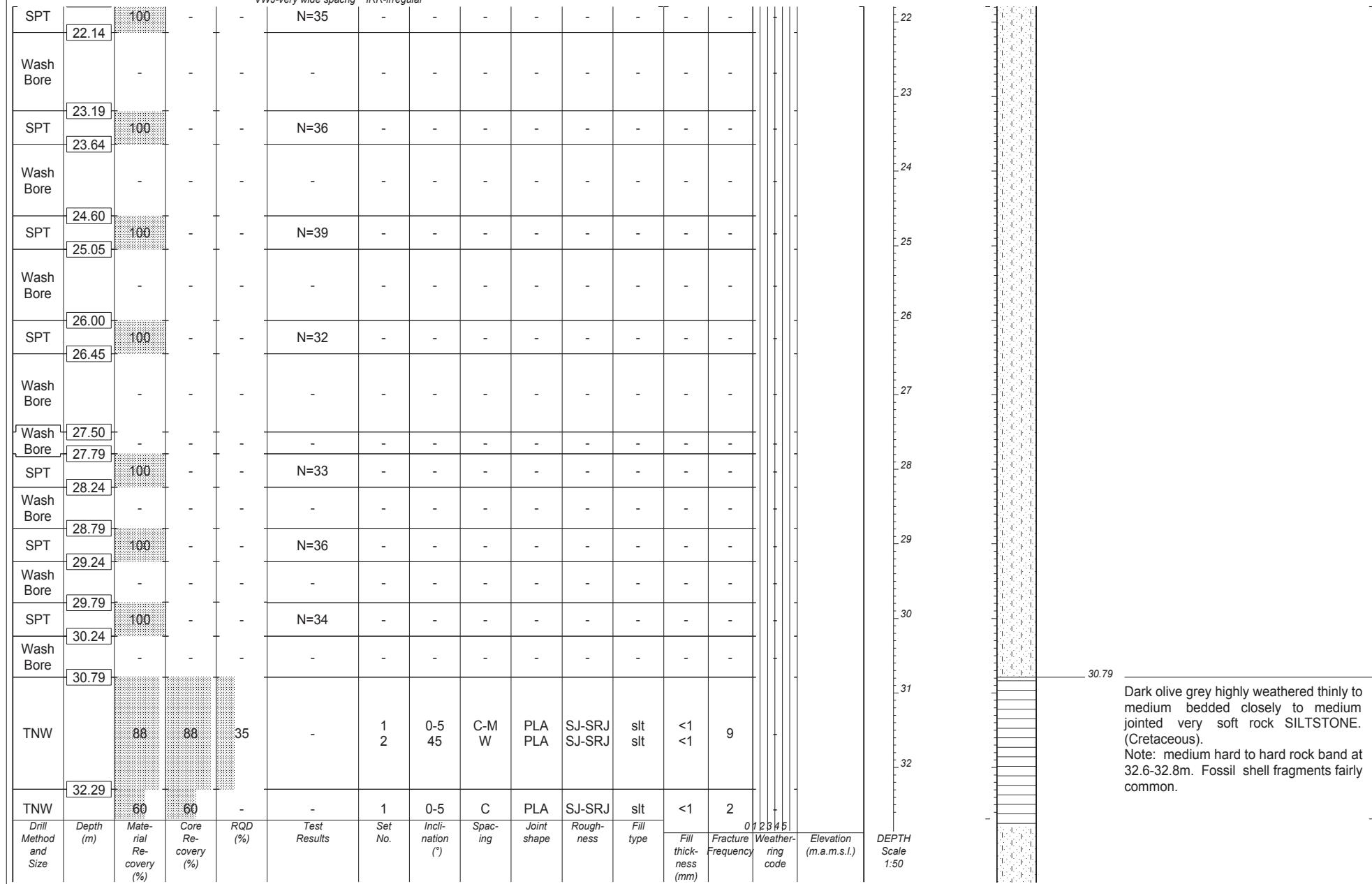
HOLE No: BD-BHM206
Sheet 3 of 6
JOB NUMBER: 07-395

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ -slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR-hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose			SR-soft rock
GF -gneissose			VSR-very soft rock
LF -laminated			
	VCJ-very close spacg	JOINT SPACING	
	CJ-close spacing	PLA-planar	
	MJ-medium spacing	UND-undulating	
	WJ-wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	



Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM206
Sheet 3 of 6
JOB NUMBER: 07-395



HOLE No: BD-BHM206
Sheet 4 of 6

JOB NUMBER: 07-395

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ -slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR-hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose			SR-soft rock
GF -gneissose			VSR-very soft rock
LF -laminated			

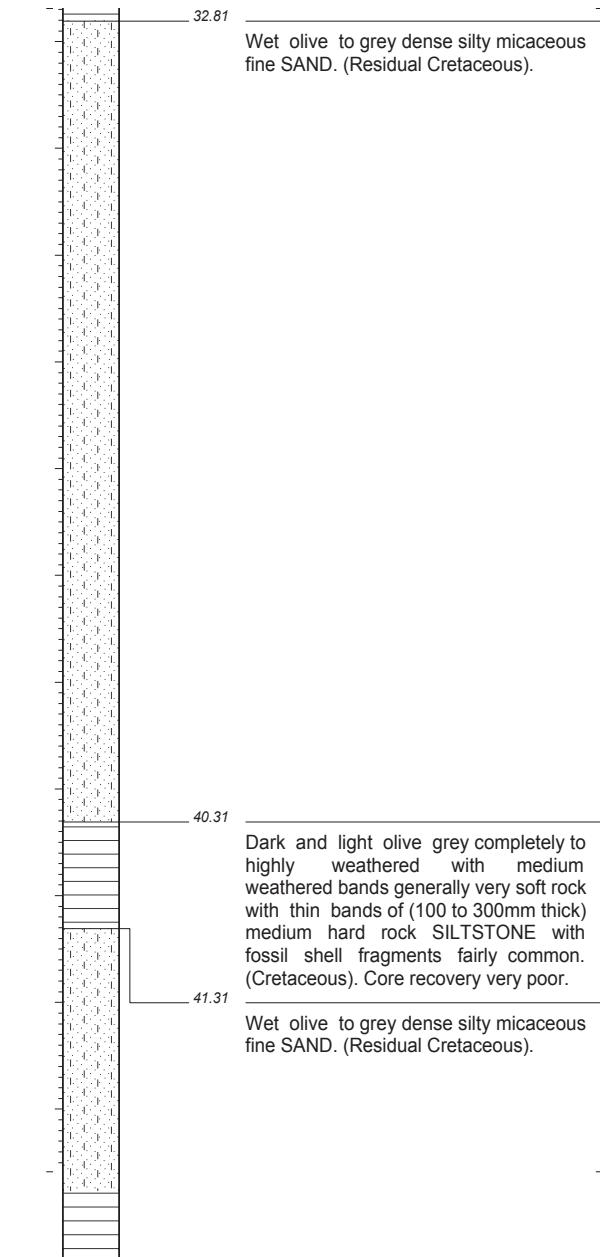
JOINT SPACING	JOINT SHAPE
VCJ-very close spacg	CUR-curvilinear
CJ -close spacing	PLA-planar
MJ -medium spacing	UND-undulating
WJ -wide spacing	STE-stepped
VWJ-very wide spacng	IRR-irregular



Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM206
Sheet 4 of 6
JOB NUMBER: 07-395

Drill Method and Size	Depth (m)	Material Recovery (%)	Core Recovery (%)	RQD (%)	Test Results	Set No.	Inclination (°)	Spac-ing	Joint shape	Rough-ness	Fill type	0	1	2	3	4	5	Elevation (m.a.m.s.l.)
												Fill thick-ness (mm)	Fracture Frequency	Weather-ing code				DEPTH Scale 1:50
Wash Bore	32.81	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
SPT	33.31	100	-	-	N=32	-	-	-	-	-	-	-	-	-	-	-	-	
Wash Bore	33.76	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
SPT	34.31	100	-	-	N=35	-	-	-	-	-	-	-	-	-	-	-	-	
Wash Bore	34.76	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
SPT	35.31	100	-	-	N=32	-	-	-	-	-	-	-	-	-	-	-	-	
Wash Bore	35.76	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
SPT	36.31	100	-	-	N=33	-	-	-	-	-	-	-	-	-	-	-	-	
Wash Bore	36.76	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
SPT	37.31	100	-	-	N=35	-	-	-	-	-	-	-	-	-	-	-	-	
Wash Bore	37.76	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
SPT	38.31	100	-	-	N=37	-	-	-	-	-	-	-	-	-	-	-	-	
Wash Bore	38.76	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
SPT	39.31	100	-	-	N=34	-	-	-	-	-	-	-	-	-	-	-	-	
Wash Bore	39.76	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
NWD4	40.31	40	40	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
NWD4	40.81	40	40	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
Wash Bore	41.31	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
SPT	41.81	100	-	-	N=33	-	-	-	-	-	-	-	-	-	-	-	-	
Wash Bore	42.26	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
Bore	42.31	100	-	-	N=35	-	-	-	-	-	-	-	-	-	-	-	-	
SPT	42.76	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
Wash Bore	43.31	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	



HOLE No: BD-BHM206
Sheet 5 of 6

JOB NUMBER: 07-395

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ -slicksided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR-hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose			SR-soft rock
GF -gneissose			VSR-very soft rock
LF -laminated			

JOINT SPACING	JOINT SHAPE
VCJ-very close spacg	CUR-curvilinear
CJ-close spacing	PLA-planar
MJ-medium spacing	UND-undulating
WJ-wide spacing	STE-stepped
VWJ-very wide spacg	IRR-irregular

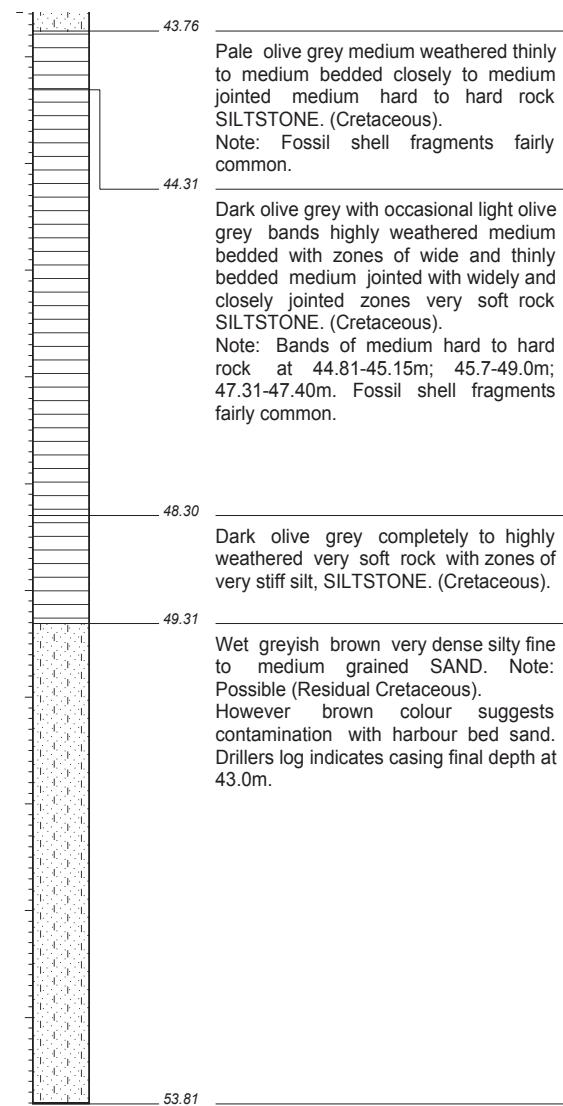


Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM206
Sheet 5 of 6

JOB NUMBER: 07-395

Drill Method and Size	Depth (m)	Material Recovery (%)	Core Recovery (%)	RQD (%)	Test Results	Set No.	Inclination (°)	Spac-ing	Joint shape	Rough-ness	Fill type	0 1 2 3 4 5	DEPTH Scale 1:50		
												Fill thick-ness (mm)	Fracture Frequency	Weathering code	Elevation (m.a.m.s.l.)
SPT	43.76	100	-	-	N=35										
TNW	44.31	100	65	39			1	0-5	C-M	PLA	SRJ	slt	<1	2	
TNW	44.81	100	88	76	UCS=1.154MPa	1	0-5	C-M	PLA	SRJ	slt	<1	3		
TNW	45.31	96	96	28		1	0-5	C-M	PLA	SRJ	slt	<1	6		
TNW	45.81	100	76	50	UCS=1.317MPa	1	0-5	C-M	PLA	SRJ	slt	<1	3		
TNW	47.31	93	93	88	UCS=1.731MPa UCS=1.588MPa	1	0-5	C-W	PLA	SRJ	slt	<1	5		
TNW	48.81	52	39	7		1	0-5	C-M	PLA	SRJ	slt	<1	5		
TNW	49.31	70	60	-		-	-	-	-	-	-	-	-		
Wash Bore	49.81	-	-	-		-	-	-	-	-	-	-	-		
Wash Bore	50.81	-	-	-	N=49	-	-	-	-	-	-	-	-		
Wash Bore	51.26	100	-	-											
Wash Bore	52.31	100	-	-	N=50	-	-	-	-	-	-	-	-		
Wash Bore	52.76	-	-	-		-	-	-	-	-	-	-	-		
Wash Bore	53.81	-	-	-											



HOLE No: BD-BHM206
Sheet 6 of 6

JOB NUMBER: 07-395

ROCK FABRIC
MF -massive
BF -bedded
FF -foliated
CF -cleaved
SF -schistose
GF -gneissose
LF -laminated

GRAIN SIZE
FG -fine grained
MG -medium grain
CG -coarse grain

JOINT ROUGHNESS

SLJ -slickensided
SJ -smooth
RJ -rough

EHR-extremely hard rock
VHR-very hard rock
HR-hard rock
MHR-medium hard rock
SR-soft rock
VSR-very soft rock

JOINT SPACING
VCJ-very close spacg
CJ -close spacing
MJ -medium spacing
WJ -wide spacing
VWJ-very wide spacng

JOINT SHAPE

CUR-curvilinear
PLA-planar
UND-undulating
STE-stepped
IRR-irregular



Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM206
Sheet 6 of 6

JOB NUMBER: 07-395

NOTES

- 1) End of borehole at 53.81m below sea floor.
- 2) Final depth of NW casing at 43.0m.
- 3) Borehole carried out from jack up barge.
- 4) Possible contamination of Cretaceous has occurred with Harbour Bed or fill materials from above.

Drill Method and Size	Depth (m)	Material Recovery (%)	Core Recovery (%)	RQD (%)	Test Results	Set No.	Inclination (°)	Spac-ing	Joint shape	Rough-ness	Fill type	Fill thick-ness (mm)	Fracture Frequency	Weather-ring code	Elevation (m.a.m.s.l.)	DEPTH Scale 1:50
																0
																1
																2
																3
																4
																5

CONTRACTOR : Geopractica

MACHINE :
DRILLED BY : Martin/ Lawrence/ Mike
PROFILED BY : MVR

TYPE SET BY : Rev 0
SETUP FILE : MSJA3.SET

INCLINATION : 90°

DIAM : N
DATE : 30/07/2008
DATE : 12/08/2008

DATE : 24/02/09 15:27
TEXT : ..\BHOLE\BD-BHM~1.TXT

ELEVATION : -12.9 (m) CD
X-COORD : 3306642.005
Y-COORD : -1919.433

HOLE No: BD-BHM206



BD-BHM206

0.0 to 19.52m

BOX 1 of 4



BD-BHM206

19.52 to 32.29m

BOX 2 of 4



BD-BHM206

32.29 to 44.55m

BOX 3 of 4



BD-BHM206

44.55 to 53.81m

BOX 4 of 4

HOLE No: BD-BHM207
Sheet 1 of 5

JOB NUMBER: 07-395

ROCK FABRIC
MF -massive
BF -bedded
FF -foliated
CF -cleaved
SF -schistose
GF -gneissose
LF -laminated

GRAIN SIZE
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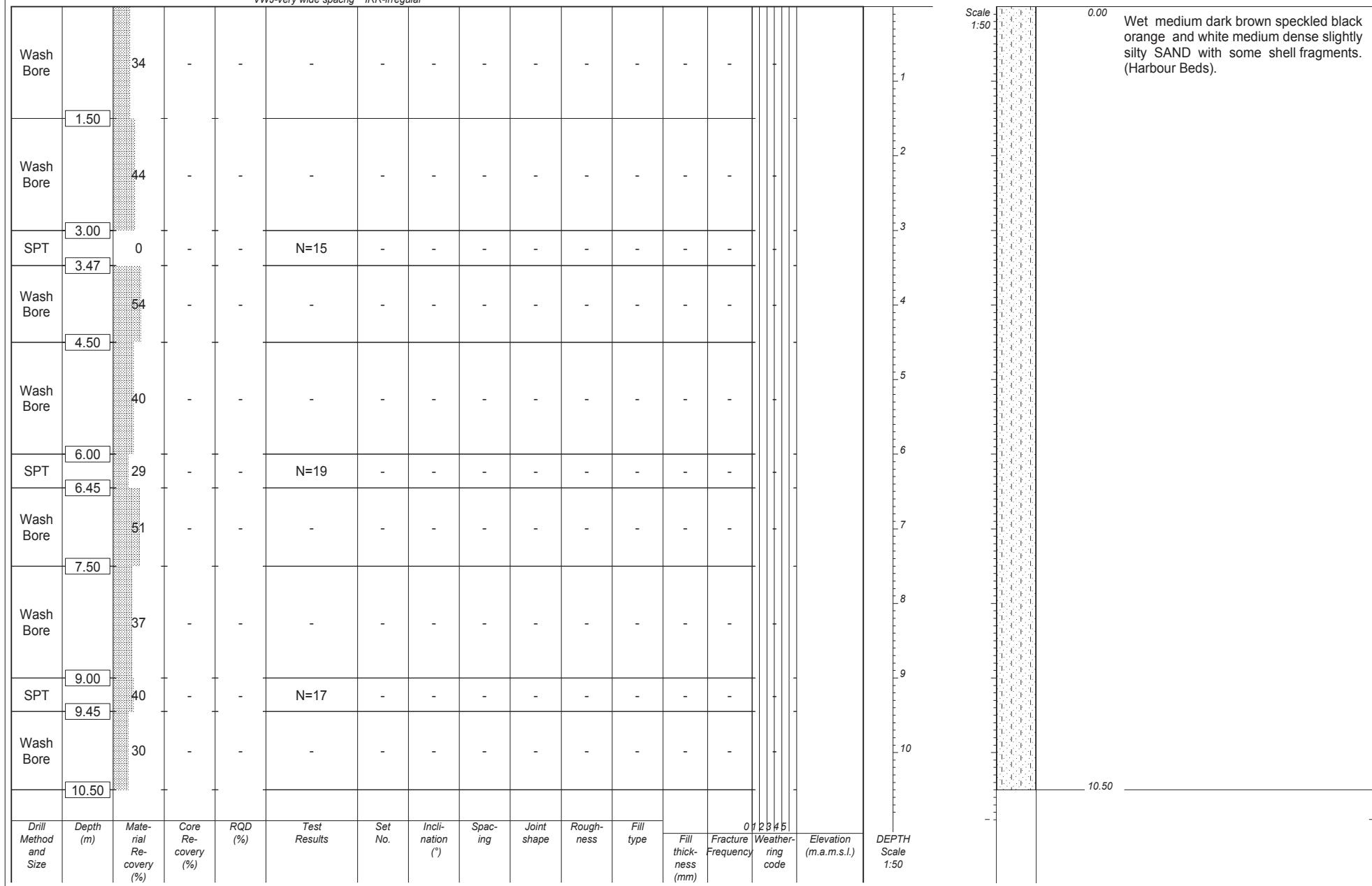
ROCK HARDNESS
EHR-extremely hard rock
VHR-very hard rock
HR-hard rock
MHR-medium hard rock
SR-soft rock
VSR-very soft rock

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Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM207
Sheet 1 of 5

JOB NUMBER: 07-395



HOLE No: BD-BHM207
Sheet 2 of 5

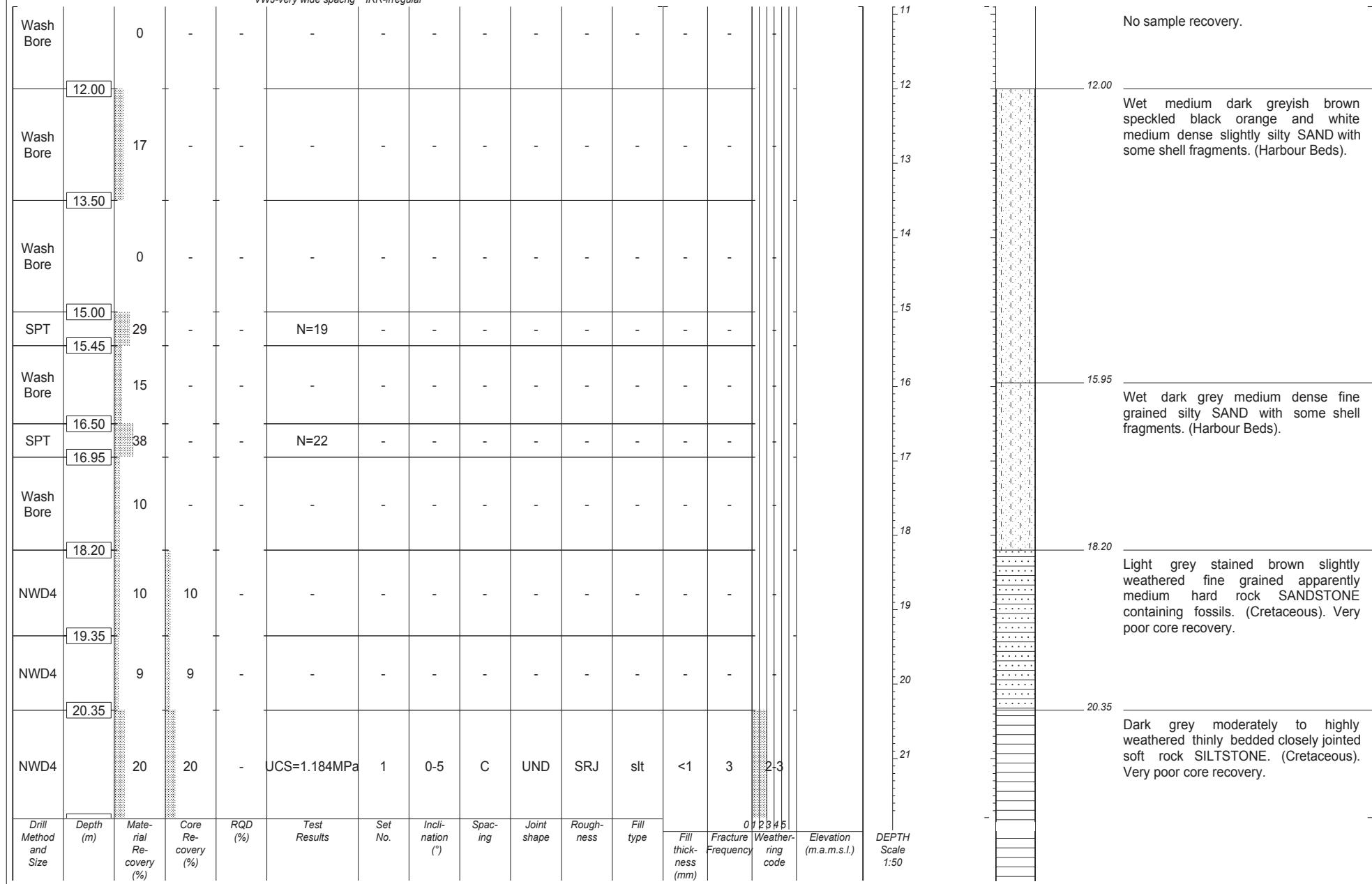
JOB NUMBER: 07-395

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ -slickensided	EHR-extremely hard rock
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GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ-close spacing	PLA-planar	
	MJ-medium spacing	UND-undulating	
	WJ-wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	



Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM207
Sheet 2 of 5
JOB NUMBER: 07-395



HOLE No: BD-BHM207
Sheet 3 of 5

JOB NUMBER: 07-395

ROCK FABRIC
MF -massive
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GRAIN SIZE
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ROCK HARDNESS
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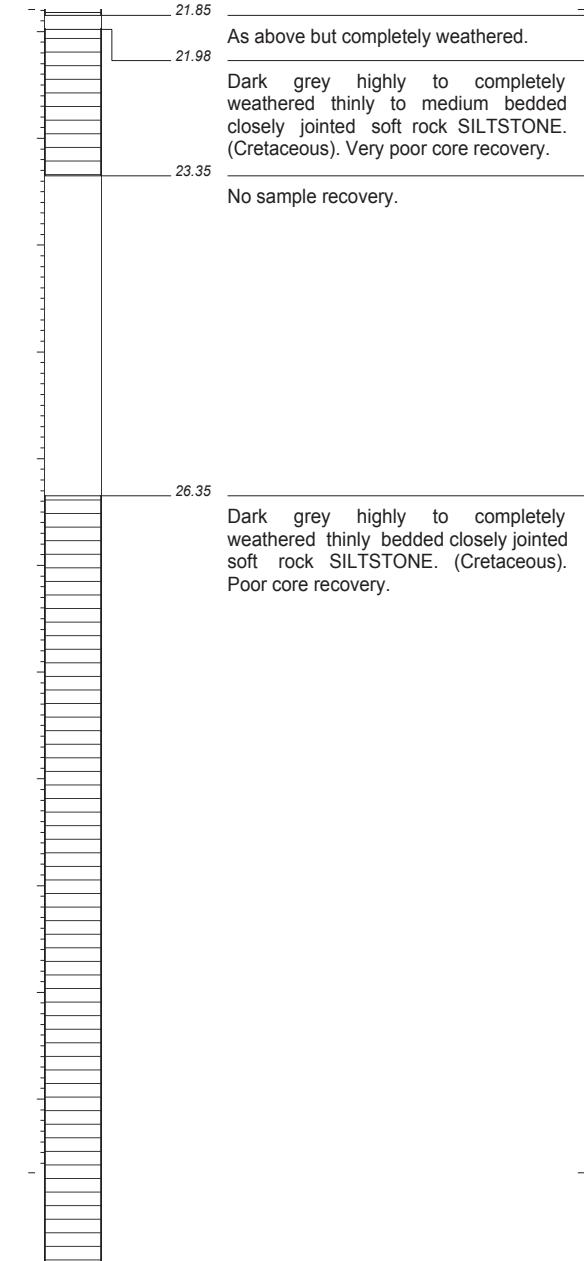


Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM207
Sheet 3 of 5

JOB NUMBER: 07-395

Drill Method and Size	Depth (m)	Material Recovery (%)	Core Recovery (%)	RQD (%)	Test Results	Set No.	Inclination (°)	Spac-ing	Joint shape	Rough-ness	Fill type	Elevation (m.a.m.s.l.)				
												0	1	2	3	4
NWD4	21.85															
	29	20														
NWD4	23.35															
Wash Bore	0	0	0													
Wash Bore	24.85															
NWD4	26.35															
	47	47														
NWD4	27.87															
	23	23														
NWD4	29.19															
	23	23														
NWD4	30.69					UCS=1.331MPa										
	36	36	11			1	0-10	C	UND	SRJ	silt	<1	6	3-4		
NWD4	32.19															
	76	76														
						UCS=1.249MPa	1	0-5	C	PLA	SRJ	silt	<1	1	0	1



HOLE No: BD-BHM207
Sheet 4 of 5

JOB NUMBER: 07-395

ROCK FABRIC
MF -massive
BF -bedded
FF -foliated
CF -cleaved
SF -schistose
GF -gneissose
LF -laminated

GRAIN SIZE
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JOINT ROUGHNESS
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SJ -smooth
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ROCK HARDNESS
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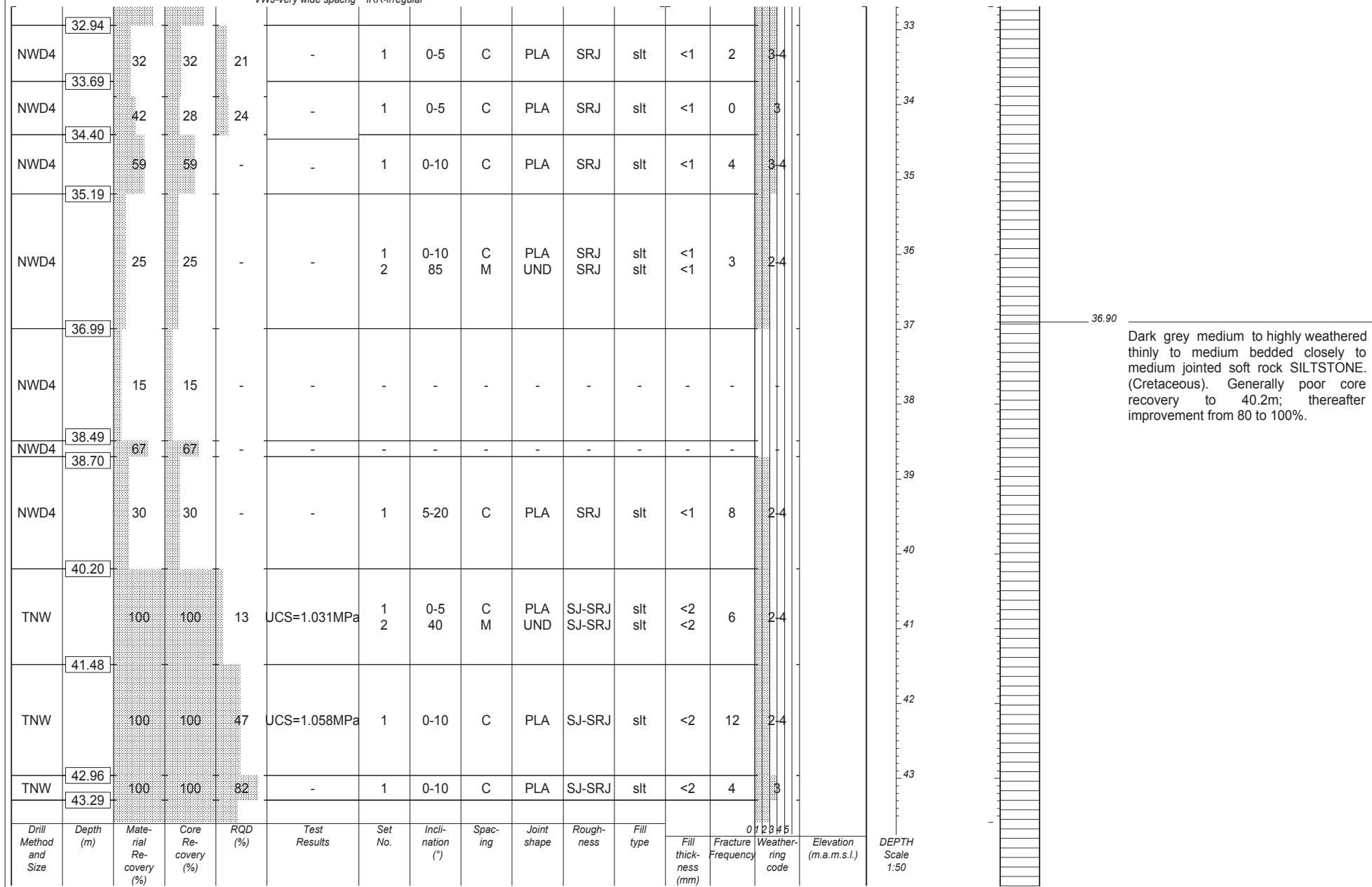
JOINT SHAPE
CUR-curvilinear
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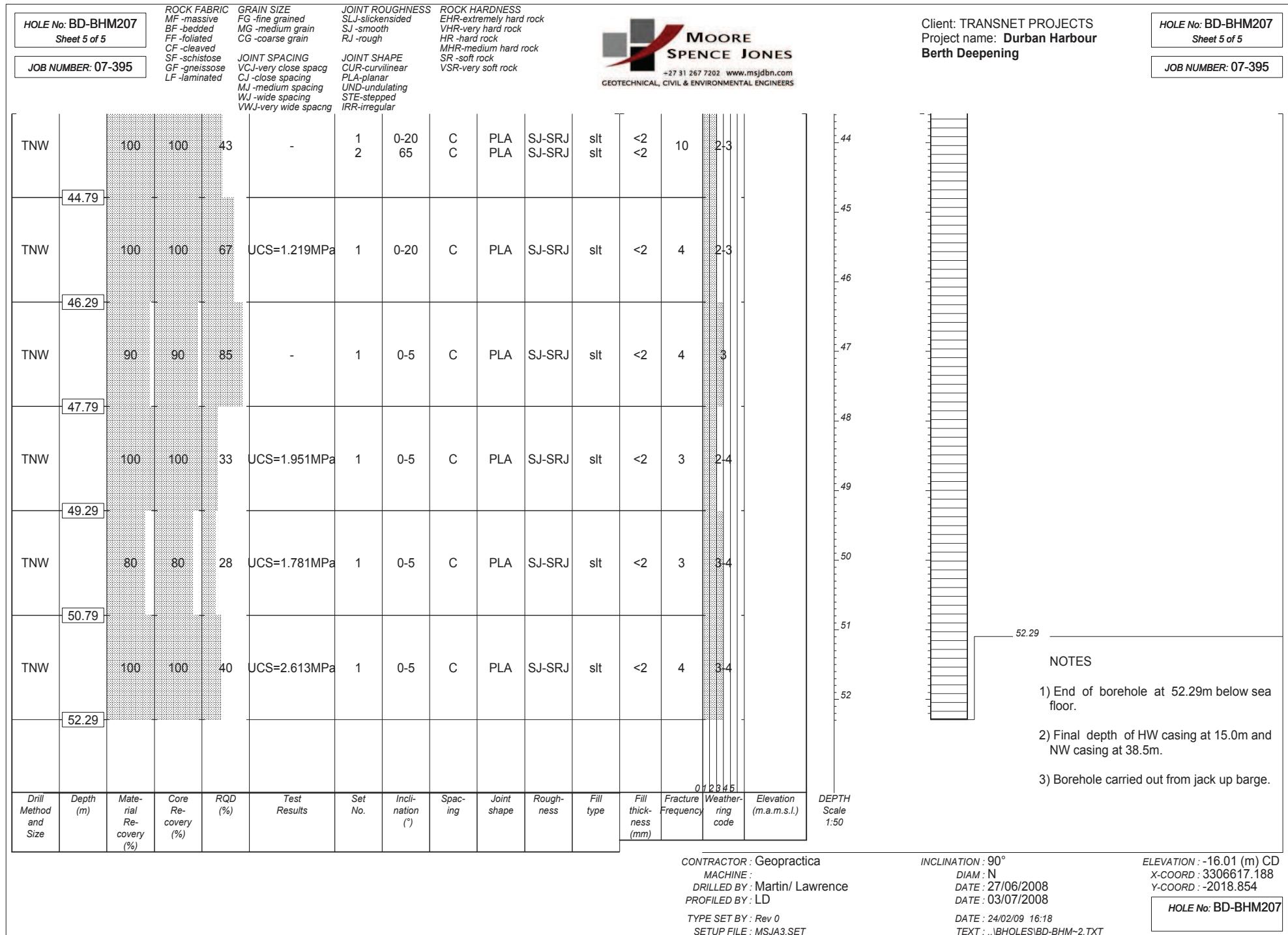


Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM207
Sheet 4 of 5

JOB NUMBER: 07-395







BD-BHM207

0.0 to 23.35m

BOX 1 of 4



BD-BHM207

23.35 to 41.48m

BOX 2 of 4



BD-BHM207

41.48 to 47.79m

BOX 3 of 4



BD-BHM207

47.79 to 52.29m

BOX 4 of 4

HOLE No: BD-BHM208
Sheet 1 of 3

JOB NUMBER: 07-395

ROCK FABRIC
MF -massive
BF -bedded
FF -foliated
CF -cleaved
SF -schistose
GF -gneissose
LF -laminated

GRAIN SIZE
FG -fine grained
MG -medium grain
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JOINT ROUGHNESS
SLJ -slickensided
SJ -smooth
RJ -rough

JOINT SPACING

VCJ -very close spacg
CJ -close spacing
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JOINT SHAPE

CUR -curvilinear

PLA -planar

UND -undulating

STE -stepped

VSR -very soft rock

ROCK HARDNESS
EHR-extremely hard rock
VHR-very hard rock
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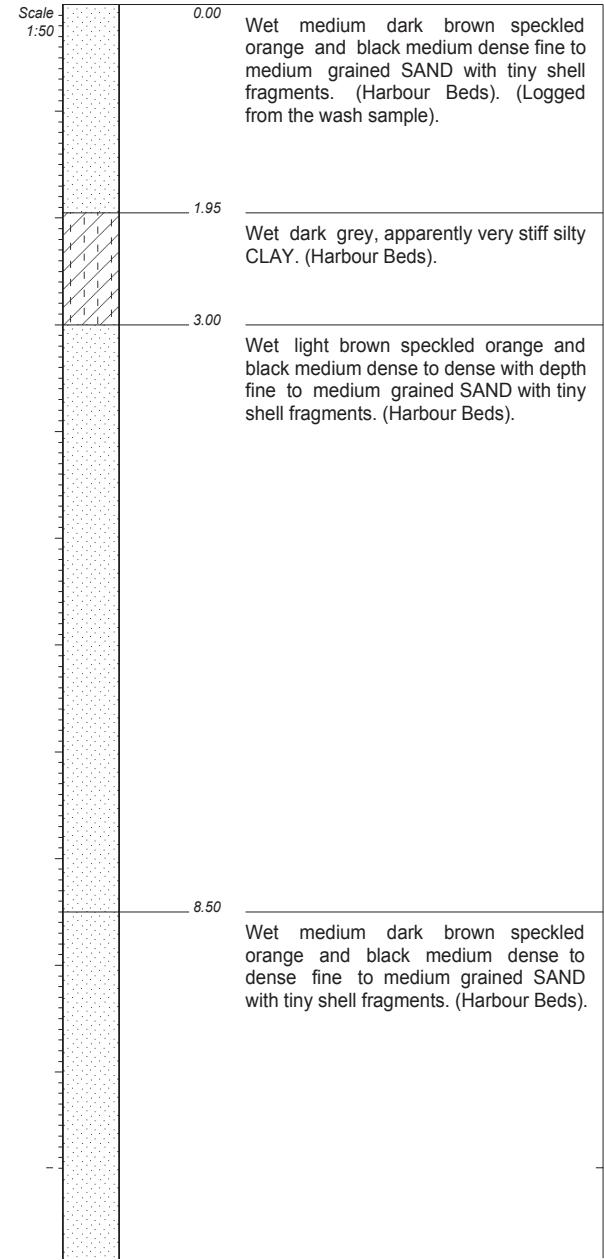
GEOTECHNICAL, CIVIL & ENVIRONMENTAL ENGINEERS

Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM208
Sheet 1 of 3

JOB NUMBER: 07-395

Drill Method and Size	Depth (m)	Material Recovery (%)	Core Recovery (%)	RQD (%)	Test Results	Set No.	Inclination (°)	Spac-ing	Joint shape	Rough-ness	Fill type	0 1 2 3 4 5	Fill thick-ness (mm)	Fracture Frequency	Weather-ing code	Elevation (m.a.m.s.l.)	DEPTH Scale 1:50
Wash Bore																	
SPT	1.50				28	-	-	-	-	-	-	-	-	-	-		
	1.95				60	-	-	N=13	-	-	-	-	-	-	-		
NWD4					29	-	-	-	-	-	-	-	-	-	-		
SPT	3.00				53	-	-	N=15	-	-	-	-	-	-	-		
	3.45				0	-	-		-	-	-	-	-	-	-		
Wash Bore					0	-	-		-	-	-	-	-	-	-		
Shelby	4.50				0	-	-	Shelby	-	-	-	-	-	-	-		
	5.05				21	-	-		-	-	-	-	-	-	-		
Wash Bore					57	-	-		-	-	-	-	-	-	-		
SPT	6.00				57	-	-	N=24	-	-	-	-	-	-	-		
	6.45				24	-	-		-	-	-	-	-	-	-		
Wash Bore					51	-	-		-	-	-	-	-	-	-		
SPT	7.50				51	-	-	N=32	-	-	-	-	-	-	-		
	7.95				21	-	-		-	-	-	-	-	-	-		
Wash Bore					60	-	-		-	-	-	-	-	-	-		
SPT	9.00				60	-	-	N=29	-	-	-	-	-	-	-		
	9.45				19	-	-		-	-	-	-	-	-	-		
Wash Bore					10.50	-	-		-	-	-	-	-	-	-		
Shelby					0	-	-	Shelby	-	-	-	-	-	-	-		



HOLE No: BD-BHM208
Sheet 2 of 3

JOB NUMBER: 07-395

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slicksided	EHR-extremely hard rock
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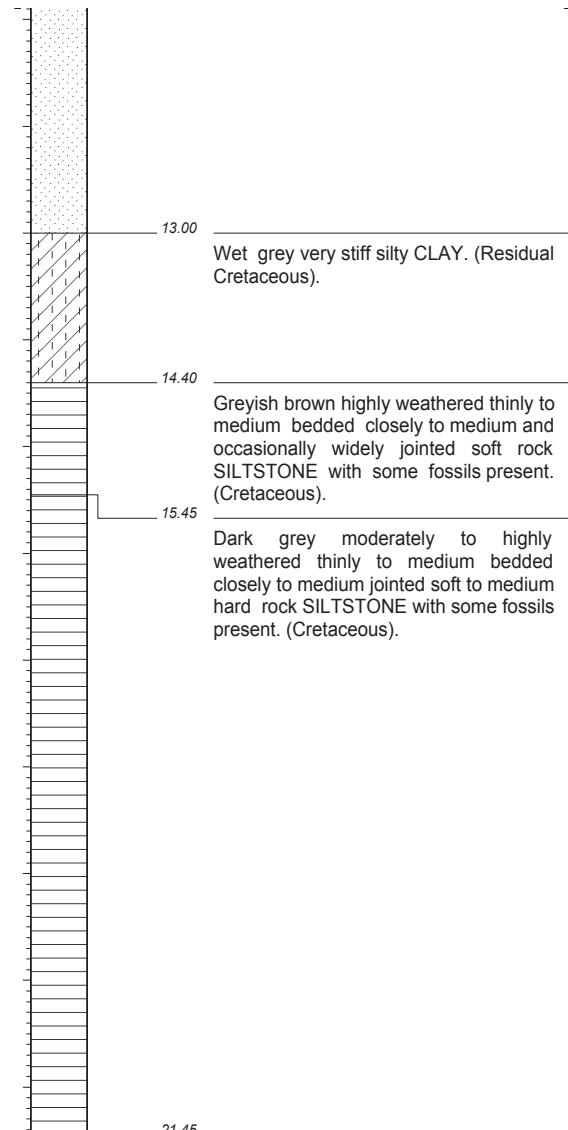


Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM208
Sheet 2 of 3

JOB NUMBER: 07-395

Drill Method and Size	Depth (m)	Material Recovery (%)	Core Recovery (%)	RQD (%)	Test Results	Set No.	Inclination (°)	Spac-ing	Joint shape	Rough-ness	Fill type	0	1	2	3	4	5	Elevation (m.a.m.s.l.)	DEPTH Scale 1:50
												Fill thick-ness (mm)	Fracture Frequency	Weathering code					
	11.05																		
Wash Bore																			
	12.00																		
SPT																			
	12.45																		
Wash Bore																			
	13.50																		
SPT																			
	13.95																		
NWD4																			
	97	86		50		UCS=1.9MPa	1 2	0-10 70	C-M W	PLA-UNDSJ-SRJ	slt cl	<1 <1	14	3.4					
NWD4																			
	15.45																		
NWD4																			
	80	80		27		UCS=2.5MPa	1	0-10	C-M	PLA-UNDSJ-SRJ	slt	<1	16	2.3					
NWD4																			
	16.95																		
NWD4																			
	85	85		15		UCS=2.4MPa	1	0-10	C-M	PLA-UNDSJ-SRJ	slt	<1	19	2.3					
NWD4																			
	18.45																		
NWD4																			
	97	97		27			1	0-10	C-M	PLA-UNDSJ-SRJ	slt	<1	14	3.4					
NWD4																			
	19.95																		
NWD4																			
	98	98		47		UCS=2.2MPa	1	0-5	C-M	PLA-UNDSJ-SRJ	slt	<1	15	2.3					
NWD4																			
	21.45																		



HOLE No: BD-BHM208
Sheet 3 of 3

JOB NUMBER: 07-395

ROCK FABRIC
MF -massive
BF -bedded
FF -foliated
CF -cleaved
SF -schistose
GF -gneissose
LF -laminated

GRAIN SIZE
FG -fine grained
MG -medium grain
CG -coarse grain

JOINT ROUGHNESS

SLJ -slickensided
SJ -smooth
RJ -rough
EHR -extremely hard rock
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CJ -close spacing
MJ -medium spacing
WJ -wide spacing
VWJ -very wide spacng

JOINT SHAPE

PLA -planar
UND -undulating
STE -stepped
IRR -irregular



Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM208
Sheet 3 of 3

JOB NUMBER: 07-395

Drill Method and Size	Depth (m)	Material Recovery (%)	Core Recovery (%)	RQD (%)	Test Results	Set No.	Inclination (°)	Spac-ing	Joint shape	Rough-ness	Fill type	Fill thick-ness (mm)	Fracture Frequency	Weather-ring code	Elevation (m.a.m.s.l.)	DEPTH Scale 1:50
																0
																1
																2
																3
																4
																5

CONTRACTOR : Geopractica

MACHINE :
DRILLED BY : PM
PROFILED BY : LD

TYPE SET BY : Rev 0
SETUP FILE : MSJA3.SET

INCLINATION : 90°

DIAM : N
DATE : 26/06/2008
DATE : 02/07/2008

DATE : 24/02/09 16:16
TEXT : ..\BHOLE\BD-BHM~3.TXT

ELEVATION : -13.118 (m) CD
X-COORD : 3306605.55
Y-COORD : -2096.114

HOLE No: BD-BHM208



BD-BHM208

0.0 to 15.45m

BOX 1 of 2



BD-BHM208

15.45 to 21.45m

BOX 2 of 2

HOLE No: BD-BHM209
Sheet 1 of 6

JOB NUMBER: 07-395

ROCK FABRIC
MF -massive
BF -bedded
FF -foliated
CF -cleaved
SF -schistose
GF -gneissose
LF -laminated

GRAIN SIZE
FG -fine grained
MG -medium grain
CG -coarse grain

JOINT SPACING
VCJ-very close spacg
CJ-close spacing
MJ-medium spacing
WJ-wide spacing
VWJ-very wide spacng

JOINT ROUGHNESS
SLJ-slickensided
SJ-smooth
RJ-rough

JOINT SHAPE
PLA-planar
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STE-stepped
IRR-irregular

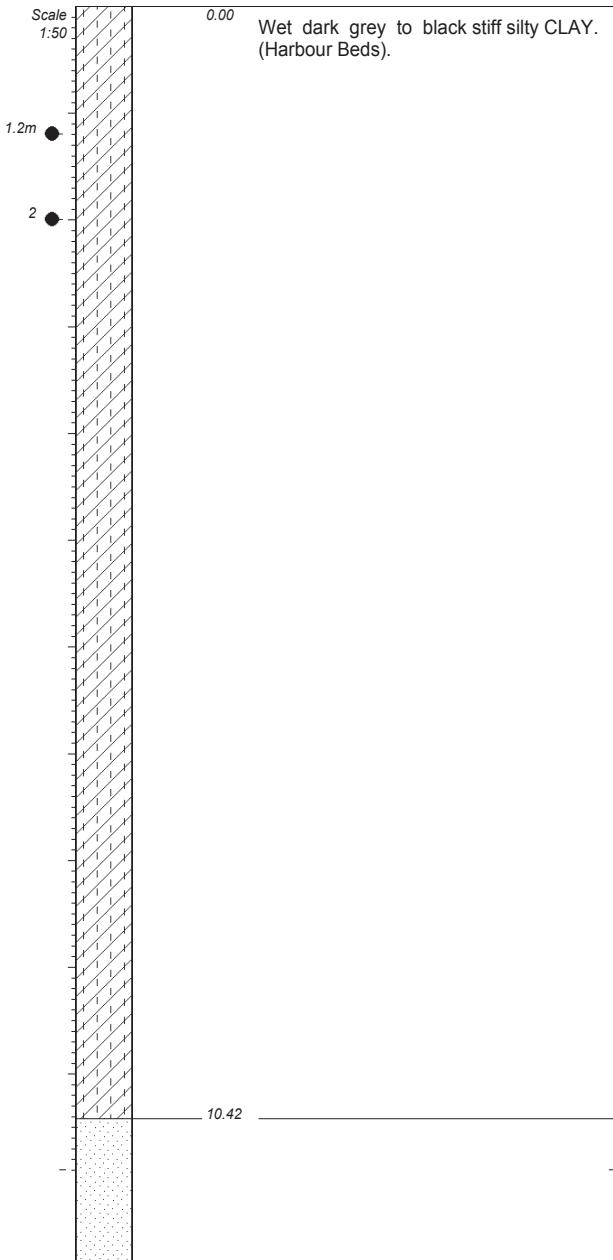
ROCK HARDNESS
EHR-extremely hard rock
VHR-very hard rock
HR-hard rock
MHR-medium hard rock
SR-soft rock
VSR-very soft rock



Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM209
Sheet 1 of 6
JOB NUMBER: 07-395

Drill Method and Size	Depth (m)	Material Recovery (%)	Core Recovery (%)	RQD (%)	Test Results	Set No.	Inclination (°)	Spac-ing	Joint shape	Rough-ness	Fill type	0	1	2	3	4	5	Elevation (m.a.m.s.l.)	DEPTH Scale 1:50
Wash Bore																			
	1.00																		
HW																			
SPT	2.50					N=16													
	2.95																		
HW																			
HW	4.00																		
Shelby	4.50					Shelby													
	4.97																		
SPT	5.42					N=19													
HW																			
	6.91																		
HW																			
	8.47																		
HW																			
	9.97																		
SPT	10.42					N=18													



HOLE No: BD-BHM209
Sheet 2 of 6

JOB NUMBER: 07-395

ROCK FABRIC
MF -massive
BF -bedded
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GRAIN SIZE
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CG -coarse grain

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VSR-very soft rock

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STE-stepped
IRR-irregular

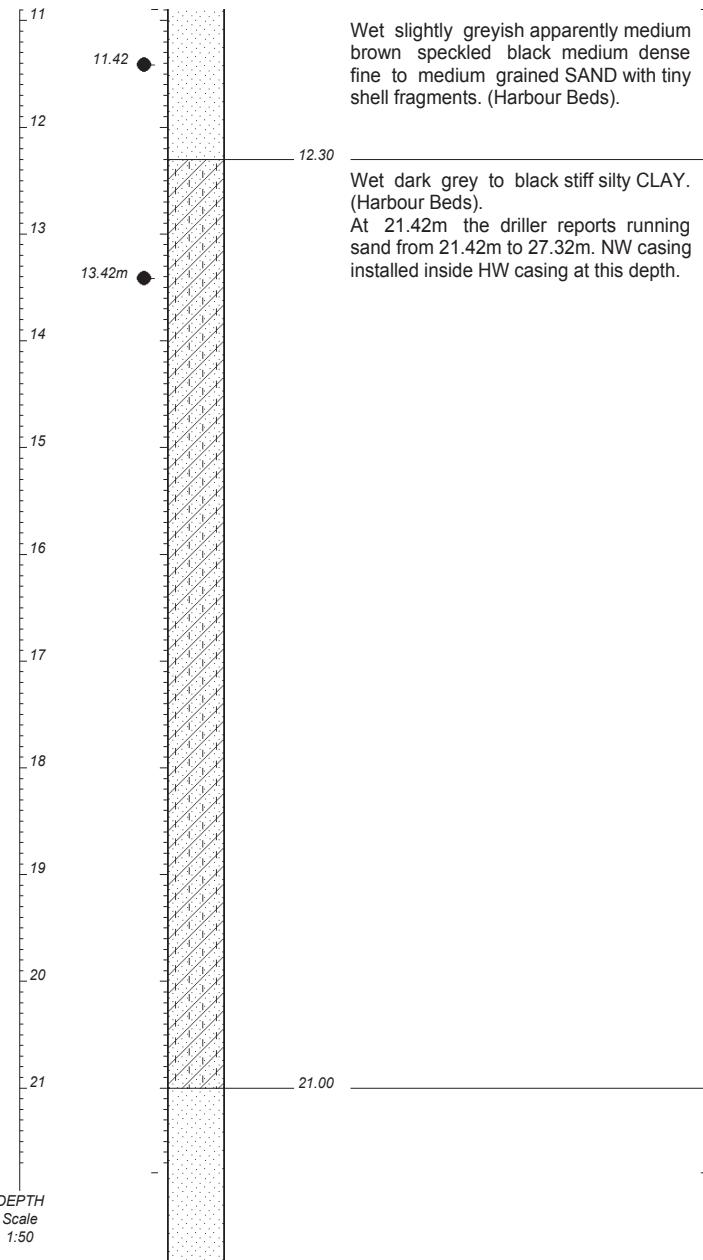


Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM209
Sheet 2 of 6

JOB NUMBER: 07-395

Drill Method and Size	Depth (m)	Material Recovery (%)	Core Recovery (%)	RQD (%)	Test Results	Set No.	Inclination (°)	Spac-ing	Joint shape	Rough-ness	Fill type	0 1 2 3 4 5	Fill thick-ness (mm)	Fracture Frequency	Weathering code	Elevation (m.a.m.s.l.)
HW	11.42															
HW	13.42															
SPT	13.87				N=32											
HW	15.37															
HW	15.87															
HW	17.37															
HW	18.87															
SPT	19.32				N=20											
HW	20.37															
Wash Bore	21.42															



HOLE No: BD-BHM209
Sheet 3 of 6

JOB NUMBER: 07-395

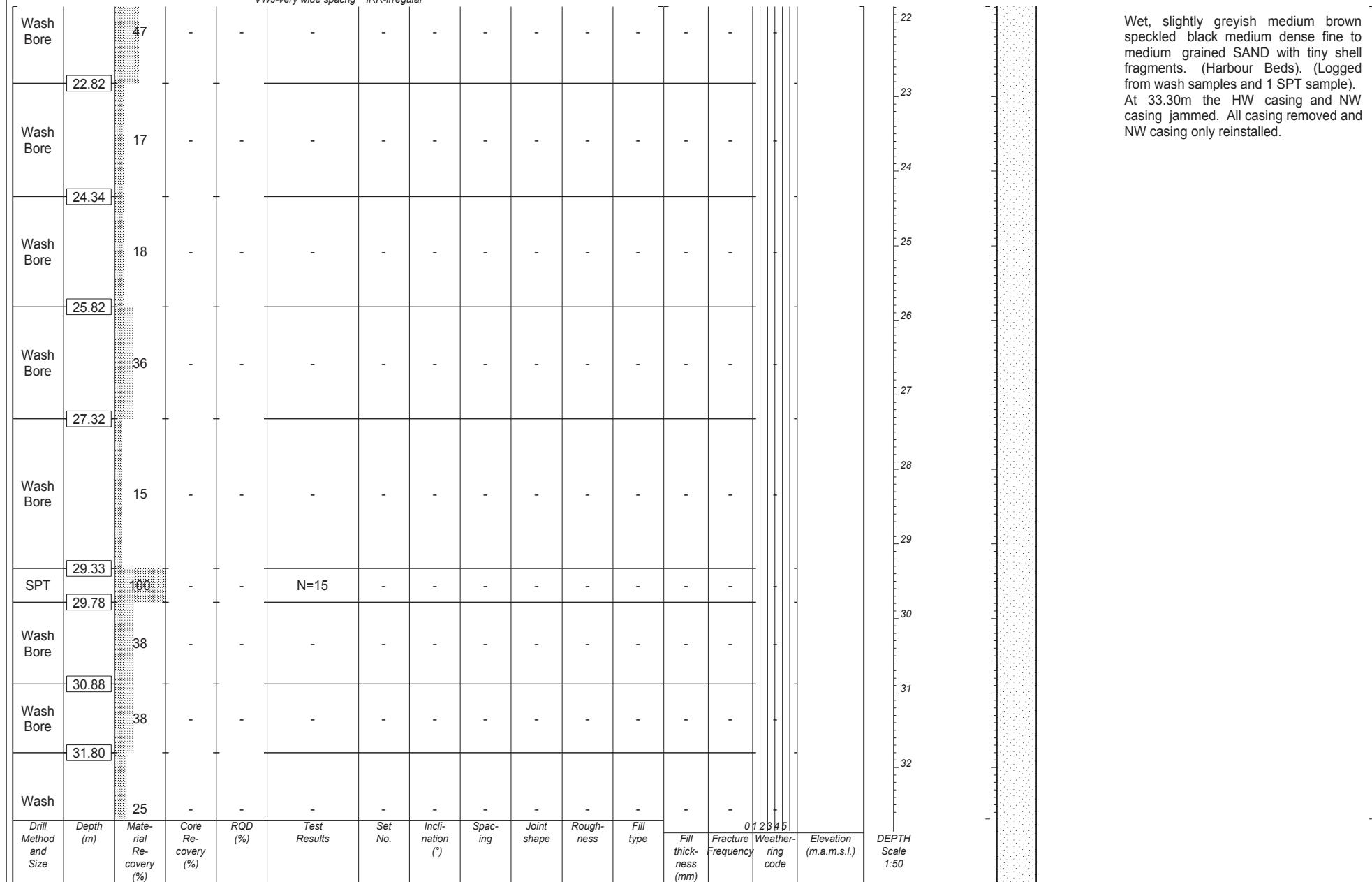
ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ -slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR-hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose			SR-soft rock
GF -gneissose	VCJ-very close spacing	JOINT SPACING	VSR-very soft rock
LF -laminated	CJ-close spacing	PLA-planar	
	MJ-medium spacing	UND-undulating	
	WJ-wide spacing	STE-stepped	
	VWJ-very wide spacing	IRR-irregular	



Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM209
Sheet 3 of 6

JOB NUMBER: 07-395



HOLE No: BD-BHM209
Sheet 4 of 6

JOB NUMBER: 07-395

ROCK FABRIC
MF -massive
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LF -laminated

GRAIN SIZE
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CJ -close spacing
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WJ -wide spacing
VWJ-very wide spacng

JOINT ROUGHNESS
SLJ-slickensided
SJ -smooth
RJ -rough

JOINT SHAPE
CUR-curvilinear
PLA-planar
UND-undulating
STE-stepped
IRR-irregular

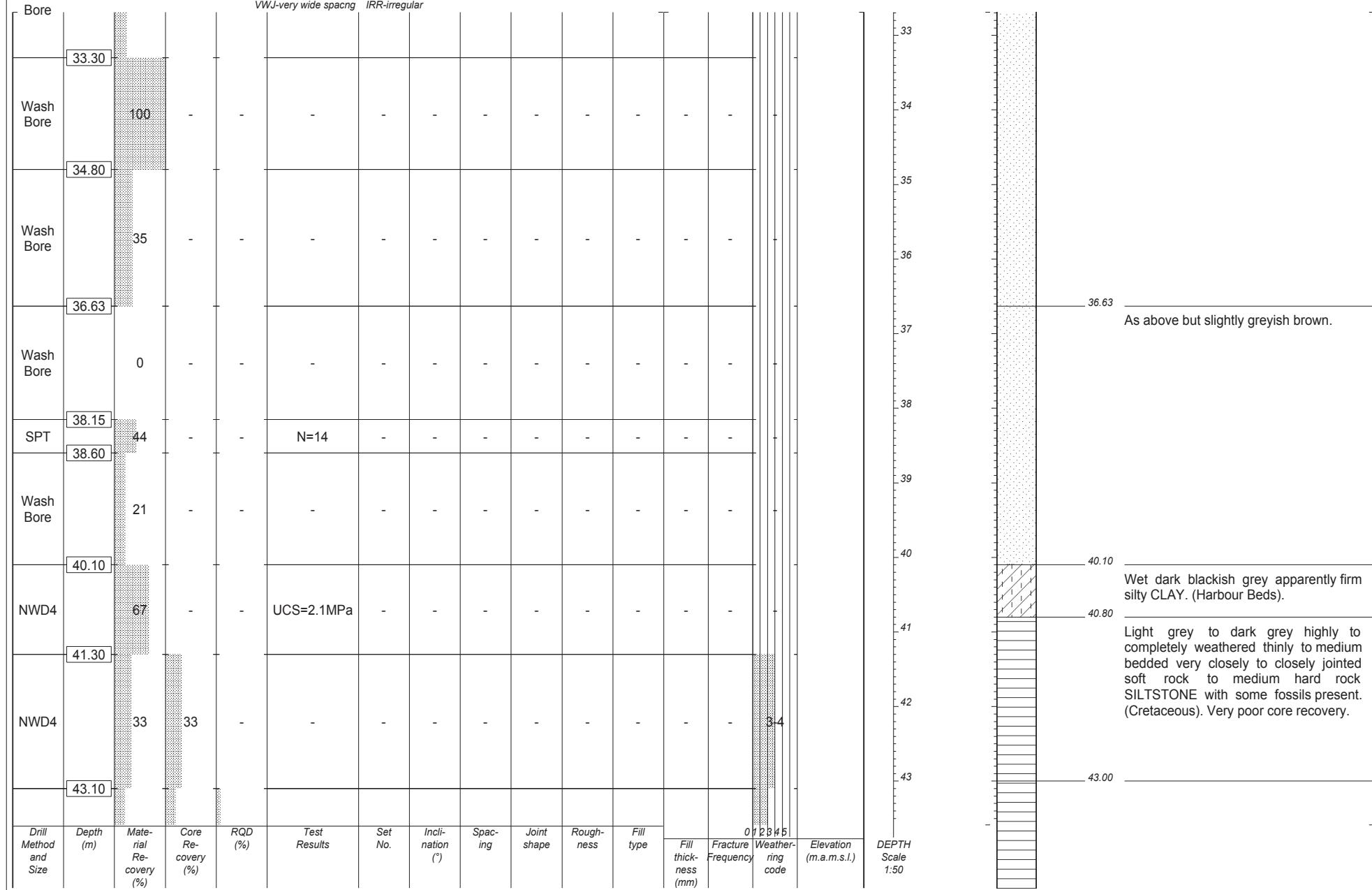
ROCK HARDNESS
EHR-extremely hard rock
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HR-hard rock
MHR-medium hard rock
SR-soft rock
VSR-very soft rock

**MOORE
SPENCE JONES**
+27 31 267 7202 www.msjdbn.com
GEOTECHNICAL, CIVIL & ENVIRONMENTAL ENGINEERS

Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM209
Sheet 4 of 6

JOB NUMBER: 07-395



HOLE No: BD-BHM209
Sheet 5 of 6

JOB NUMBER: 07-395

ROCK FABRIC
MF -massive
BF -bedded
FF -foliated
CF -cleaved
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GF -gneissose
LF -laminated

GRAIN SIZE
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JOINT ROUGHNESS
SLJ -slickensided
SJ -smooth
RJ -rough

JOINT SPACING
VCJ -very close spacing
CJ -close spacing
MJ -medium spacing
WJ -wide spacing
VWJ -very wide spacing

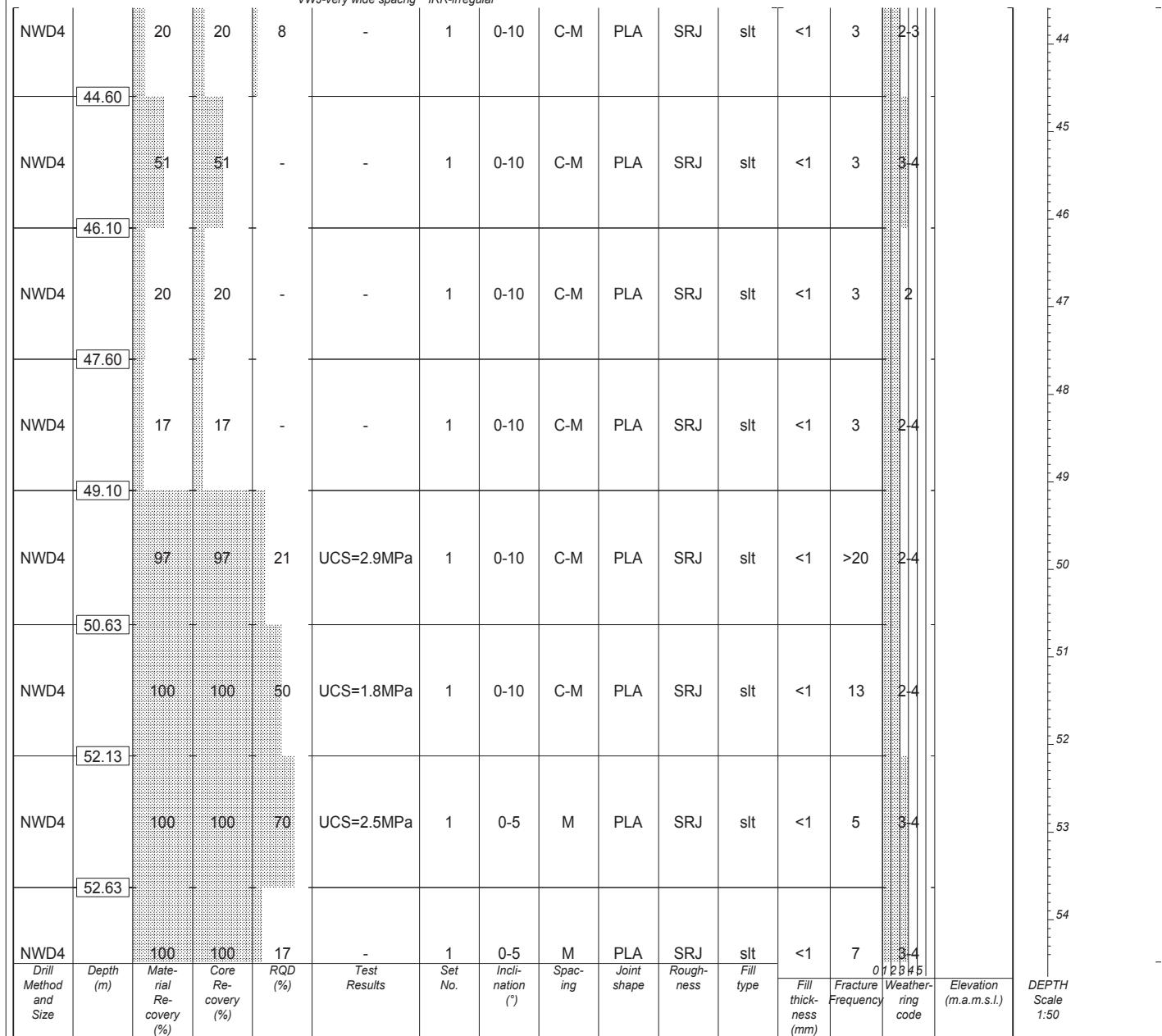
JOINT SHAPE
CUR -curvilinear
PLA -planar
UND -undulating
STE -stepped
IRR -irregular



Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM209
Sheet 5 of 6

JOB NUMBER: 07-395



Light grey to olive, medium to highly weathered thinly to medium bedded closely to medium jointed very soft rock SILTSTONE with fossils. (Cretaceous). (Note: very poor core recovery to 49.10m).

HOLE No: BD-BHM209
Sheet 6 of 6
JOB NUMBER: 07-395

ROCK FABRIC
MF -massive
BF -bedded
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GF -gneissose
LF -laminated

GRAIN SIZE
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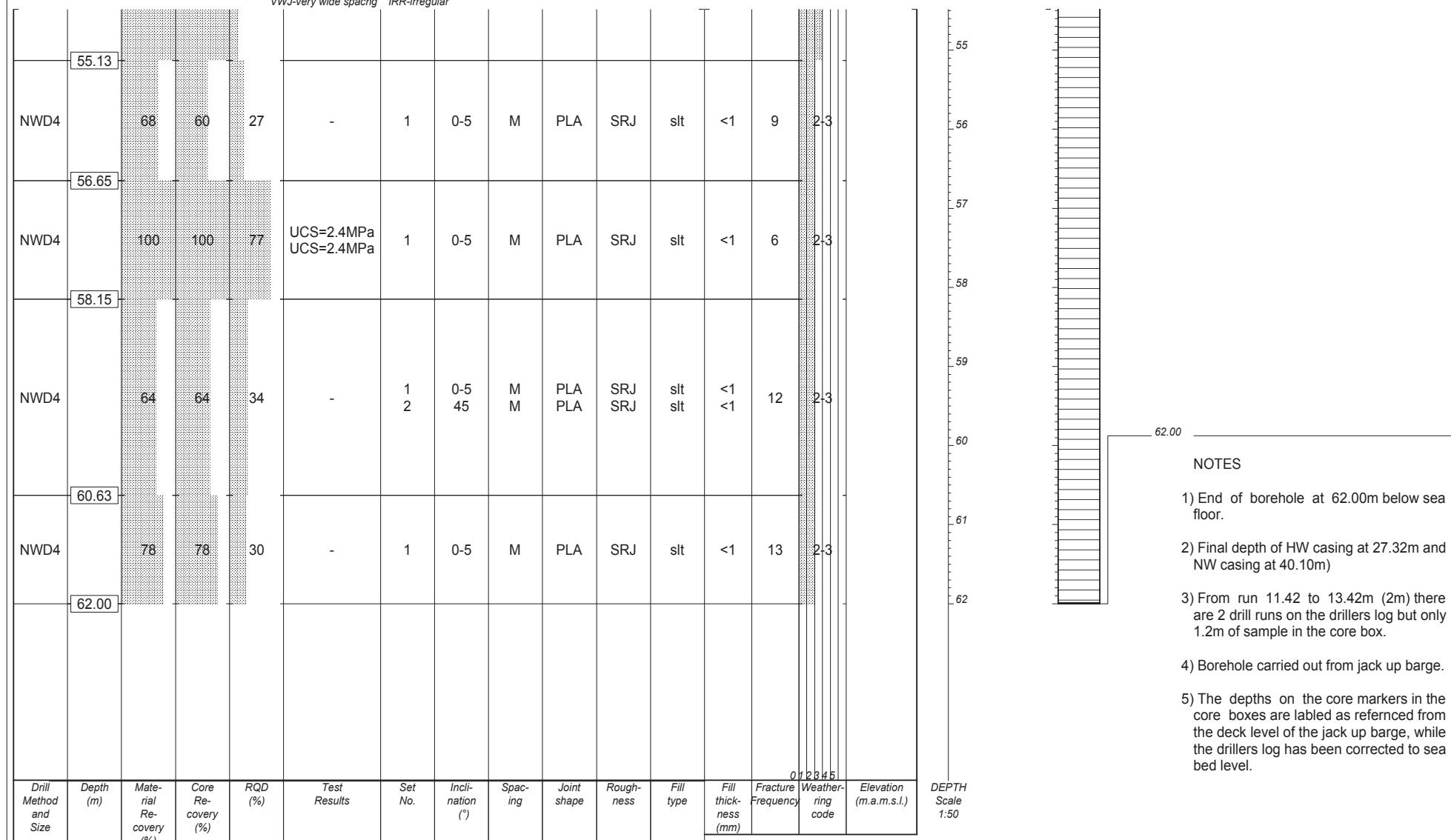
JOINT SHAPE
CUR-curvilinear
PLA-planar
UND-undulating
STE-stepped
IRR-irregular

ROCK HARDNESS
EHR-extremely hard rock
VHR-very hard rock
HR-hard rock
MHR-medium hard rock
SR-soft rock
VSR-very soft rock



Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM209
Sheet 6 of 6
JOB NUMBER: 07-395



CONTRACTOR : Geopractica

MACHINE :
DRILLED BY : Martin / Mike / Daniel
PROFILED BY : SAP

TYPE SET BY : Rev 0
SETUP FILE : MSJA3.SET

INCLINATION : 90°

DIAM : N
DATE : 16/06/2008
DATE : 04/07/2008

DATE : 24/02/09 15:28
TEXT : ..\BHOLE\BD-BHM~4.TXT

ELEVATION : -13.01 (m) CD
X-COORD : 3306577.189
Y-COORD : -2218.109

HOLE No: BD-BHM209



BD-BHM209

18.00 to 30.30m

BOX 1 of 5

Core markers in the core box indicate that the drilling started from the top of the jackup barge at 18.0m and not from sea bed level at 0m.



BD-BHM209

30.30 to 40.82m

BOX 2 of 5



BD-BHM209

40.82 to 58.80m

BOX 3 of 5



BD-BHM209

58.80 to 70.84m

BOX 4 of 5



BD-BHM209

70.84 to 80.00m

BOX 5 of 5

HOLE No: BD-BHM210A
Sheet 1 of 7

JOB NUMBER: 07-395

ROCK FABRIC
MF -massive
BF -bedded
FF -foliated
CF -cleaved
SF -schistose
GF -gneissose
LF -laminated

GRAIN SIZE
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MG -medium grain
CG -coarse grain

JOINT SPACING
VCJ-very close spacg
CJ -close spacing
MJ -medium spacing
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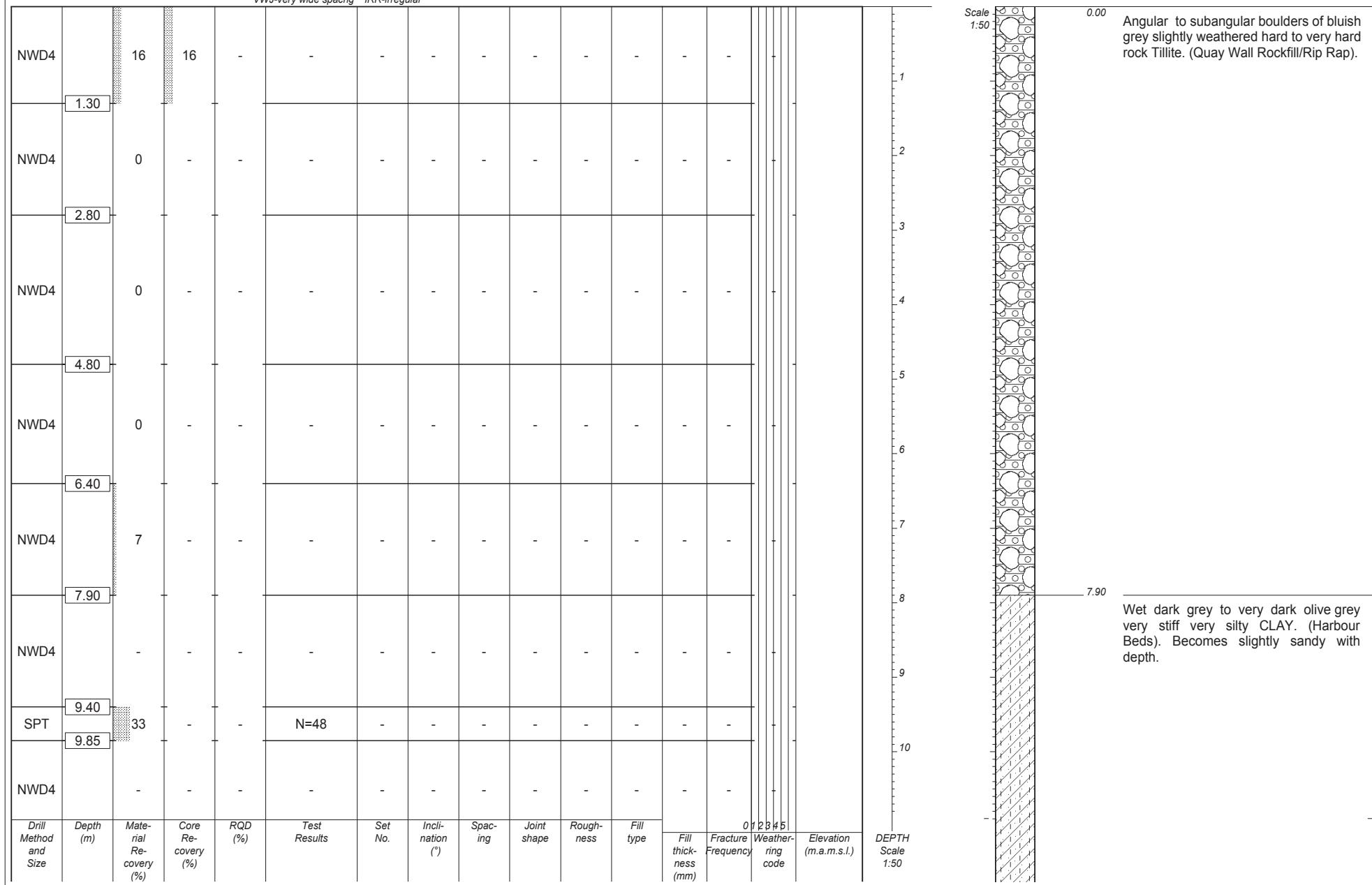
JOINT ROUGHNESS
SLJ-slickensided
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RJ -rough

ROCK HARDNESS
EHR-extremely hard rock
VHR-very hard rock
HR-hard rock
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SR-soft rock
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Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM210A
Sheet 1 of 7
JOB NUMBER: 07-395



HOLE No: BD-BHM210A
Sheet 2 of 7

JOB NUMBER: 07-395

ROCK FABRIC
MF -massive
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SJ -smooth
RJ -rough

JOINT SPACING
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CJ -close spacing
MJ -medium spacing
WJ -wide spacing
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JOINT SHAPE
PLA-planar
UND-undulating
STE-stepped
IRR-irregular

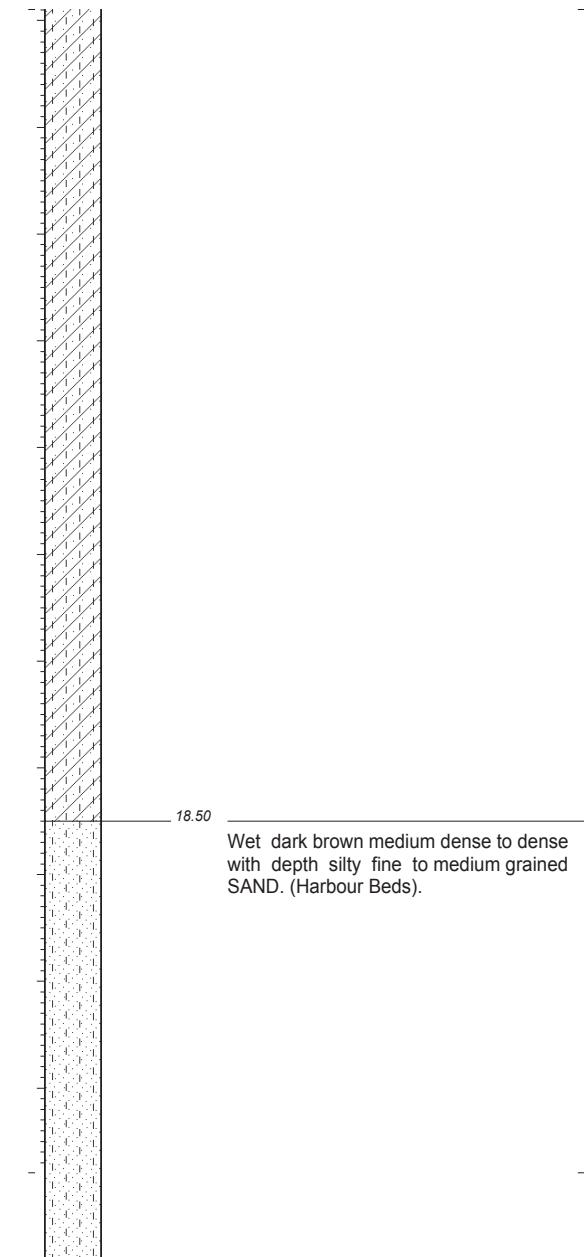


Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM210A
Sheet 2 of 7

JOB NUMBER: 07-395

Drill Method and Size	Depth (m)	Material Recovery (%)	Core Recovery (%)	RQD (%)	Test Results	Set No.	Inclination (°)	Spac-ing	Joint shape	Rough-ness	Fill type	0	1	2	3	4	5	Elevation (m.a.m.s.l.)
												Fill thick-ness (mm)	Fracture Frequency	Weather-ing code				DEPTH Scale 1:50
SPT	11.15																	
Shelby	11.60																	
NWD4	11.96																	
Solid Tube	12.57																	
	14.07																	
NWD4																		
NWD4	15.57																	
NWD4	16.07																	
NWD4	16.57																	
NWD4	17.07																	
NWD4	18.07																	
Wash Bore																		
SPT	19.65																	
Wash Bore	20.10																	
SPT	21.15																	
	21.60																	



HOLE No: BD-BHM210A
Sheet 3 of 7

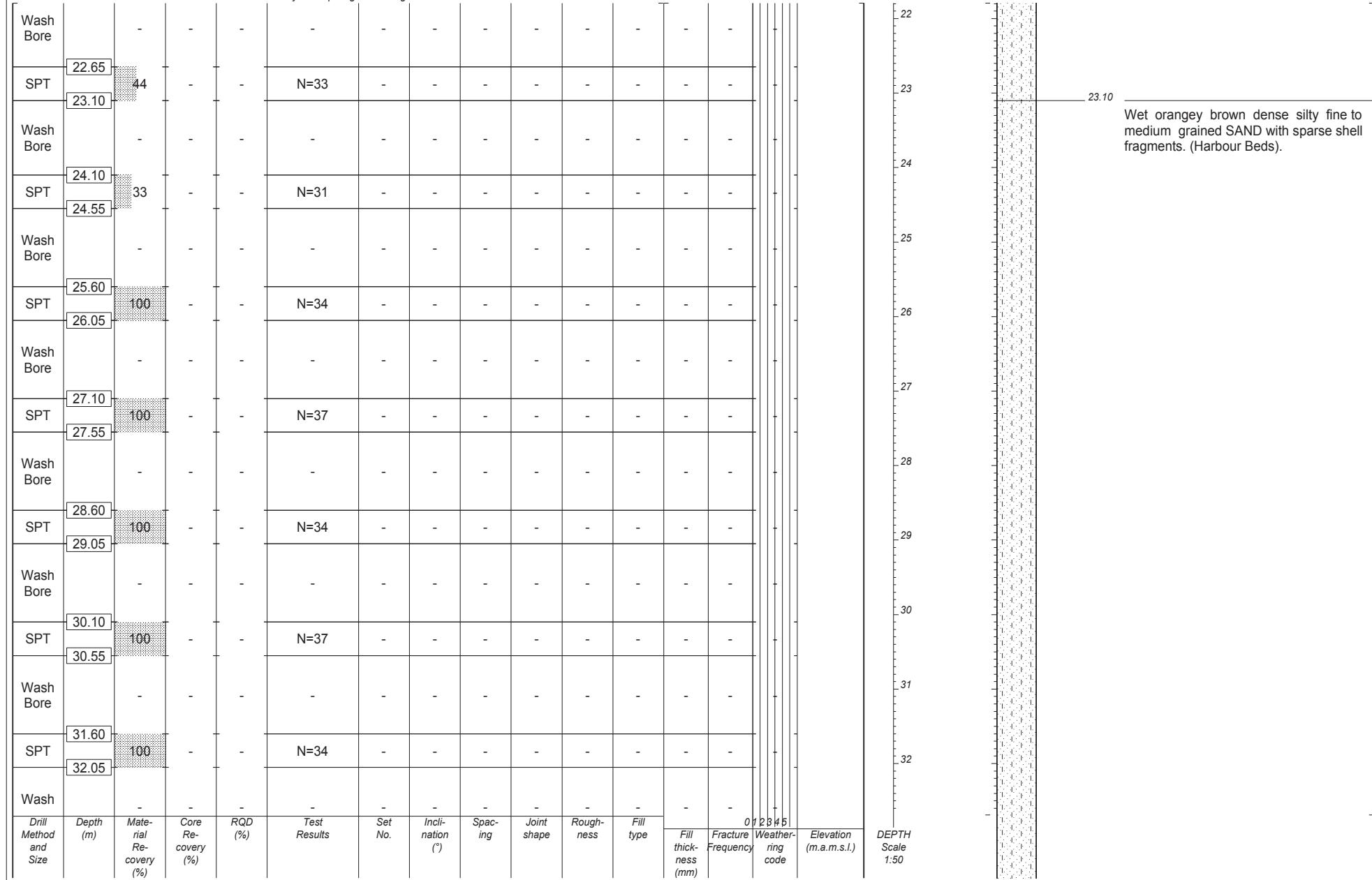
JOB NUMBER: 07-395

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
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GF -gneissose			VSR-very soft rock
LF -laminated			
	JOINT SPACING	JOINT SHAPE	
	VCJ-very close spacg	CJA-planar	
	CJ-close spacing	UND-undulating	
	MJ-medium spacing	STE-stepped	
	WJ-wide spacing	IRR-irregular	
	VWJ-very wide spacng		



Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM210A
Sheet 3 of 7
JOB NUMBER: 07-395



HOLE No: BD-BHM210A
Sheet 4 of 7

JOB NUMBER: 07-395

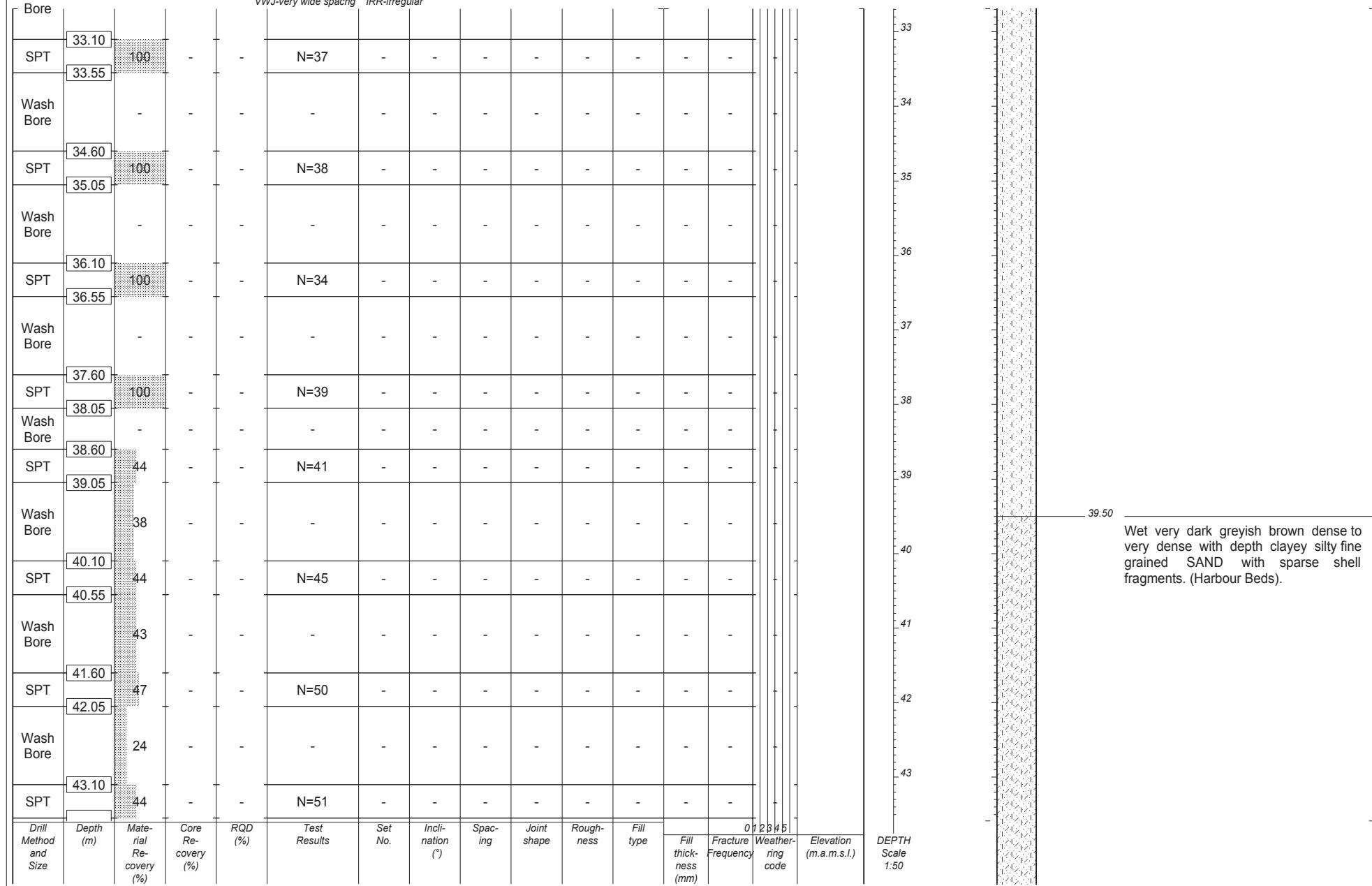
ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
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GF -gneissose	MJ-medium spacing	UND-undulating	VSR-very soft rock
LF -laminated	WJ-wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	



Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM210A
Sheet 4 of 7

JOB NUMBER: 07-395



HOLE No: BD-BHM210A
Sheet 5 of 7

JOB NUMBER: 07-395

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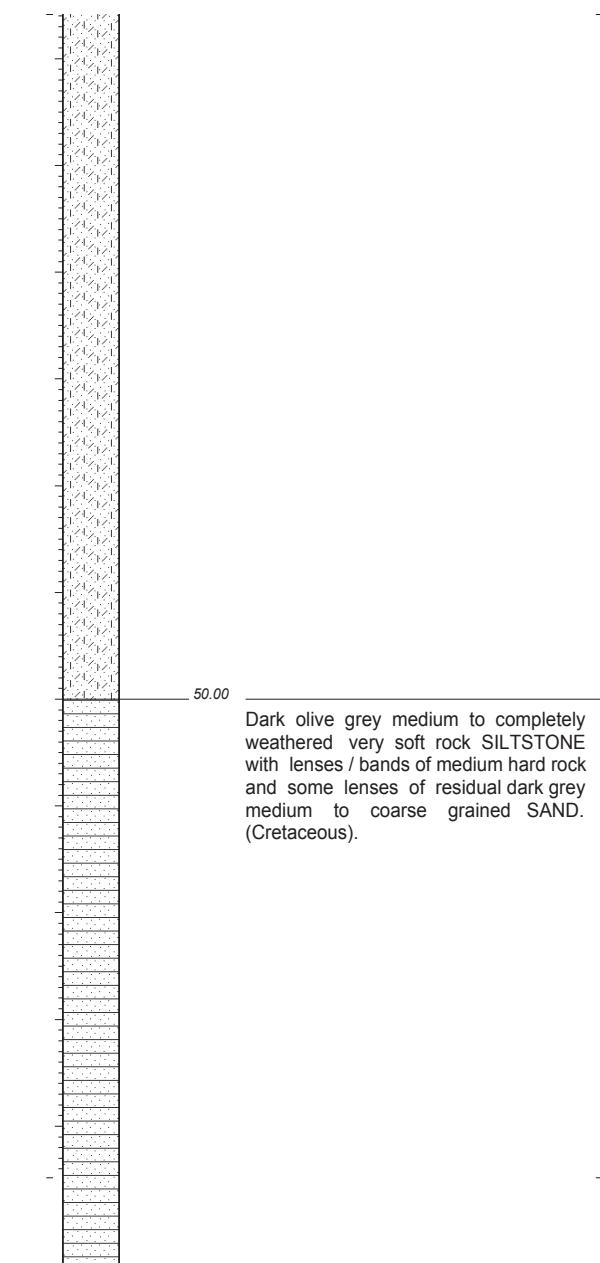
ROCK HARDNESS
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Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM210A
Sheet 5 of 7
JOB NUMBER: 07-395

Drill Method and Size	Depth (m)	Material Recovery (%)	Core Recovery (%)	RQD (%)	Test Results	Set No.	Inclination (°)	Spac-ing	Joint shape	Rough-ness	Fill type	0	1	2	3	4	5	Elevation (m.a.m.s.l.)
												Fill thick-ness (mm)	Fracture Frequency	Weathering code				DEPTH Scale 1:50
Wash	43.55	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
Bore	44.60	-	-	-	N=52	-	-	-	-	-	-	-	-	-	-	-	-	
	45.05	47	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
Wash	46.10	-	-	-	N=44	-	-	-	-	-	-	-	-	-	-	-	-	
Bore	46.55	100	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
Wash	48.00	-	-	-	N=50	-	-	-	-	-	-	-	-	-	-	-	-	
Bore	48.45	100	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
Wash	49.55	-	-	-	N=55	-	-	-	-	-	-	-	-	-	-	-	-	
Bore	50.00	100	100	25	-	1	90	M	UND	SRJ	silt	1	-	-	-	-	-	
NWD4	50.50	-	-	-	0	-	1	90	C	IRR	MRJ	silt	1	-	-	-	-	
NWD4	51.00	95	20	0	-	-	1	90	C	IRR	MRJ	silt	1	-	-	-	-	
NWD4	51.75	-	-	-	0	-	-	-	-	-	-	-	-	-	-	-	-	
Wash	52.25	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
Bore	52.65	85	22	0	-	-	1	90	M	UND	SRJ	silt	1	-	-	-	-	
NWD4	53.25	-	-	-	0	-	1	90	M	UND	SRJ	silt	1	-	-	-	-	
Wash	53.75	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
NWD4	54.25	98	60	0	-	-	1	90	M	UND	SRJ	silt	1	-	-	-	-	
Wash	54.75	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
NWD4	55.25	54	54	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
Wash	55.75	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	



HOLE No: BD-BHM210A
Sheet 6 of 7

JOB NUMBER: 07-395

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ -slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
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LF -laminated			
JOINT SPACING	JOINT SHAPE		
VCJ-very close spacg	CUR-curvilinear		
CJ-close spacing	PLA-planar		
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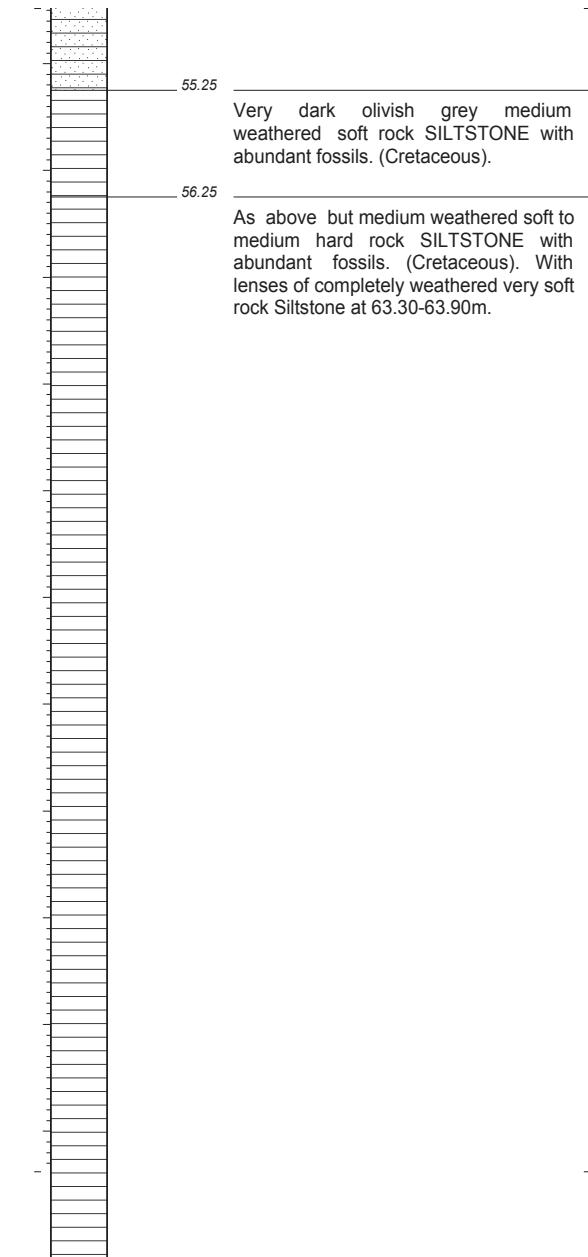


Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM210A
Sheet 6 of 7

JOB NUMBER: 07-395

Drill Method and Size	Depth (m)	Material Recovery (%)	Core Recovery (%)	RQD (%)	Test Results	Set No.	Inclination (°)	Spac-ing	Joint shape	Rough-ness	Fill type	0	1	2	3	4	5	Elevation (m.a.m.s.l.)	DEPTH Scale 1:50
Bore	54.75																		
Wash Bore	-																		
NWD4	55.25	100	100	33		-	1	90	C	UND	MRJ	slt	1	>20				55	
NWD4	55.75	68	68	-		-	-	-	-	-	-	-	-	-	-	-	-	56	
NWD4	56.25	100	100	60		-	1	0-50	C-M	UND	SRJ	slt	<1	6				57	
NWD4	56.75	100	100	80		-	1	0-50	C-M	UND	SRJ	slt	<1	2				58	
NWD4	57.25	100	100	80		-	1	0-50	C-M	UND	SRJ	slt	<1	4				59	
NWD4	57.75	72	72	70		-	1	0-50	C-M	UND	SRJ	slt	<1	4				60	
NWD4	58.25	76	76	-		-	1	0-50	C-M	UND	SRJ	slt	<1	8				61	
NWD4	58.75	100	100	67		-	1	0-50	C-M	UND	SRJ	slt	<1	7				62	
NWD4	60.25	93	93	93		-	1	0-50	M	UND	SRJ	slt	<1	2				63	
NWD4	61.00	97	97	43		-	1	0-50	C-M	UND	SRJ	slt	<1	5				64	
NWD4	61.70	100	100	75		-	1	0-50	C-M	UND	MRJ	slt	<1	13				65	
NWD4	62.50	100	100	81		-	1	0-50	M	UND	SRJ	slt	<1	4					
NWD4	63.30	100	100	-		-	-	-	-	-	-	-	-	-					
NWD4	63.90	100	100	73		-	1	0-50	M	UND	SRJ	slt	<1	9					



HOLE No: BD-BHM210A
Sheet 7 of 7

JOB NUMBER: 07-395

ROCK FABRIC
MF -massive
BF -bedded
FF -foliated
CF -cleaved
SF -schistose
GF -gneissose
LF -laminated

GRAIN SIZE
FG -fine grained
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CG -coarse grain

JOINT ROUGHNESS
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ROCK HARDNESS
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SR-soft rock
VSR-very soft rock

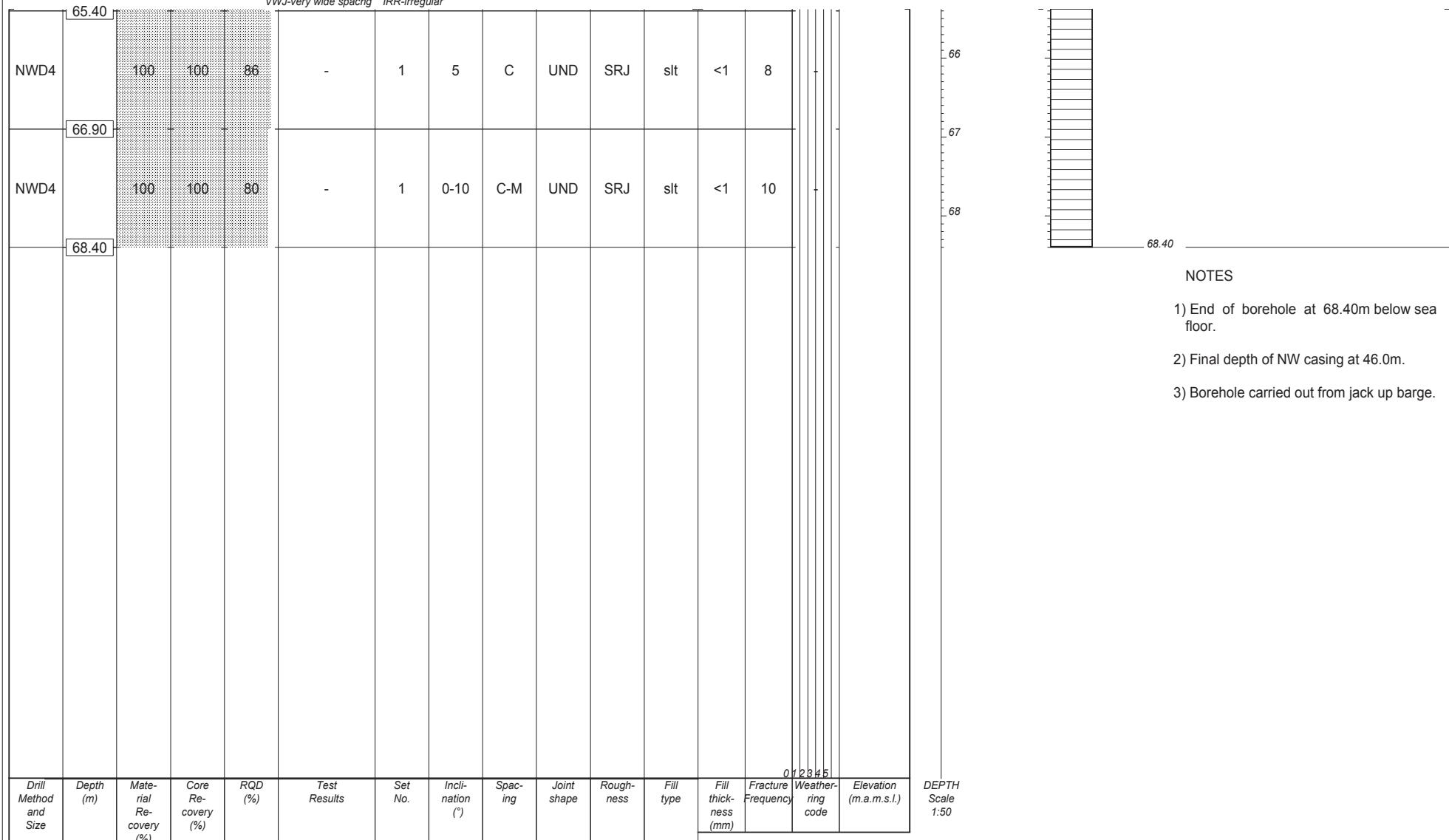
JOINT SPACING
VCJ-very close spacg
CJ-close spacing
MJ-medium spacing
WJ-wide spacing
VWJ-very wide spaeng

JOINT SHAPE
CUR-curvilinear
PLA-planar
UND-undulating
STE-stepped
IRR-irregular



Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM210A
Sheet 7 of 7
JOB NUMBER: 07-395



NOTES

- 1) End of borehole at 68.40m below sea floor.
- 2) Final depth of NW casing at 46.0m.
- 3) Borehole carried out from jack up barge.

CONTRACTOR : Geopractica

MACHINE :
DRILLED BY : Martin / Mike / Lawrence
PROFILED BY : LD

TYPE SET BY : Rev 0
SETUP FILE : MSJA3.SET

INCLINATION : 90°

DIAM : N
DATE : 01/09/2008
Y-COORD : -2280.970
DATE : 25/09/2008

DATE : 04/03/09 14:06
TEXT : ..\BHOLE\BDCF87~1.TXT

ELEVATION : -13.205 (m) CD
X-COORD : 3306551.300
Y-COORD : -2280.970

HOLE No: BD-BHM210A



BD-BHM210A

0.00 to m 17.05m

BOX 1 of 6



BD-BHM210A

17.05 to 32.05m

BOX 2 of 6



BD-BHM210A

32.05 to 46.55m

BOX 3 of 6



BD-BHM210A

46.55 to 55.25m

BOX 4 of 6



BD-BHM210A

55.25 to 61.70m

BOX 5 of 6



BD-BHM210A

61.70 to 68.40m

BOX 6 of 6

HOLE No: BD-BHM211
Sheet 1 of 6

JOB NUMBER: 07-395

ROCK FABRIC
 MF -massive
 BF -bedded
 FF -foliated
 CF -cleaved
 SF -schistose
 GF -gneissose
 LF -laminated

GRAIN SIZE
FG -fine grained
MG -medium grain
CG -coarse grain

JOINT SPACING
VCJ-very close spa
CJ -close spacing
MJ -medium joint

JOINT ROUGH	SLJ-slickenside
SJ -smooth	
RJ -rough	
JOINT SHAPE	
cg	CUR-curvilinear
	PLA-planar
	LMP-longitudinal

NESS	ROCK HARDNESS
d	EHR-extremely hard rock
	VHR-very hard rock
	HR-hard rock
	MHR-medium hard rock
-	SR-soft rock
	VSR-very soft rock

-27 31 267 7202 www.msjdbn.com



Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM211

JOB NUMBER: 07-395

HOLE No: BD-BHM211
Sheet 2 of 6

JOB NUMBER: 07-395

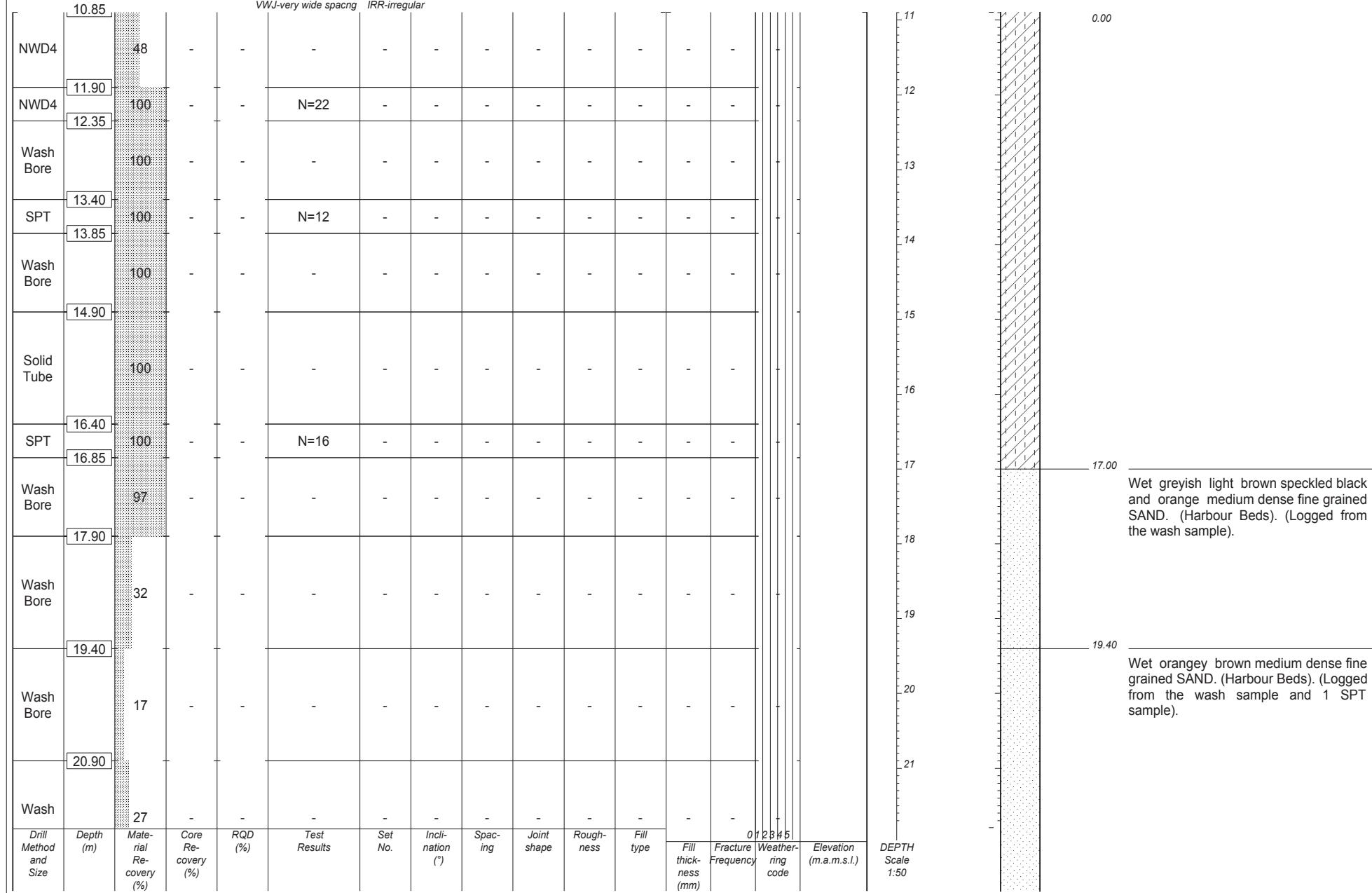
ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ -slicksided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR-hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose			SR-soft rock
GF -gneissose			VSR-very soft rock
LF -laminated			
JOINT SPACING	JOINT SHAPE		
VCJ-very close spacg	CUR-curvilinear		
CJ -close spacing	PLA-planar		
MJ -medium spacing	UND-undulating		
WJ -wide spacing	STE-stepped		
VWJ-very wide spacng	IRR-irregular		



Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM211
Sheet 2 of 6

JOB NUMBER: 07-395



HOLE No: BD-BHM211
Sheet 3 of 6

JOB NUMBER: 07-395

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ -slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR-hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose			SR-soft rock
GF -gneissose			VSR-very soft rock
LF -laminated			

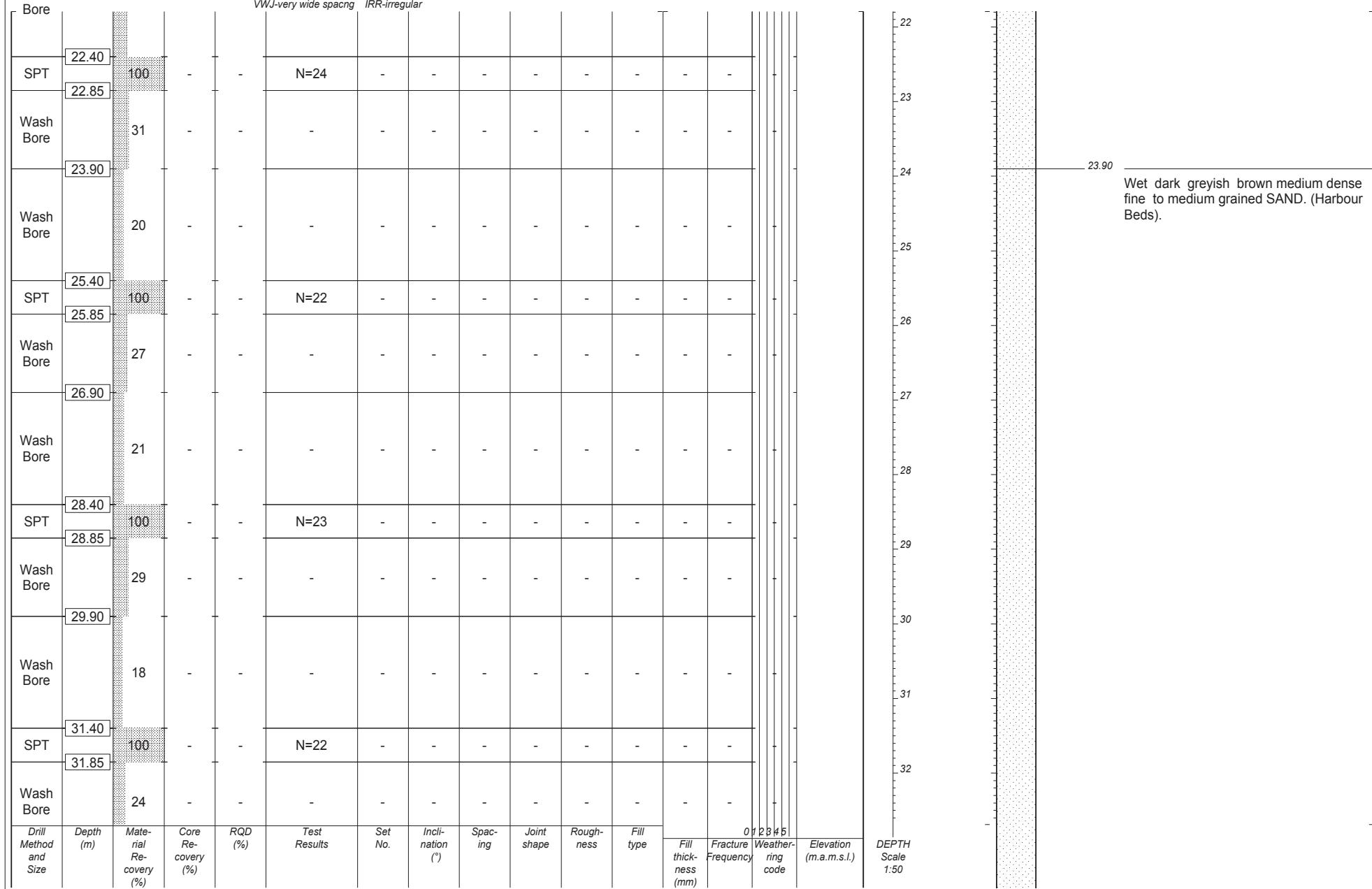
JOINT SPACING	JOINT SHAPE
VCJ-very close spacg	CUR-curvilinear
CJ-close spacing	PLA-planar
MJ-medium spacing	UND-undulating
WJ-wide spacing	STE-stepped
VWJ-very wide spaing	IRR-irregular



Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM211
Sheet 3 of 6

JOB NUMBER: 07-395



HOLE No: BD-BHM211
Sheet 4 of 6

JOB NUMBER: 07-395

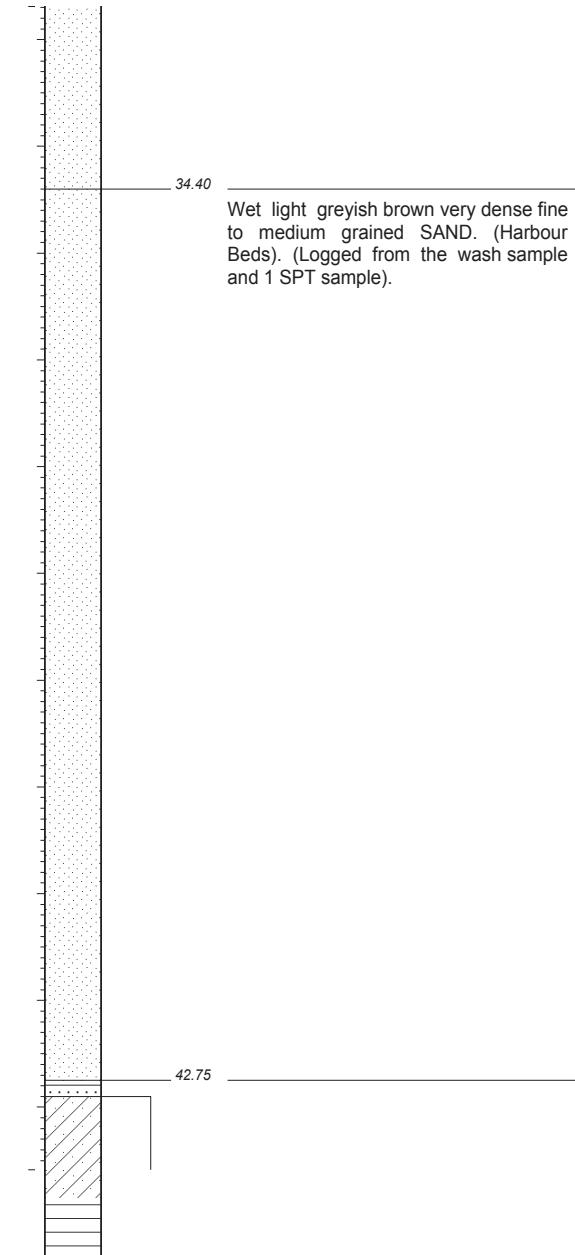
ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ -slicksided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR-hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose			SR-soft rock
GF -gneissose	VCJ-very close spacg	JOINT SHAPE	VSR-very soft rock
LF -laminated	CFJ-close spacing	CUR-curvilinear	
	MJ -medium spacing	PLA-planar	
	WJ -wide spacing	UND-undulating	
	VWJ-very wide spacg	STE-stepped	
		IRR-irregular	



**Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening**

HOLE No: BD-BHM211

JOB NUMBER: 07-395



HOLE No: BD-BHM211
Sheet 5 of 6

JOB NUMBER: 07-395

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ -slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR-hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING		SR-soft rock
GF -gneissose	VCJ-very close spacg	JOINT SHAPE	VSR-very soft rock
LF -laminated	CJ-close spacing	CUR-curvilinear	
	MJ-medium spacing	PLA-planar	
	WJ-wide spacing	UND-undulating	
	VWJ-very wide spacg	STE-stepped	
		IRR-irregular	

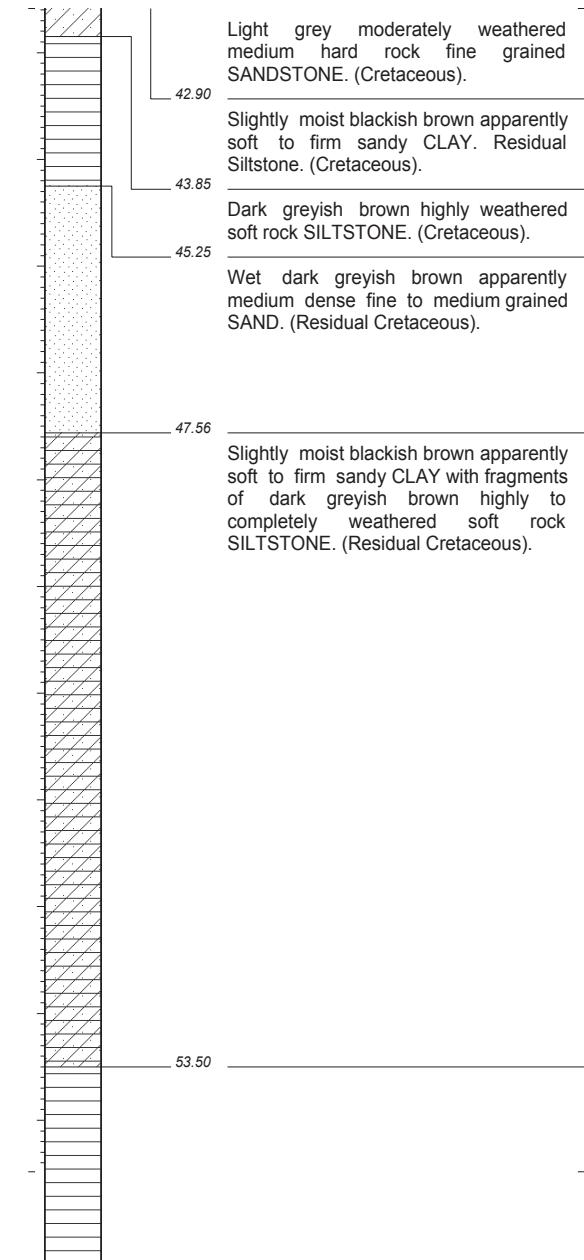


Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM211
Sheet 5 of 6

JOB NUMBER: 07-395

Drill Method and Size	Depth (m)	Material Recovery (%)	Core Recovery (%)	RQD (%)	Test Results	Set No.	Inclination (°)	Spac-ing	Joint shape	Rough-ness	Fill type	DEPTH Scale 1:50															
												1	0-5	C-M	PLA-UNDSJ-SRJ	slt	<1	5	0	1	2	3	4	5	Elevation (m.a.m.s.l.)		
NWD4	43.75																										
		68	60	20																							
NWD4	45.25																										
Wash Bore	46.06																										
Wash Bore	47.56																										
NWD4	49.06																										
NWD4	50.56																										
NWD4	52.06																										
NWD4	53.56																										
NWD4	29		29	-									1	0-5	C-M	PLA-UNDSJ-SRJ	slt	<1	5	0	1	2	3	4	5	Elevation (m.a.m.s.l.)	
Drill Method and Size	Depth (m)	Material Recovery (%)	Core Recovery (%)	RQD (%)	Test Results	Set No.	Inclination (°)	Spac-ing	Joint shape	Rough-ness	Fill type		Fill thickness (mm)	Fracture Frequency	Weathering code	DEPTH Scale 1:50											



HOLE No: BD-BHM211
Sheet 6 of 6

JOB NUMBER: 07-395

ROCK FABRIC
MF -massive
BF -bedded
FF -foliated
CF -cleaved
SF -schistose
GF -gneissose
LF -laminated

GRAIN SIZE
FG -fine grained
MG -medium grain
CG -coarse grain

JOINT ROUGHNESS
SLJ -slickensided
SJ -smooth
RJ -rough

JOINT SPACING
VCJ -very close spacg
CJ -close spacing
MJ -medium spacing
WJ -wide spacing
VWJ -very wide spacng

JOINT SHAPE
CUR -curvilinear
PLA -planar
UND -undulating
STE -stepped
IRR -irregular

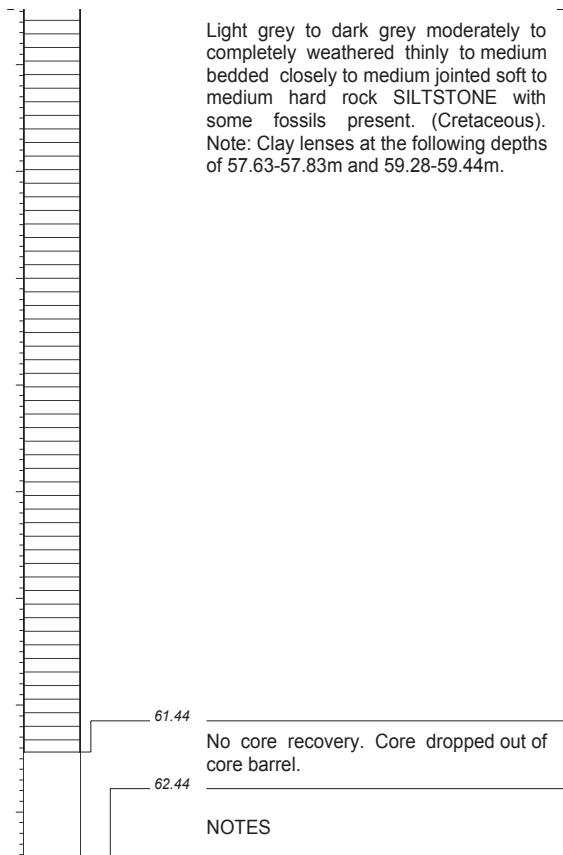


Client: TRANSNET PROJECTS
Project name: Durban Harbour
Berth Deepening

HOLE No: BD-BHM211
Sheet 6 of 6

JOB NUMBER: 07-395

Drill Method and Size	Depth (m)	Material Recovery (%)	Core Recovery (%)	RQD (%)	Test Results	Set No.	Inclination (°)	Spac-ing	Joint shape	Rough-ness	Fill type	Fill thick-ness (mm)	Fracture Frequency	Weather-ring code	Elevation (m.a.m.s.l.)	DEPTH Scale 1:50
																0 1 2 3 4 5
NWD4	55.06	64	64	-	-	1	0-5	C-M	PLA-UNDSJ-SRJ	slt	<1	13				
NWD4	56.56	53	36	-	-	1	0-5	C-M	PLA-UNDSJ-SRJ	slt	<1	5				
NWD4	57.94	87	80	-	UCS=2.152MPa	1 2	0-5 75-85	C-M C	PLA-UNDSJ-SRJ SJ-SRJ	slt slt	<1 <1	4				
NWD4	59.44	100	100	17	UCS=1.830MPa UCS=2.10MPa	1	0-5	C-M	PLA-UNDSJ-SRJ	slt	<1	9				
NWD4	60.94	33	33	17	UCS=1.131MPa	1	0-5	C-M	PLA-UNDSJ-SRJ	slt	<1	3				
NWD4	61.44	0	0	0	-	-	-	-	-	-	-	-				
	62.44															



1) End of borehole at 62.44m below sea floor.

2) Final depth of HW casing at 38.5m and NW casing at 59.44m. (Interpreted from the driller's log).

3) Borehole carried out from jack up barge.

CONTRACTOR : Geopractica
MACHINE :
DRILLED BY : Martin/ Mike/ Lawrence
PROFILED BY : SAP
TYPE SET BY : Rev 0
SETUP FILE : MSJA3.SET

INCLINATION : 90°

DIAM : N
DATE : 03/07/2008
DATE : 21/07/2008

DATE : 24/02/09 16:18
TEXT : ..\BHOLE\BD5C75~1.TXT

ELEVATION : -12.187 (m) CD
X-COORD : 3306547.080
Y-COORD : -2356.105

HOLE No: BD-BHM211



BD-BHM211

0.00 to m 9.63m

BOX 1 of 5



BD-BHM211

9.63 to 20.90m

BOX 2 of 5



BD-BHM211

20.90 to 40.75m

BOX 3 of 5



BD-BHM211

40.75 to 53.56m

BOX 4 of 5



BD-BHM211

53.56 to 61.44m

BOX 5 of 5



2.4: BOREHOLE LOGS – BASIN DREDGE AREA

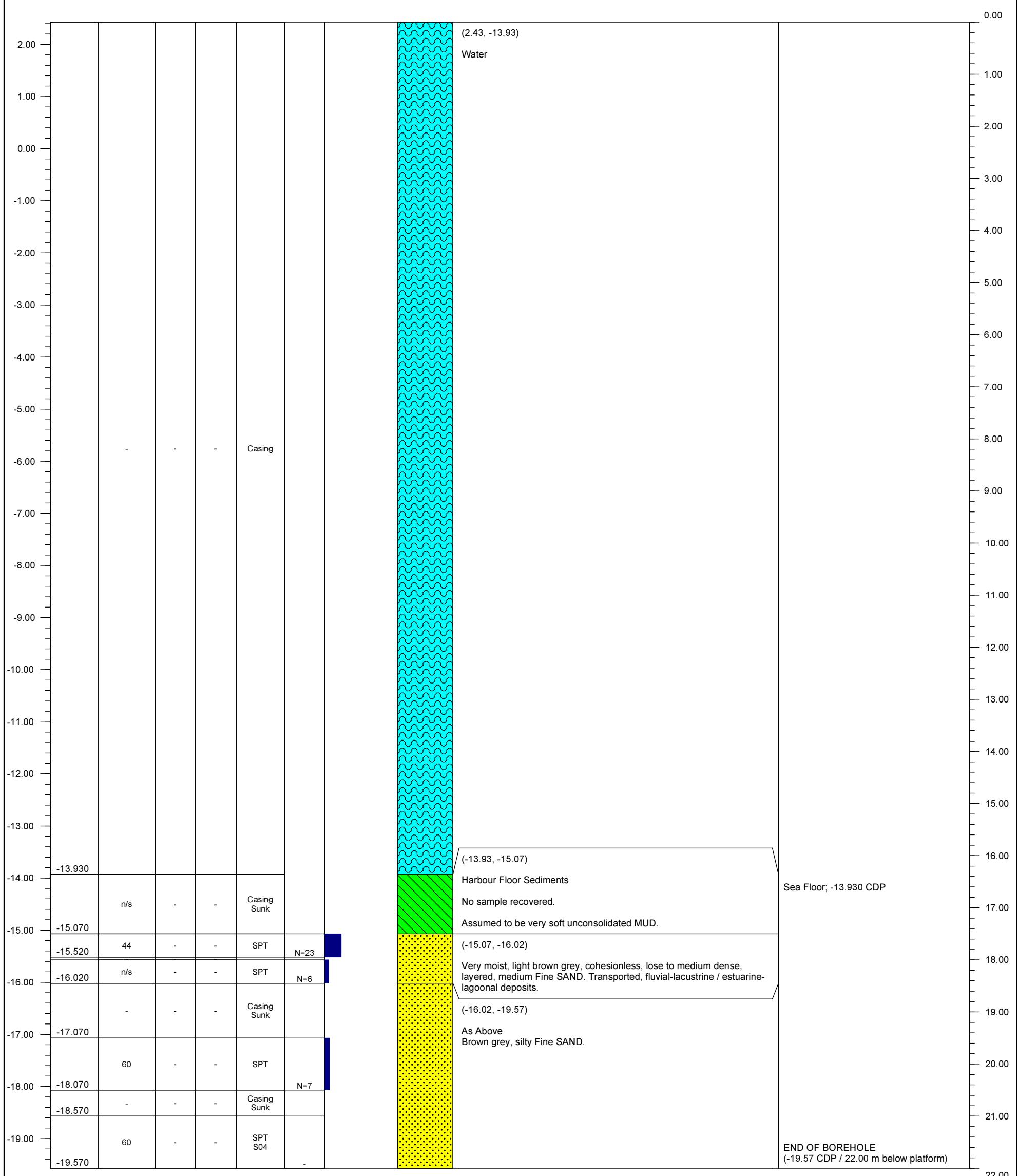


EXPLORATORY BOREHOLE LOGS – BASIN DREDGE AREA

BH	Location	End	Start depth	End depth	y	x
-	-	-	CDP	CDP	-	-
BDS01	Marine	2012/10/04	2.430	-19.570	-1796.00	3306450.00
BDS02	Marine	2012/10/05	2.493	-23.207	-2207.00	3306250.00
BDS03	Marine	2012/10/06	2.350	-19.360	-2498.00	3306153.00
BDS04	Marine	2012/10/09	2.345	-21.105	-2600.00	3306055.00
BDS05	Marine	2012/10/10	2.390	-19.060	-1990.73	3306466.63
BDS06	Marine	2012/10/22	2.293	-19.157	-1938.00	3306387.00
BDS07	Marine	2012/10/23	1.855	-19.595	-2252.00	3306308.00
BDS08	Marine	2012/10/24	1.943	-19.507	-2344.00	3306198.00
BDS09	Marine	2012/10/25	1.780	-18.170	-2451.00	3306214.00
BDS10	Marine	2012/10/26	1.930	-19.020	-2172.00	3306294.00

		ZAA Engineering Projects & Naval Architecture (Pty) Ltd						Sandy Materials		Clayey Materials		Borehole No. 1370 BDS01			
								Descriptions	SPT N	Descriptions	SPT N	Start Date	2012/10/04	End Date	2012/10/04
Project		PORT OF DURBAN BERTH DEEPENING Berths 203 to 205						Very loose	<5	Very Soft	<2	Project No:	1370 - DCT	Orientation:	Vertical
								Loose	5-10	Soft	2-4	Location:	Dredge Area Survey	Elevation:	+2.43 CDP
								Medium dense	10-30	Firm	4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306450.00
								Dense	30-50	Stiff	8-15	Drilling -	Fairbrother	Y:	-1796.00
								Very dense	>50	Very Stiff	15-30	Contractor:	Geotechnical Engineering CC	Page 1 of 1	

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description			Remarks	Depth (m below platform)
0	100												0.00

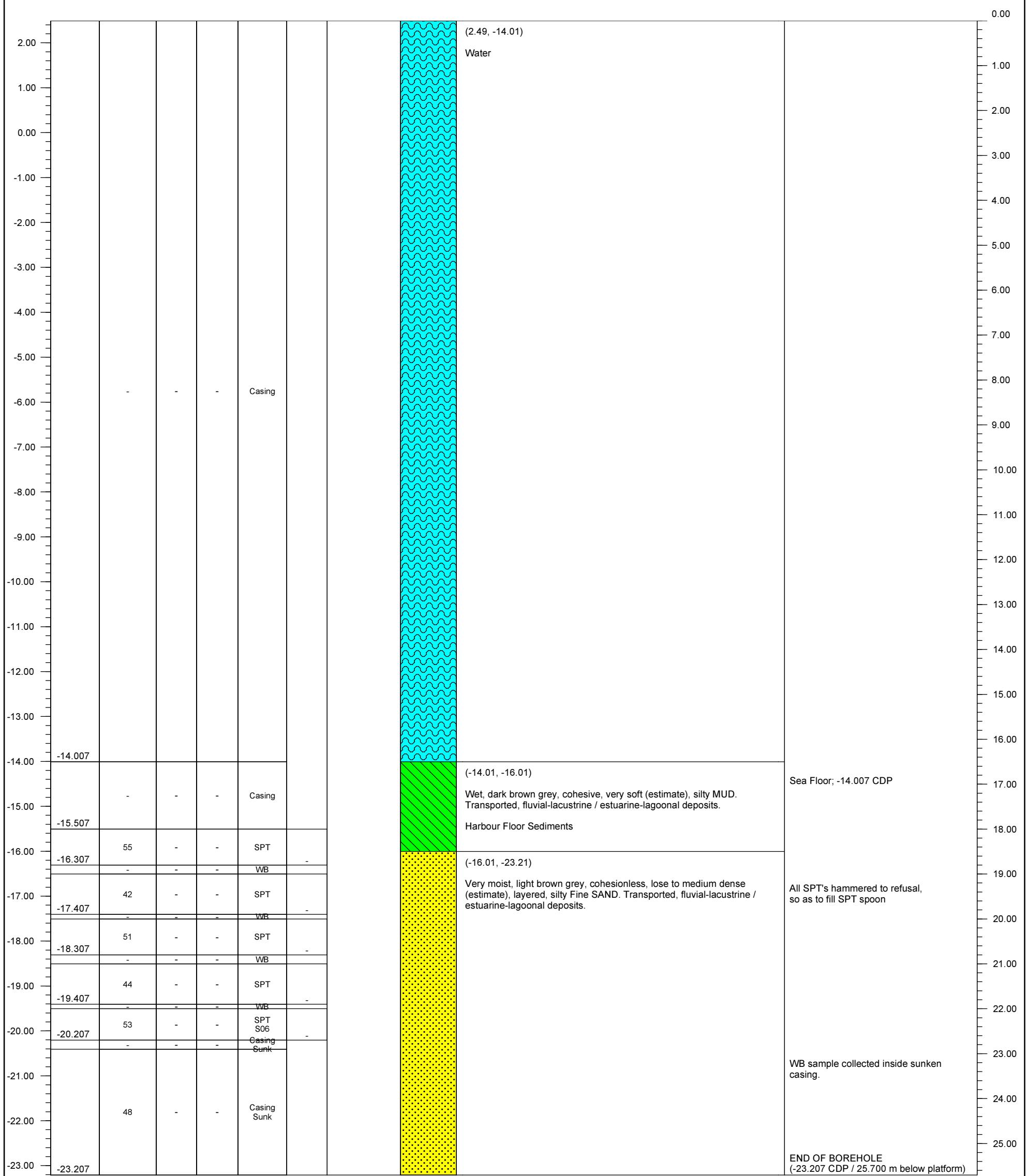




1370-BDS01: Box 1 of 1

 ZAA Engineering Projects & Naval Architecture (Pty) Ltd	ZAA Engineering Projects & Naval Architecture (Pty) Ltd						Sandy Materials		Clayey Materials		Borehole No. 1370 BDS02			
							Descriptions	SPT N	Descriptions	SPT N	Start Date	2012/10/09	End Date	2012/10/09
							Very loose	<5	Very Soft	<2	Project No:	1370 - DCT	Orientation:	Vertical
							Loose	5-10	Soft	2-4	Location:	Dredge Area Survey	Elevation:	+2.493 CDP
							Medium dense	10-30	Firm	4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306250.00
							Dense	30-50	Stiff	8-15	Drilling -	Fairbrother	Y:	-2207.00
							Very dense	>50	Very Stiff	15-30	Contractor:	Geotechnical Engineering CC	Page 1 of 1	

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description	Remarks	Depth (m below platform)
0	100										0.00

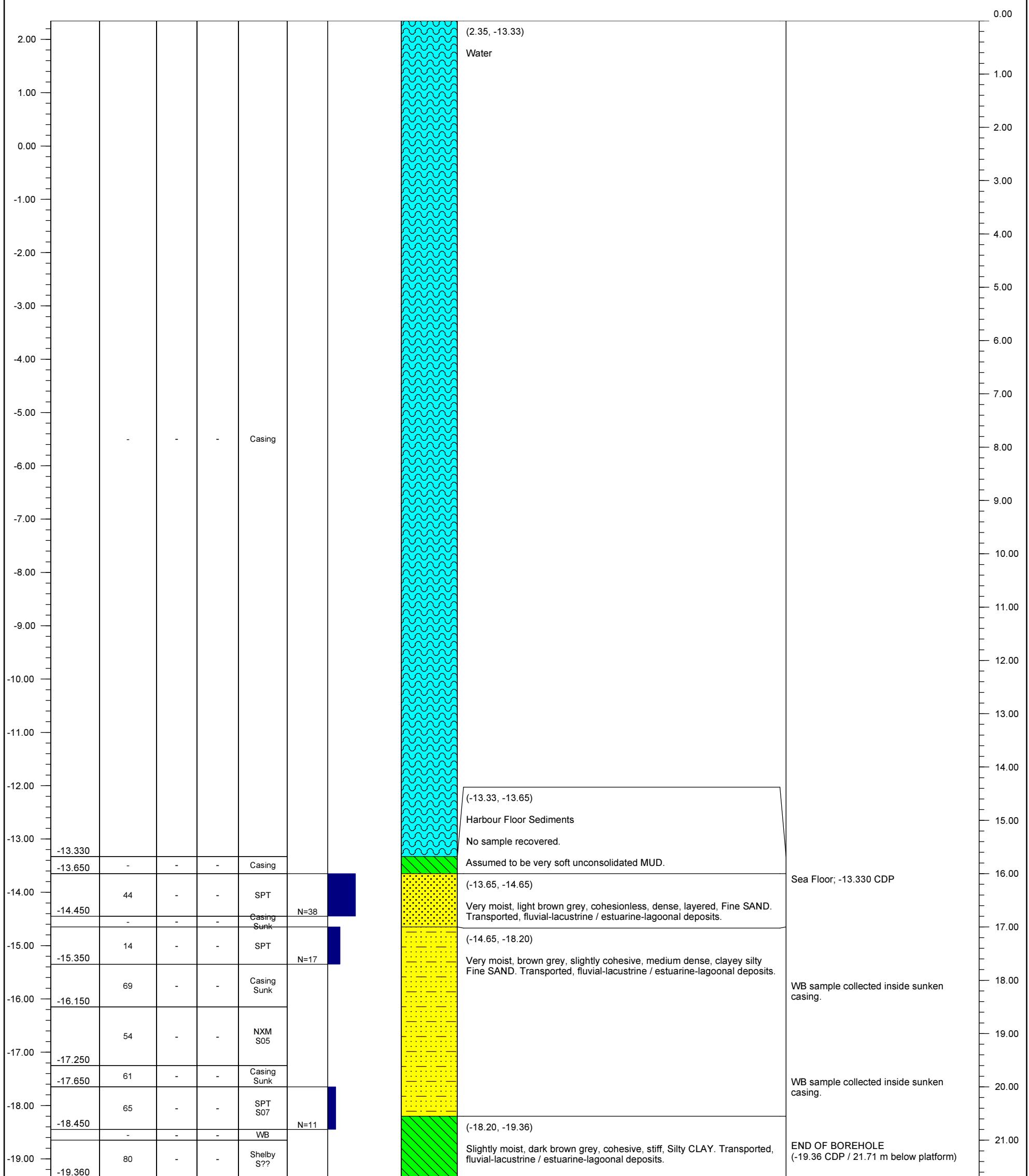




1370-BDS02: Box 1 of 1

 ZAA Engineering Projects & Naval Architecture (Pty) Ltd	ZAA Engineering Projects & Naval Architecture (Pty) Ltd							Sandy Materials		Clayey Materials		Borehole No. 1370 BDS03			
								Descriptions	SPT N	Descriptions	SPT N	Start Date	2012/10/06 <th>End Date</th> <td>2012/10/06</td>	End Date	2012/10/06
								Very loose	<5	Very Soft	<2	Project No:	1370 - DCT	Orientation:	Vertical
								Loose	5-10	Soft	2-4	Location:	Dredge Area Survey	Elevation:	+2.35 CDP
								Medium dense	10-30	Firm	4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306153.00
								Dense	30-50	Stiff	8-15	Drilling -	Fairbrother	Y:	-2498.00
								Very dense	>50	Very Stiff	15-30	Contractor:	Geotechnical Engineering CC	Page 1 of 1	

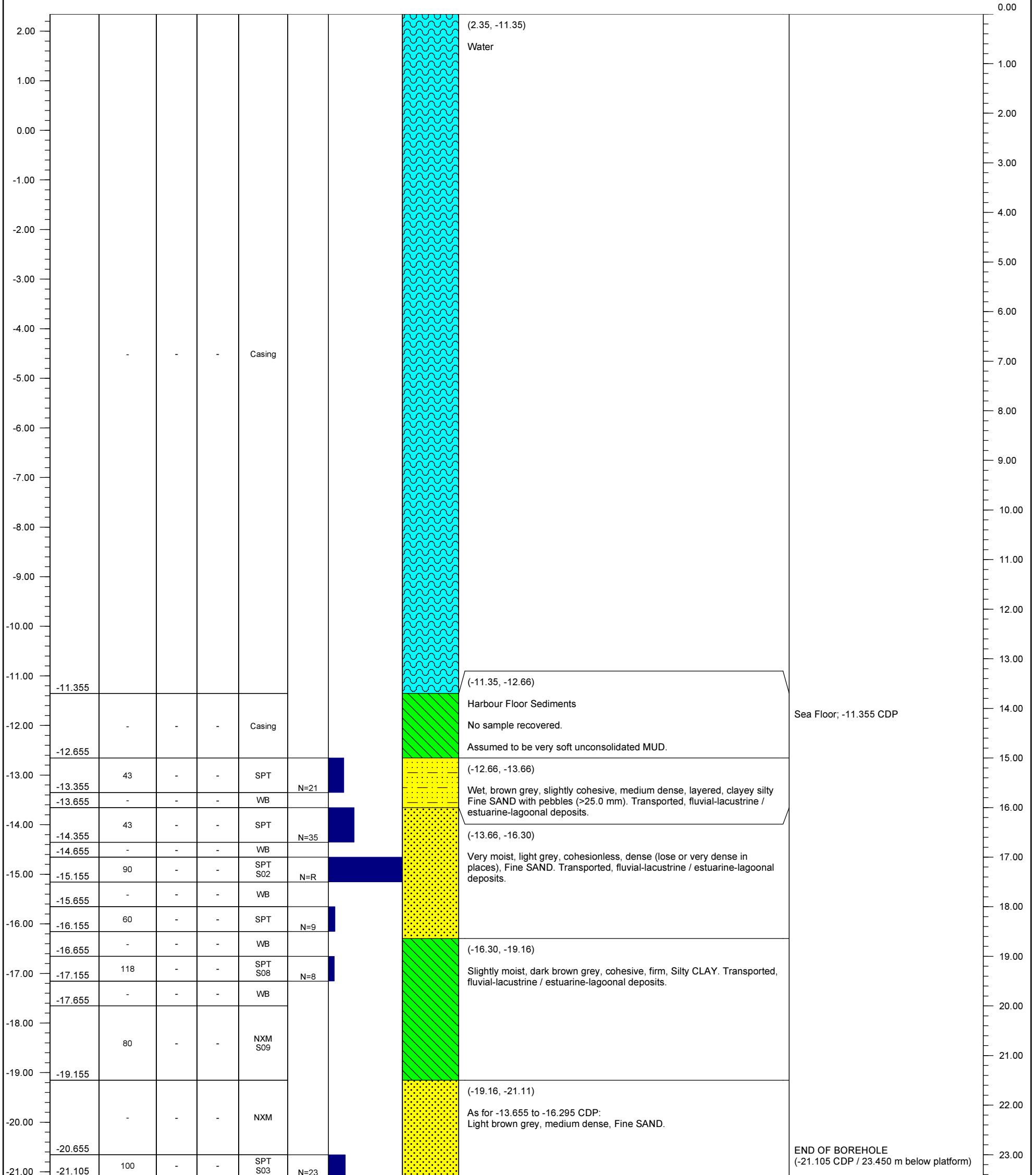
Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description			Remarks	Depth (m below platform)
0	100												0.00



ZAA Engineering Projects & Naval Architecture (Pty) Ltd 31 Melkhout Crescent Hout Bay Cape Town 7806 T: +27 (0) 21 791 9100	ZAA Project Number: 1370 Client Project Number: M-2122830-408	Client: Transnet Project: PORT OF DURBAN BERTH DEEPENING Berths 203 to 205
		Borehole No.: 1370 BDS03



1370-BDS03: Box 1 of 1

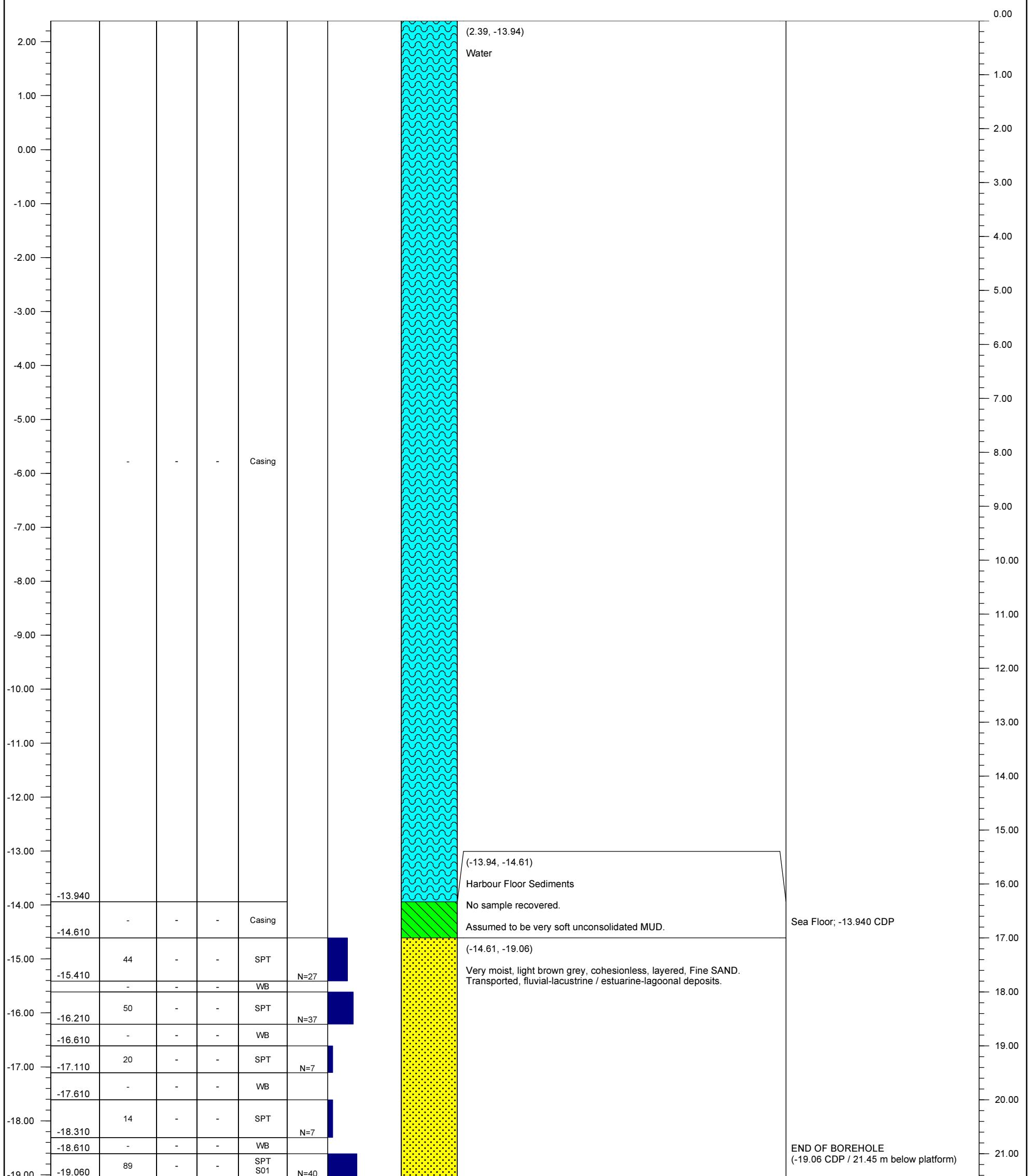
		ZAA Engineering Projects & Naval Architecture (Pty) Ltd						Sandy Materials		Clayey Materials		Borehole No. 1370 BDS04					
								Descriptions		SPT N	Descriptions		SPT N	Start Date	2012/10/09	End Date	2012/10/09
Project		PORT OF DURBAN BERTH DEEPENING Berths 203 to 205						Very loose		<5	Very Soft		<2	Project No:	1370 - DCT	Orientation:	Vertical
								Loose		5-10	Soft		2-4	Location:	Dredge Area Survey	Elevation:	+2.345 CDP
								Medium dense		10-30	Firm		4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306055.00
								Dense		30-50	Stiff		8-15	Drilling -	Fairbrother	Y:	-2600.00
								Very dense		>50	Very Stiff		15-30	Contractor:	Geotechnical Engineering CC	Page 1 of 1	
Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	0	100	Lithology	General Description			Remarks		Depth (m below platform)	
 <p>The borehole log diagram illustrates the subsurface profile from elevation -11.355 m to -21.105 m. The vertical axis on the right shows depth in meters (0.00 to 23.00 m), and the horizontal axis at the bottom shows elevation in meters (-21.00 to 2.00 m). The log is divided into several distinct layers, each with a unique color and texture. Key features include:</p> <ul style="list-style-type: none"> Top Layer: Water (blue wavy pattern) from -11.355 m to -11.35 m. Second Layer: Casing (light grey) from -11.35 m to -12.65 m. Third Layer: Harbour Floor Sediments (green diagonal lines) from -12.65 m to -11.35 m. Notes: No sample recovered, Assumed to be very soft unconsolidated MUD. Fourth Layer: Sea Floor; -11.355 CDP (grey) from -11.35 m to -12.65 m. Fifth Layer: Layered clayey silty Fine SAND with pebbles (yellow dotted pattern) from -12.65 m to -13.35 m. Notes: Wet, brown grey, slightly cohesive, medium dense. Sixth Layer: Layered clayey silty Fine SAND (yellow dotted pattern) from -13.35 m to -13.65 m. Notes: Transported, fluvial-lacustrine / estuarine-lagoonal deposits. Seventh Layer: Layered clayey silty Fine SAND (yellow dotted pattern) from -13.65 m to -14.35 m. Notes: Very moist, light grey, cohesionless, dense (lose or very dense in places). Eighth Layer: Layered clayey silty Fine SAND (yellow dotted pattern) from -14.35 m to -14.65 m. Notes: Transported, fluvial-lacustrine / estuarine-lagoonal deposits. Ninth Layer: Layered clayey silty Fine SAND (yellow dotted pattern) from -14.65 m to -15.15 m. Notes: Very moist, light grey, cohesionless, dense (lose or very dense in places). Tenth Layer: Layered clayey silty Fine SAND (yellow dotted pattern) from -15.15 m to -15.65 m. Notes: Transported, fluvial-lacustrine / estuarine-lagoonal deposits. Eleventh Layer: Layered clayey silty Fine SAND (yellow dotted pattern) from -15.65 m to -16.15 m. Notes: Very moist, light grey, cohesionless, dense (lose or very dense in places). Twelfth Layer: Layered clayey silty Fine SAND (yellow dotted pattern) from -16.15 m to -16.65 m. Notes: Transported, fluvial-lacustrine / estuarine-lagoonal deposits. Thirteenth Layer: Layered clayey silty Fine SAND (yellow dotted pattern) from -16.65 m to -17.15 m. Notes: Slightly moist, dark brown grey, cohesive, firm. Fourteenth Layer: Layered clayey silty Fine SAND (yellow dotted pattern) from -17.15 m to -17.65 m. Notes: Transported, fluvial-lacustrine / estuarine-lagoonal deposits. Fifteenth Layer: Layered clayey silty Fine SAND (yellow dotted pattern) from -17.65 m to -18.00 m. Notes: As for -13.655 to -16.295 CDP. Sixteenth Layer: Layered clayey silty Fine SAND (yellow dotted pattern) from -18.00 m to -19.15 m. Notes: Light brown grey, medium dense. Seventeenth Layer: Layered clayey silty Fine SAND (yellow dotted pattern) from -19.15 m to -20.65 m. Notes: As for -13.655 to -16.295 CDP. Eighteenth Layer: Layered clayey silty Fine SAND (yellow dotted pattern) from -20.65 m to -21.105 m. Notes: Light brown grey, medium dense. 																	



1370-BDS04: Box 1 of 1

 ZAA Engineering Projects & Naval Architecture (Pty) Ltd	ZAA Engineering Projects & Naval Architecture (Pty) Ltd						Sandy Materials		Clayey Materials		Borehole No. 1370 BDS05				
							Descriptions	SPT N	Descriptions	SPT N	Start Date	2012/10/10	End Date	2012/10/10	
							Very loose	<5	Very Soft	<2	Project No:	1370 - DCT	Orientation:	Vertical	
							Loose	5-10	Soft	2-4	Location:	Dredge Area Survey	Elevation:	+2.390 CDP	
							Medium dense	10-30	Firm	4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306463.00	
							Dense	30-50	Stiff	8-15	Drilling -	Fairbrother	Y:	-1994.00	
Project		PORT OF DURBAN BERTH DEEPENING Berths 203 to 205						Very dense	>50	Very Stiff	15-30	Contractor:	Geotechnical Engineering CC	Page 1 of 1	

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description			Remarks	Depth (m below platform)
0	100												0.00





1370-BDS05: Box 1 of 1

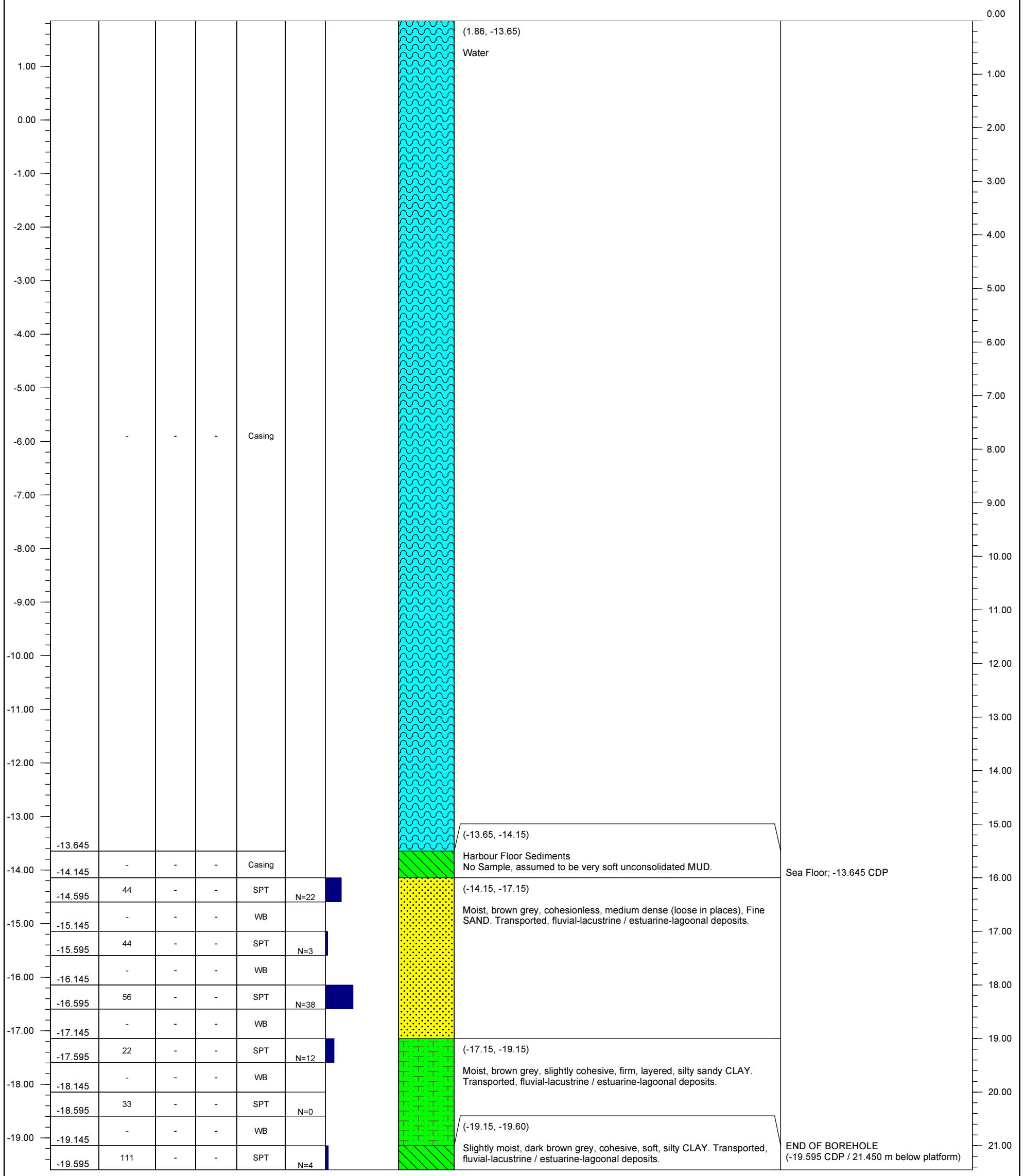
		ZAA Engineering Projects & Naval Architecture (Pty) Ltd						Sandy Materials		Clayey Materials		Borehole No. 1370 BDS06					
								Descriptions		SPT N	Descriptions		SPT N	Start Date	2012/10/22	End Date	2012/10/22
Project		PORT OF DURBAN BERTH DEEPENING Berths 203 to 205						Very loose		<5	Very Soft		<2	Project No:	1370 - DCT	Orientation:	Vertical
								Loose		5-10	Soft		2-4	Location:	Dredge Area Survey	Elevation:	+2.293 CDP
								Medium dense		10-30	Firm		4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306387.00
								Dense		30-50	Stiff		8-15	Drilling -	Fairbrother	Y:	-1938.00
								Very dense		>50	Very Stiff		15-30	Contractor:	Geotechnical Engineering CC	Page 1 of 1	
Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	0	100	Lithology	General Description			Remarks		Depth (m below platform)	
																0.00	
																1.00	
																2.00	
																3.00	
																4.00	
																5.00	
																6.00	
																7.00	
																8.00	
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																14.00	
																15.00	
																16.00	
																17.00	
																18.00	
																19.00	
																20.00	
																21.00	
ZAA Engineering Projects & Naval Architecture (Pty) Ltd 31 Melkhout Crescent Hout Bay Cape Town 7806 T: +27 (0) 21 791 9100							ZAA Project Number: 1370 Client Project Number: M-2122830-408						Client: Transnet Project: PORT OF DURBAN BERTH DEEPENING Berths 203 to 205 Borehole No.: 1370 BDS06				



1370-BDS06: Box 1 of 1

 ZAA Engineering Projects & Naval Architecture (Pty) Ltd								Sandy Materials		Clayey Materials		Borehole No. 1370 BDS07			
Project		PORT OF DURBAN BERTH DEEPENING Berths 203 to 205					Descriptions	SPT N	Descriptions	SPT N	Start Date	2012/10/23	End Date	2012/10/23	
							Very loose	<5	Very Soft	<2	Project No:	1370 - DCT	Orientation:	Vertical	
							Loose	5-10	Soft	2-4	Location:	Dredge Area Survey	Elevation:	+1.855 CDP	
							Medium dense	10-30	Firm	4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306308.00	
							Dense	30-50	Stiff	8-15	Drilling -	Fairbrother	Y:	-2252.00	
							Very dense	>50	Very Stiff	15-30	Contractor:	Geotechnical Engineering CC	Page 1 of 1		

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description			Remarks		Depth (m below platform)
0	100													



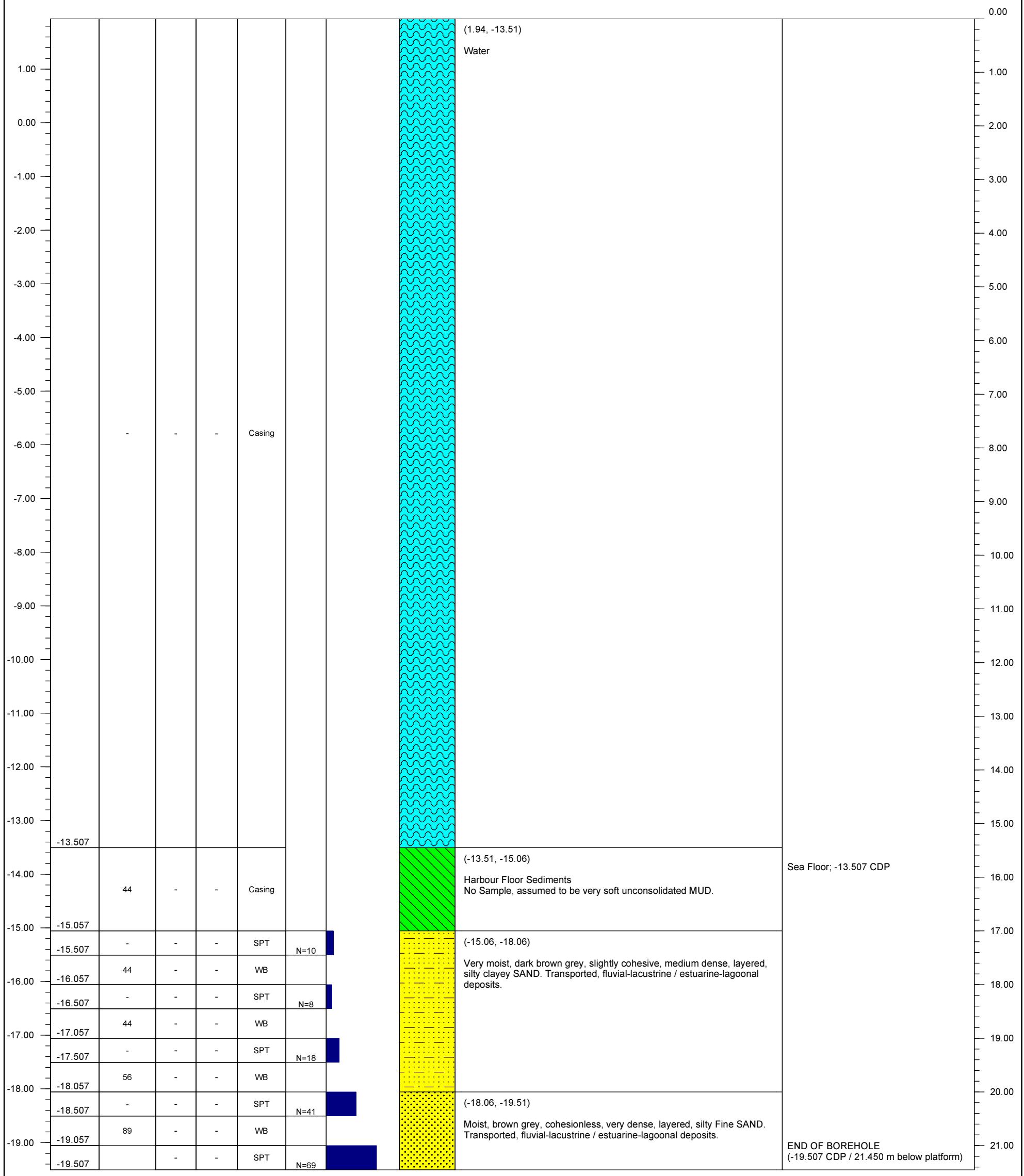
ZAA Engineering Projects & Naval Architecture (Pty) Ltd 31 Melkhout Crescent Hout Bay Cape Town 7806 T: +27 (0) 21 791 9100	ZAA Project Number: 1370 Client Project Number: M-2122830-408	Client: Transnet Project: PORT OF DURBAN BERTH DEEPENING Berths 203 to 205 Borehole No.: 1370 BDS07
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1370-BDS07: Box 1 of 1

 ZAA Engineering Projects & Naval Architecture (Pty) Ltd	ZAA Engineering Projects & Naval Architecture (Pty) Ltd							Sandy Materials		Clayey Materials		Borehole No. 1370 BDS08			
								Descriptions	SPT N	Descriptions	SPT N	Start Date	2012/10/24 <th>End Date</th> <td>2012/10/24</td>	End Date	2012/10/24
								Very loose	<5	Very Soft	<2	Project No:	1370 - DCT	Orientation:	Vertical
								Loose	5-10	Soft	2-4	Location:	Dredge Area Survey	Elevation:	+1.943 CDP
								Medium dense	10-30	Firm	4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306198.00
								Dense	30-50	Stiff	8-15	Drilling -	Fairbrother	Y:	-2344.00
								Very dense	>50	Very Stiff	15-30	Contractor:	Geotechnical Engineering CC	Page 1 of 1	

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description			Remarks	Depth (m below platform)
						0	100						

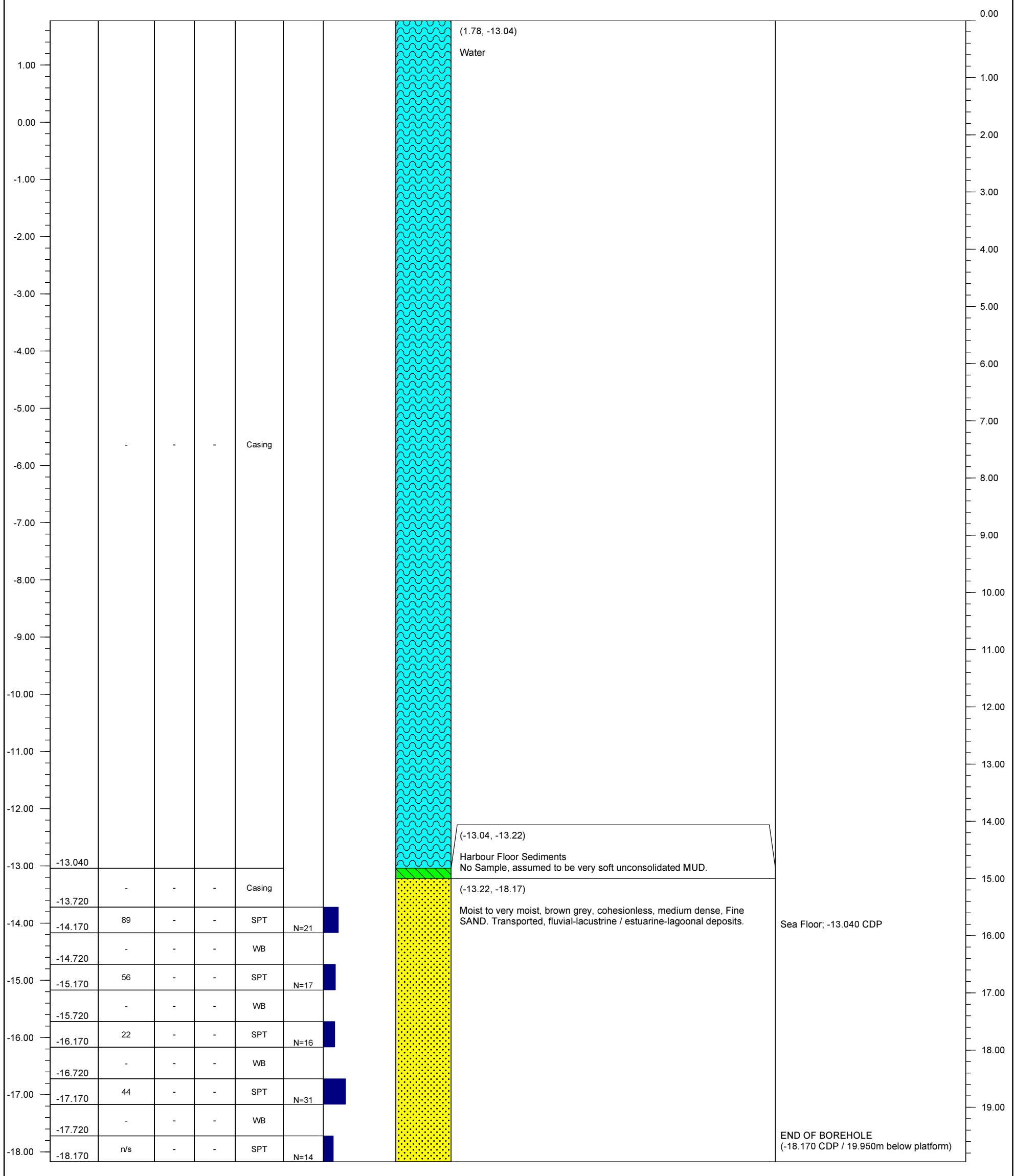




1370-BDS08: Box 1 of 1

 ZAA Engineering Projects & Naval Architecture (Pty) Ltd	ZAA Engineering Projects & Naval Architecture (Pty) Ltd							Sandy Materials		Clayey Materials		Borehole No. 1370 BDS09			
								Descriptions	SPT N	Descriptions	SPT N	Start Date	2012/10/25	End Date	2012/10/25
	Project PORT OF DURBAN BERTH DEEPENING Berths 203 to 205							Very loose	<5	Very Soft	<2	Project No:	1370 - DCT	Orientation:	Vertical
								Loose	5-10	Soft	2-4	Location:	Dredge Area Survey	Elevation:	+1.780 CDP
								Medium dense	10-30	Firm	4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306214.00
								Dense	30-50	Stiff	8-15	Drilling -	Fairbrother	Y:	-2451.00
								Very dense	>50	Very Stiff	15-30	Contractor:	Geotechnical Engineering CC	Page 1 of 1	

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description			Remarks	Depth (m below platform)
0	100												0.00



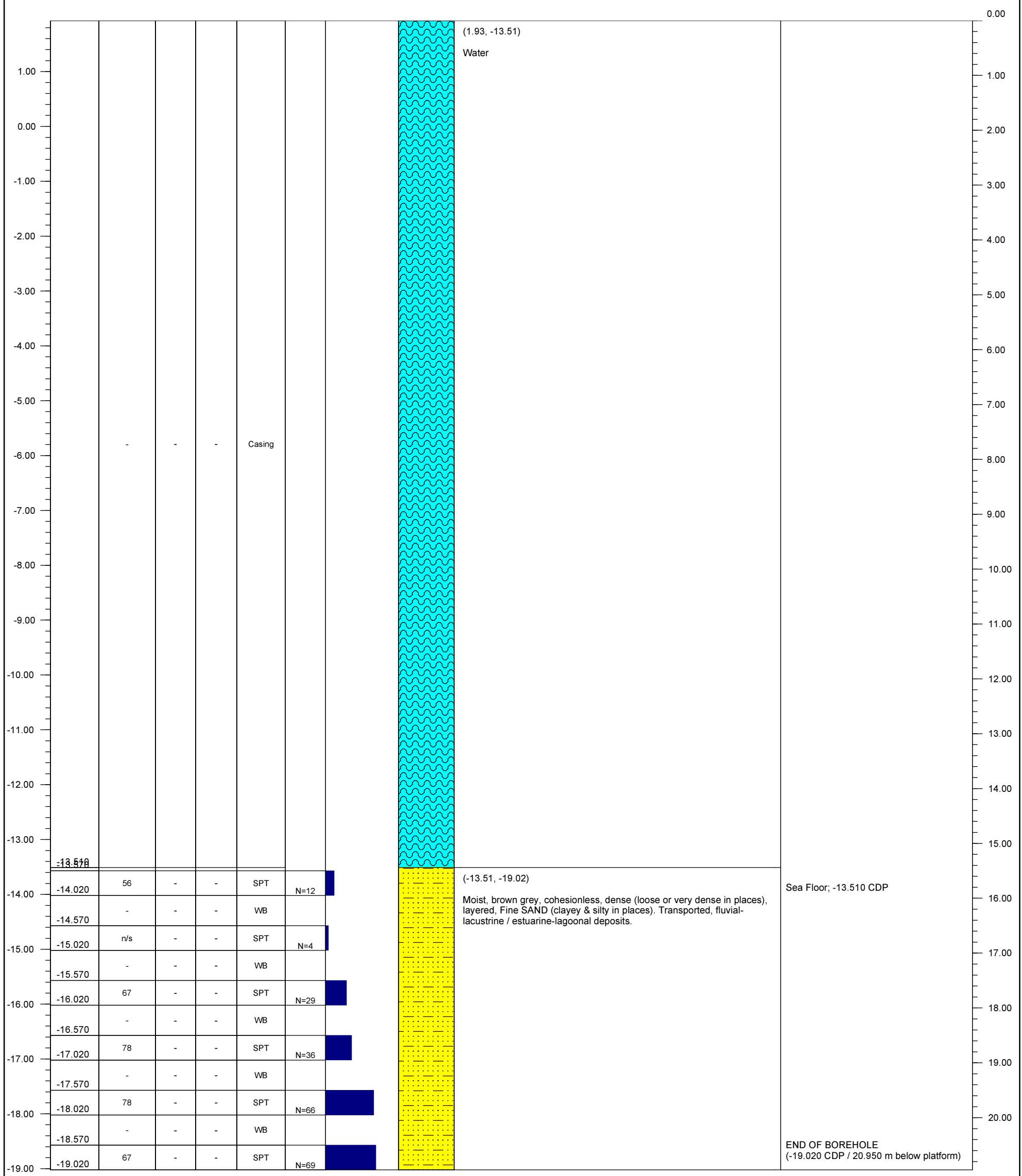
ZAA Engineering Projects & Naval Architecture (Pty) Ltd 31 Melkhout Crescent Hout Bay Cape Town 7806 T: +27 (0) 21 791 9100	ZAA Project Number: 1370 Client Project Number: M-2122830-408	Client: Transnet Project: PORT OF DURBAN BERTH DEEPENING Berths 203 to 205
		Borehole No.: 1370 BDS09



1370-BDS09: Box 1 of 1

 ZAA Engineering Projects & Naval Architecture (Pty) Ltd	ZAA Engineering Projects & Naval Architecture (Pty) Ltd				Sandy Materials		Clayey Materials		Borehole No. 1370 BDS10			
					Descriptions	SPT N	Descriptions	SPT N	Start Date	2012/10/26	End Date	2012/10/26
					Very loose	<5	Very Soft	<2	Project No:	1370 - DCT	Orientation:	Vertical
					Loose	5-10	Soft	2-4	Location:	Dredge Area Survey	Elevation:	+1.930 CDP
					Medium dense	10-30	Firm	4-8	Logged By:	MAS Baleta	Coordinates:	X: 3306294.00
					Dense	30-50	Stiff	8-15	Drilling -	Fairbrother	Y:	-2172.00
					Very dense	>50	Very Stiff	15-30	Contractor:	Geotechnical Engineering CC	Page 1 of 1	

Elev. (CDP)	Run Elev. (CDP)	Material / Core Recovery %	RQD %	FF (J/m)	Method & Sample	SPT Value 'N'	SPT 'N' Bar Graph	Lithology	General Description		Remarks	Depth (m below platform)
0	100											





1370-BDS010: Box 1 of 1



ANNEXURE 3: CPT_U RESULTS

- 3.1 EXPLANATORY NOTE ON CPT_U LOGS**
- 3.2 OSIMO METHOD STATEMENT**
- 3.3 CPT_U LOGS – QUAY WALL**
- 3.4 CPT_U LOGS – BASIN DREDGE AREA**
- 3.5 CPT_U LOGS – LOT 10 CASTING YARD**



3.1: EXPLANATORY NOTE ON CPTu LOGS



EXPLANATION OF CPTU LOGS

This page will describe the information presented on the CPTU logs which follow.

The CPTU naming convention is as follows:

- CPT series CPTU's – Marine and land boreholes probed along the new quay wall line.
- P & S series CPTU's – Primary and Secondary marine CPTU's probed in the dredge area.

FIRST GRAPH (CONE RESISTANCE VS DEPTH)

These plots show cone resistance and pore pressure, measured in MPa and kPa respectively, against depth. Depth is recorded in meters below platform level or ground level depending on where the probing was performed.

SECOND GRAPH (CONE PRESSURE, PORE PRESSURE & HYDRODYNAMIC PRESSURE VS DEPTH)

The second series of plots present the same information however these plots introduce an additional variable that is the hydrostatic head, represented in kPa. The hydrostatic head increases linearly with depth

DISSIPATION TESTS

The dissipation graphs show the depth at which the dissipation test was performed, the peak pore water pressure (in kPa) and the time elapsed since the start of the dissipation test.

The red line indicates the hydrostatic head at the depth of testing.

SOIL IDENTIFICATION CHART

The soil identification chart utilises measured parameters (i.e. Pore water pressure and cone resistance) to classify the soil through which the probe is being pushed.

The soils identification chart may be found in the following document: A.C.Meigh-CIRIA,1987.



3.2: OSIMO METHOD STATEMENT

Operating Procedure for Osimo Probe Rig

Get to specific probe position

Drive to the place of work
Drive under escort with vehicle lights on (if required).
Be aware of earth moving equipment as well as other heavy machinery
Give earth moving equipment right of way
Put vehicle in 4 x 4 when driving at inclines and declines
Drive at 10km per hour
Be aware of wet ground

Unloading testing equipment from trailer using ramps

Do a 'TAKE 5 RISK ASSESSMENT'
Check cable and hook arrangement for damage
Open the flap of the trailer and pull out the ramps
Inspect ramp for damage
Auger in anchors manually (Approx. 1 meter)
Wind down the rig slowly by means of the manual winch
Lift mast (cylinders) to the horizontal position (two persons – one lift, one secure)
Secure cylinders by means of a securing pin
Secure rig to anchors by screwing the nuts on
Complete daily check list and refuel by using the funnel arrangement attached to the Jerry can
(Check that engine is still cold before doing this)
Start engine and monitor if in good working condition
Connect electronic cables
Switch on logger
Install and align first rod in rig (Place rod in guide)
Push down the first rod and put second rod in guide – screw in
The operation is a hydraulic system that operates by means of hand operated lever system
Repeat this process until completion.

Taking out the rods

Reverse the process and pull out the first rod
Clamp the second rod (Hydraulic operated clamp)
Unscrew the first rod and place in trailer
Repeat process until completion

De-commission

Unscrew holding nuts
Auger out the anchors
Lower cylinders to transport position
Attached winch – winch the rig onto trailer
Secure the rig and move to the next position

Refueling during operation (petrol / oil)

1. Stop engine
2. Clean area around petrol tank.
3. Allow cooling period of 10 minutes
4. Fill tank
5. Ensure area clean of petrol/oil spillage
6. Start engine

Reference test procedure.

The first International Reference Test Procedure (IRTP) for Cone Penetration Test (CPT) tips with electronic transducers was published in the First International Symposium on Penetration Testing (ISOPT -1, 1988) by the TC16 of the ISSMGE in Orlando, Florida. It is also included as part of the CIRIA report by A.C.Meigh on Cone Penetration Testing. A further IRTP, based on the 1988 document, was published by TC16 in 1999. Some minor amendments have been published since then.

Most countries where CPT testing is carried out have their own National Reference Test Procedure (NRTP) usually based on the 1988 or 1999 IRTP but with amendments to address their particular circumstances and these often differ significantly from the IRTP. Here are some examples:

- **Europe: CEN standard EN ISO 22476-1 (2007) for electrical piezocone**
- Austria: DIN 4094-1
- France: NF P94-113 CPT and NF P94-119 CPTu
- Germany: DIN 4094-1
- Netherlands: NEN 5140
- **United Kingdom: BS1377, BS5930**
- Bulgarian Standard (use German DIN 4094-90)
- Czech Standard STN 721033
- Slovakia Standard CSN 721033
- Estonia Reference LBN 207-01
- Latvia Reference LVS 437
- Lithuania Reference LBN 005-99
- Hungary Standard MI 15000/2 (1989)
- Poland: Polish Standard PN-B-04452 (2002)
- Romanian Standard NP 074-2007
- Russia Standards GOST 19912-2001 Code SP 11 105 97
- Egypt: Use British Standards: BS 1377 - Part 9 on In-Situ Testing
- USA and Canada: ASTM D 3441 (mechanical cone); ASTM D 5778 (2007) - electric piezocone
- Norway: Norwegian Geotechnical Society Guideline 5 (1995)
- Sweden: Swedish Geotechnical Society SGF Report 1:93E
- Spain: Reference is the UNE 103-804-93
- Australian National Standard: AS1289.6.5.1-1999
- New Zealand: National Standard NZS 4402.6.5.3:1988

The IRTP has not been formally adopted in South Africa and we have not drawn up our own NRTP. However, the general practice has been to comply with the IRTP as far as practically possible. The Osimo Piezocone was designed as a Reference Cone (R) with the pore pressure measured at position u_2 , the “preferred position” as specified in the IRTP. The geometry is a

standard 60°, 36mm (10cm²) cone with one metre rods with a diameter of 36mm. The CPT rig has a capacity of at least 100kN.

Calibration of the pore pressure transducer as well as the cone load cell is carried out on a Budenberg “dead-load hydraulic calibration rig” No 25470. The Budenberg has a calibration certificate valid until 14/03/2013. (IMP UHQ- 18065 dated 14/03/2012).

However there are some areas where we do not comply with the INTP.

The first is the presentation of the data. We prefer to plot the measured pore pressure and cone resistance on the same plot. It is more informative in highly layered profiles such as mine tailings where the interaction of these two measurements is more apparent plotted in this fashion.

However, the data is available in electronic format so this could easily be changed to suit the requirements of the client.

The second is that we do not as a rule record sleeve friction in Piezocone (CPTu) testing. The reason for this is that pore pressure response to penetration is a far better indication of soil type than the friction ratio. The sleeve friction is also measured over 130mm which is often more than the layer thickness, especially in mine tailings which is a large proportion of CPTu testing in SA. The sleeve friction is therefore not applicable under these circumstances. Furthermore, our soils are often residual soils, or sedimentary soil (or tailings) over residual soils which often contain boulders. The cones capable of measuring sleeve friction are quite fragile and susceptible to damage in our residual soils. These conditions are rare in Europe or the US so they do not have this problem.

Prof. Eben Rust. BSc(Eng)Civil. BEng(Hons). MEng. PhD (Geotechnics)Surrey



3.3: CPTu LOGS – QUAY WALL



EXPLORATORY BOREHOLE LOGS – BASIN DREDGE AREA

CPT	Location	Date	Start depth	End depth	y	x
-	-	-	CDP	CDP	-	-
01c	Marine	2012/06/25	3.557	-24.443	-1382.73	3306756.41
2	Marine	2012/06/20	3.365	-25.105	-1406.22	3306746.82
3	Marine	2012/06/18	3.605	-24.395	-1426.44	3306720.30
4	Marine	2012/06/21	3.277	-24.143	-1446.81	3306734.96
5	Marine	2012/06/26	3.800	-25.500	-1469.72	3306729.06
6	Marine	2012/06/22	4.033	-25.967	-1481.97	3306710.49
7	Marine	2012/06/24	3.865	-32.135	-1526.40	3306703.31
8	Marine	2012/06/23	3.840	-26.810	-1544.20	3306703.49
9	Marine	2012/06/06	3.137	-26.463	-1568.23	3306705.75
10	Marine	2012/06/28	3.667	-28.333	-1597.75	3306694.66
11	Marine	2012/06/14	3.543	-34.257	-1620.31	3306678.86
12	Marine	2012/06/15	2.849	-30.751	-1643.43	3306677.06
13	Marine	2012/06/16	3.720	-23.580	-1665.74	3306677.68
14	Marine	2012/07/04	3.500	-24.200	-1686.51	3306675.07
15	Marine	2012/07/05	3.100	-25.200	-1707.32	3306664.23
16	Marine	2012/06/27	3.565	-24.735	-1739.48	3306676.34
17	Marine	2012/07/05	3.350	-25.300	-1762.98	3306661.79
18	Marine	2012/06/27	3.440	-29.060	-1785.97	3306654.34
19	Marine	2012/06/26	3.720	-24.280	-1809.11	3306649.93
20	Marine	2012/06/29	3.650	-26.350	-1830.02	3306641.90
21	Marine	2012/07/02	4.010	-25.290	-1856.76	3306632.69
22	Marine	2012/07/02	3.846	-23.054	-1885.63	3306632.39
23	Marine	2012/06/13	3.526	-21.774	-1907.47	3306624.08
24	Marine	2012/06/14	3.547	-20.553	-1932.31	3306620.35
25	Marine	2012/07/04	2.920	-21.080	-1958.52	3306640.29
26	Marine	2012/07/03	3.490	-19.510	-1983.13	3306644.16
27	Marine	2012/06/29	2.790	-26.510	-2002.34	3306622.44
28	Marine	2012/06/30	3.485	-21.615	-2037.19	3306631.35
29	Marine	2012/06/12	3.420	-21.750	-2052.54	3306601.04
30	Marine	2012/06/12	3.375	-23.245	-2084.30	3306585.08
31	Marine	2012/06/09	3.940	-26.410	-2103.97	3306581.39
32	Marine	2012/06/10	3.800	-24.200	-2119.98	3306581.64
33b	Marine	2012/06/07	3.835	-28.345	-2149.92	3306567.17
34	Marine	2012/06/08	3.855	-22.845	-2178.24	3306565.11
35	Marine	2012/06/03	3.257	-26.243	-2201.36	3306557.36
36	Marine	2012/06/02	4.307	-28.193	-2226.59	3306553.22
37	Marine	2012/06/01	3.803	-25.197	-2251.72	3306548.81

PIEZOCONE PENETRATION TEST

PROJECT: Durban Harbour Over Water

SITE: Pier2

CPTU (RE): CPT01C

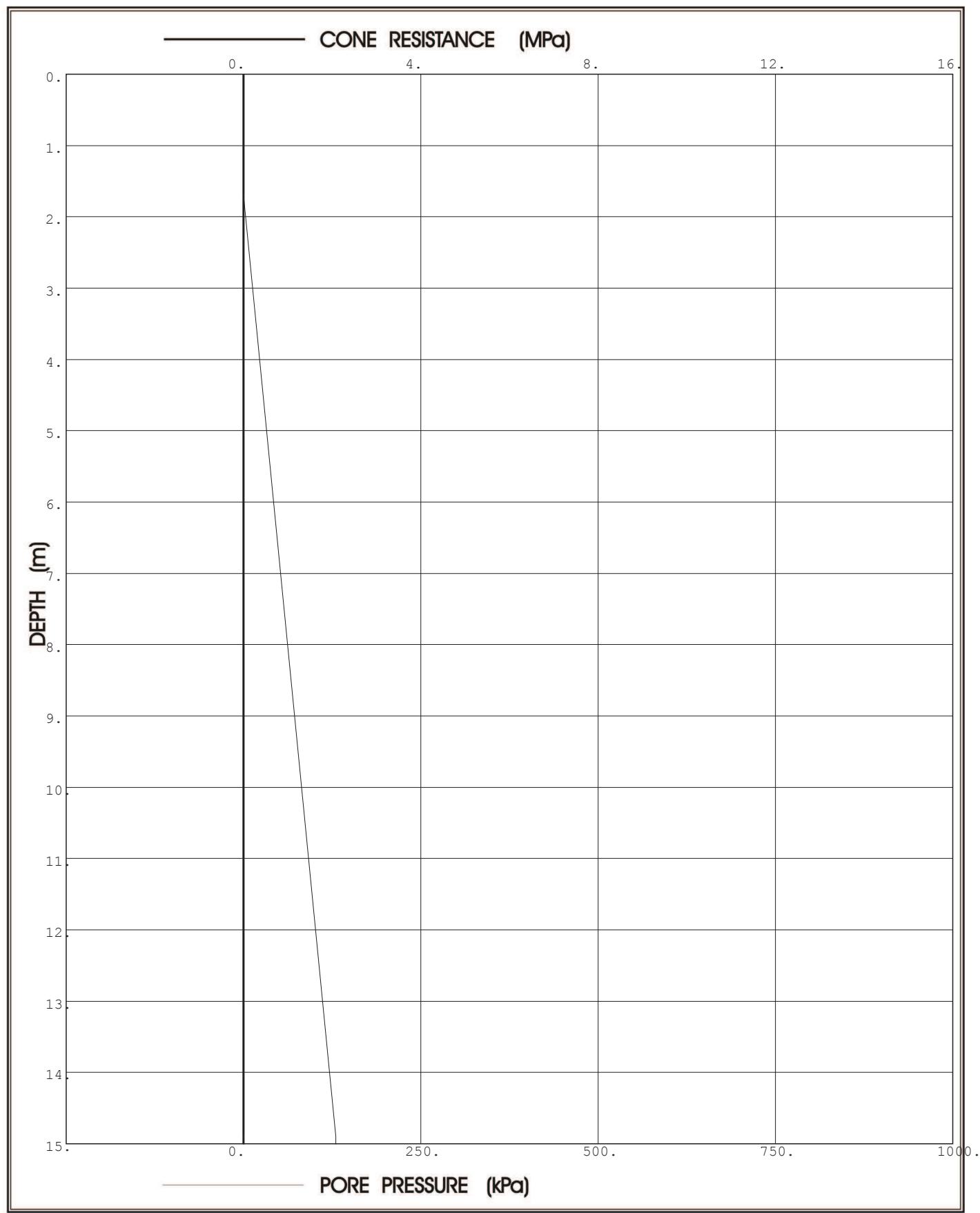
DATE: 2012/06/25

Osimo
cc

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Moreleta Park
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Page:1

Probe: GHANA



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PIEZOCONE PENETRATION TEST

PROJECT: Durban Harbour Over Water

SITE: Pier2

CPTU (RE): CPT01C

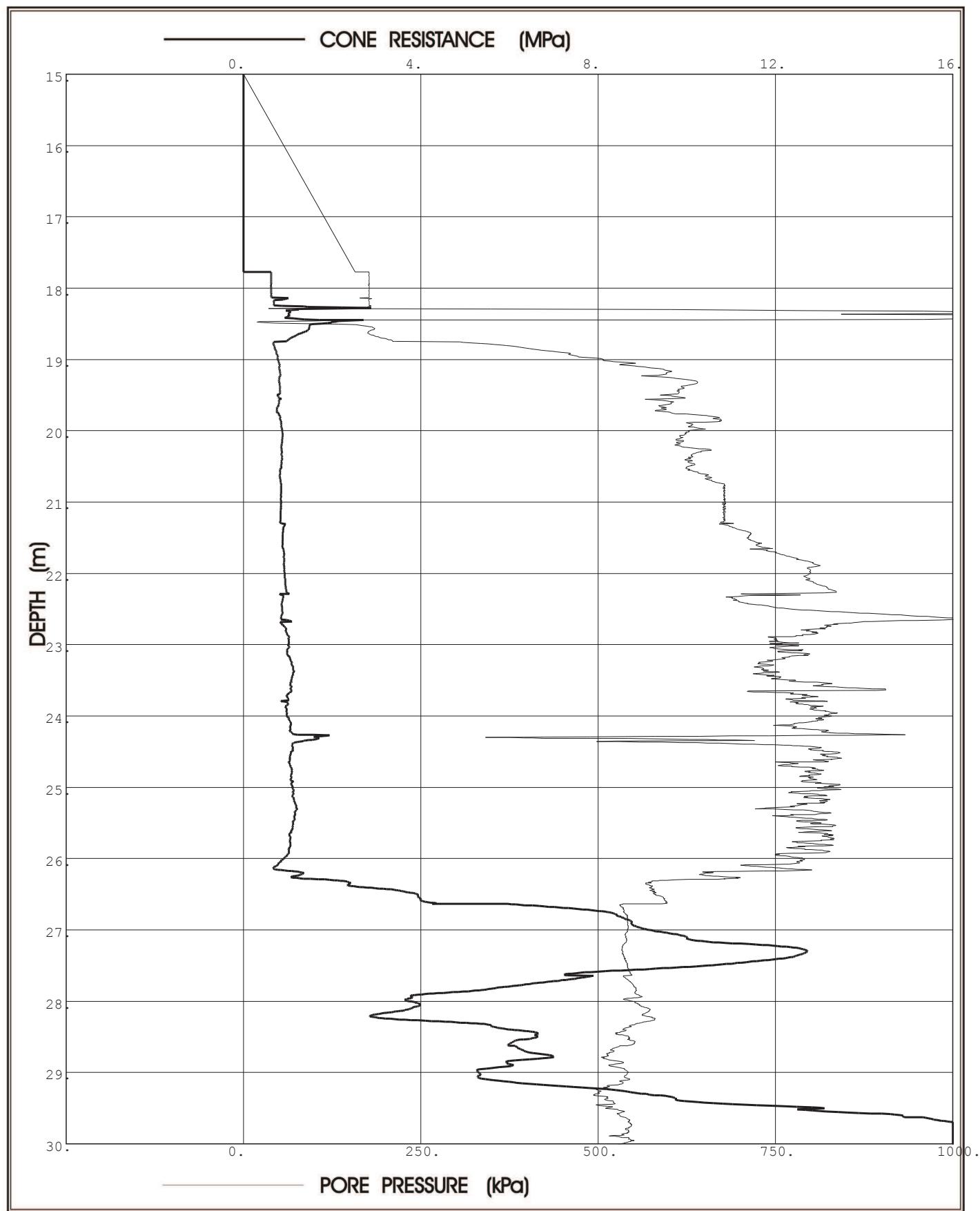
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Osimo
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Page: 2

Probe: GHANA



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PIEZOCONE PENETRATION TEST

PROJECT: Durban Harbour Over Water

SITE: Pier2

CPTU (RE): CPT01C

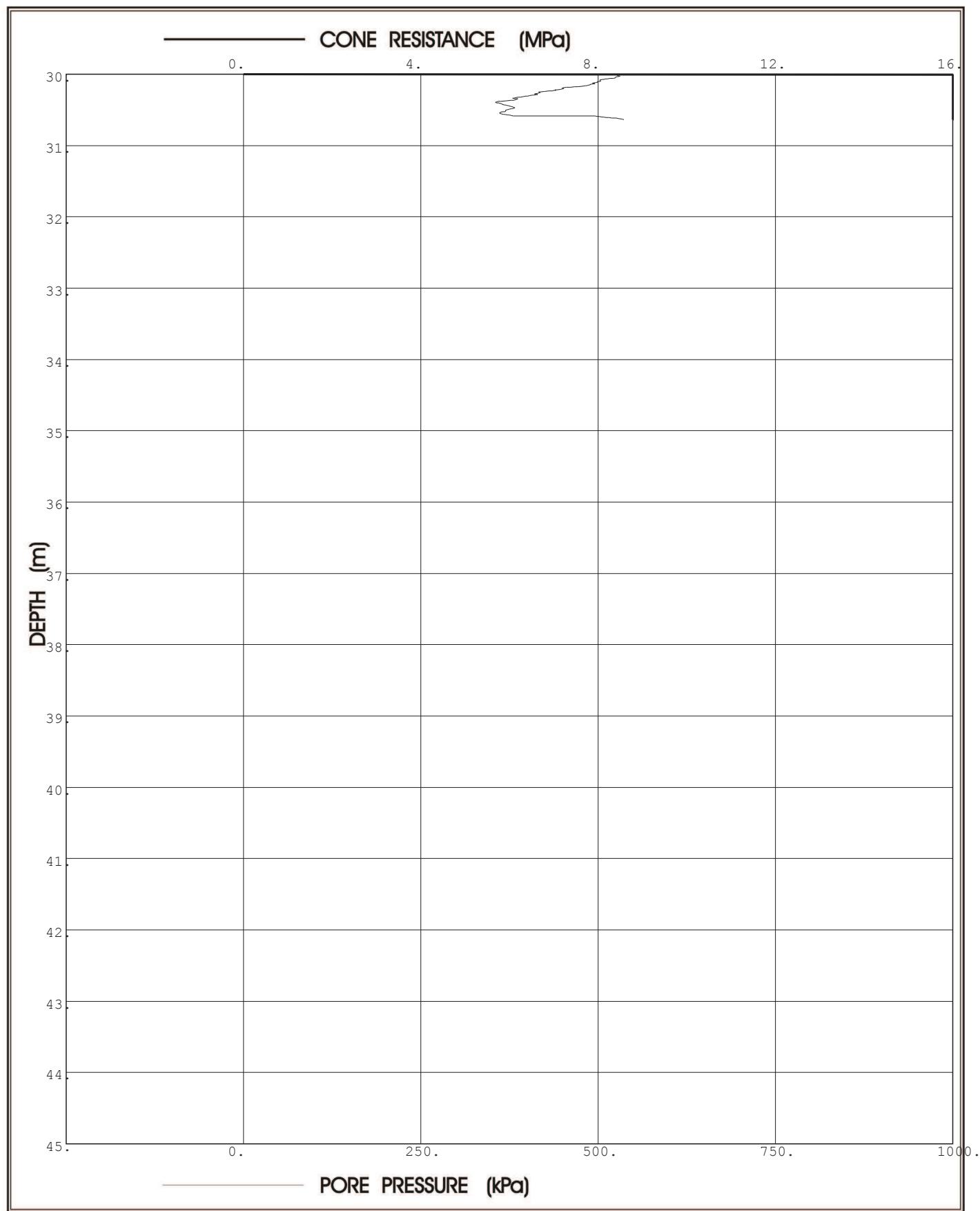
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Project: Durban Harbour Over Water

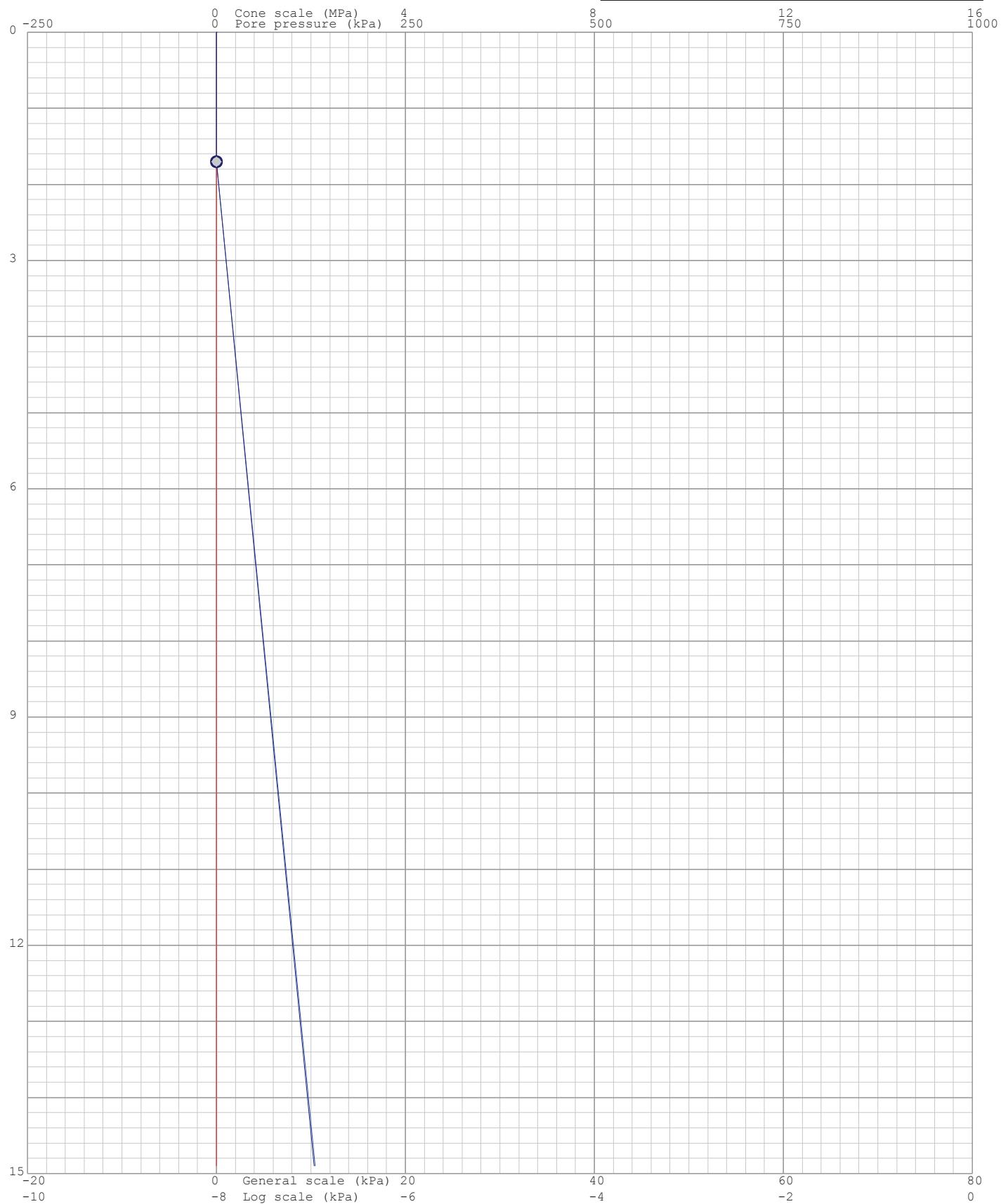
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Hole No: CPT01C
Date: 2012/06/25

Probe: GHANA

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qc (Cone Scale)
ut (Pore Pressure Scale)
uo (Pore Pressure Scale)
Diss uo (Pore Pressure Scale)



Project: Durban Harbour Over Water

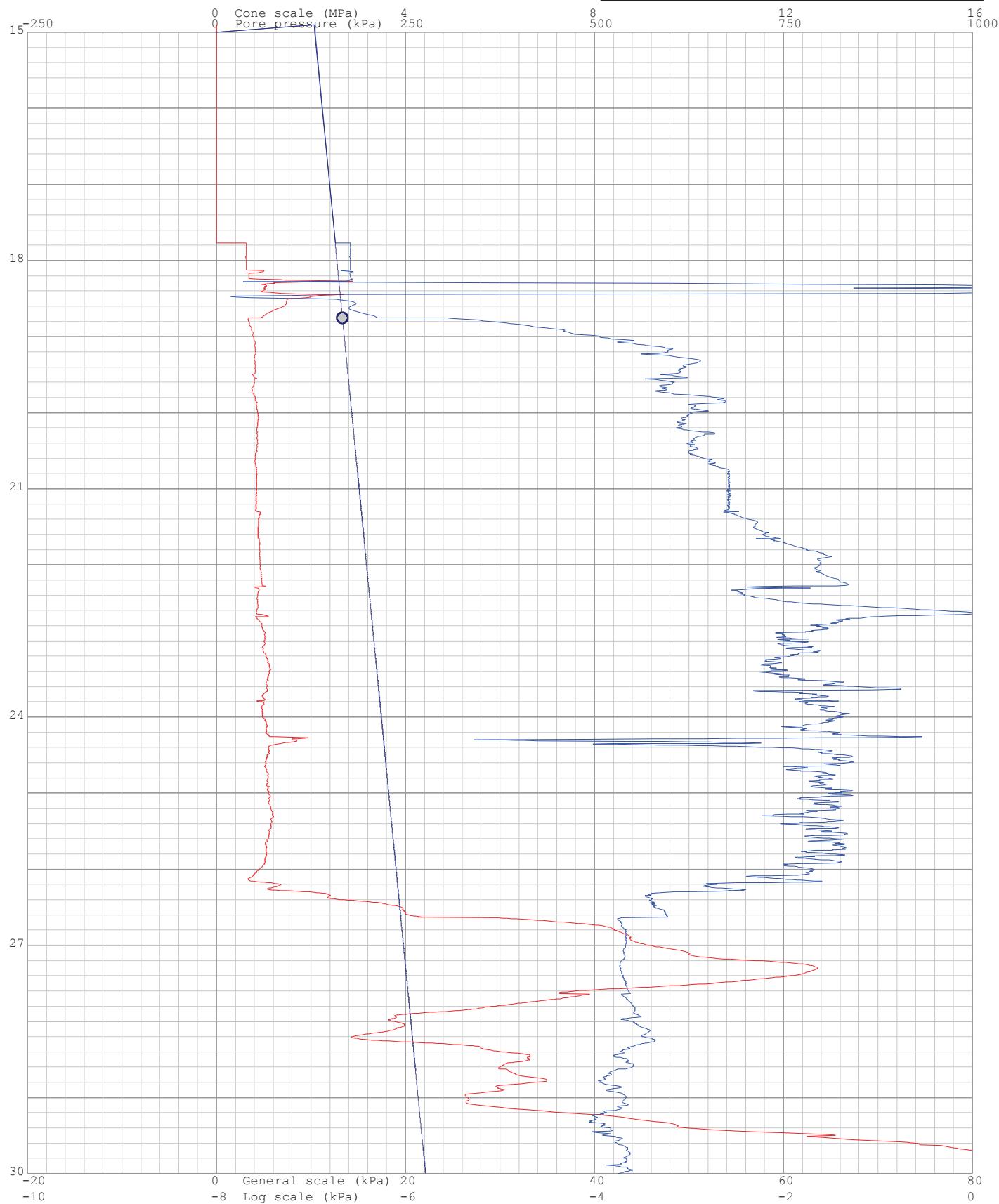
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Hole No: CPT01C
Date: 2012/06/25

Probe: GHANA

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qc (Cone Scale)
ut (Pore Pressure Scale)
uo (Pore Pressure Scale)
Diss uo (Pore Pressure Scale)



Project: Durban Harbour Over Water

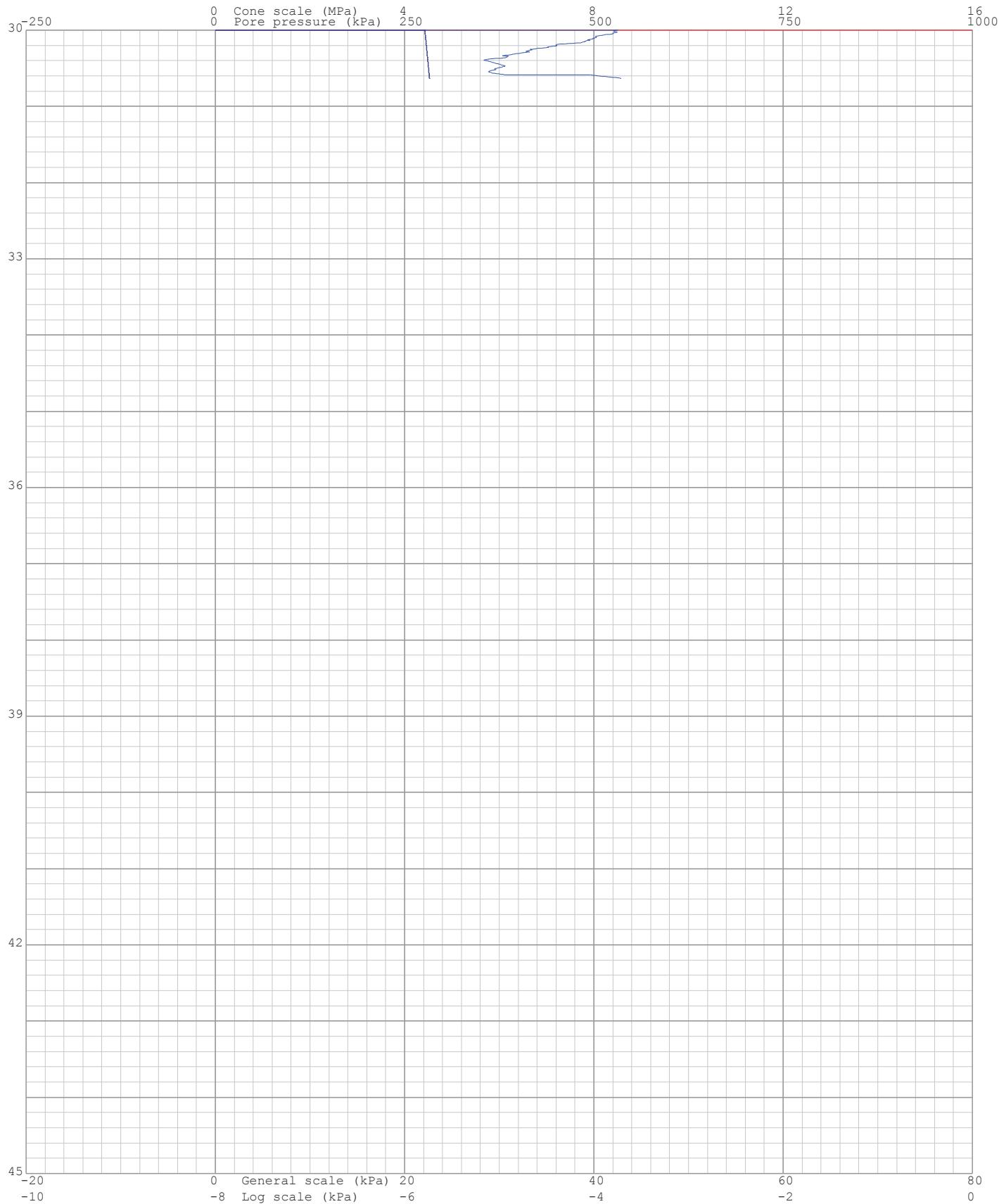
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Hole No: CPT01C
Date: 2012/06/25

Probe: GHANA

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qc (Cone Scale)
ut (Pore Pressure Scale)
uo (Pore Pressure Scale)
Diss uo (Pore Pressure Scale)



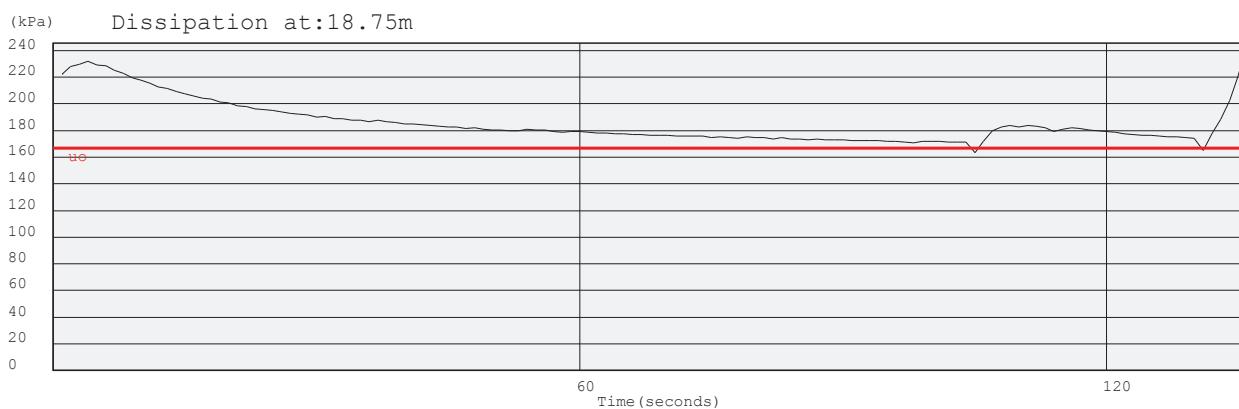
Project: Durban Harbour Over Water

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Hole: CPT01C

Date: 2012/06/25

Probe: GHANA
Page: 1



Project: Durban Harbour Over Water

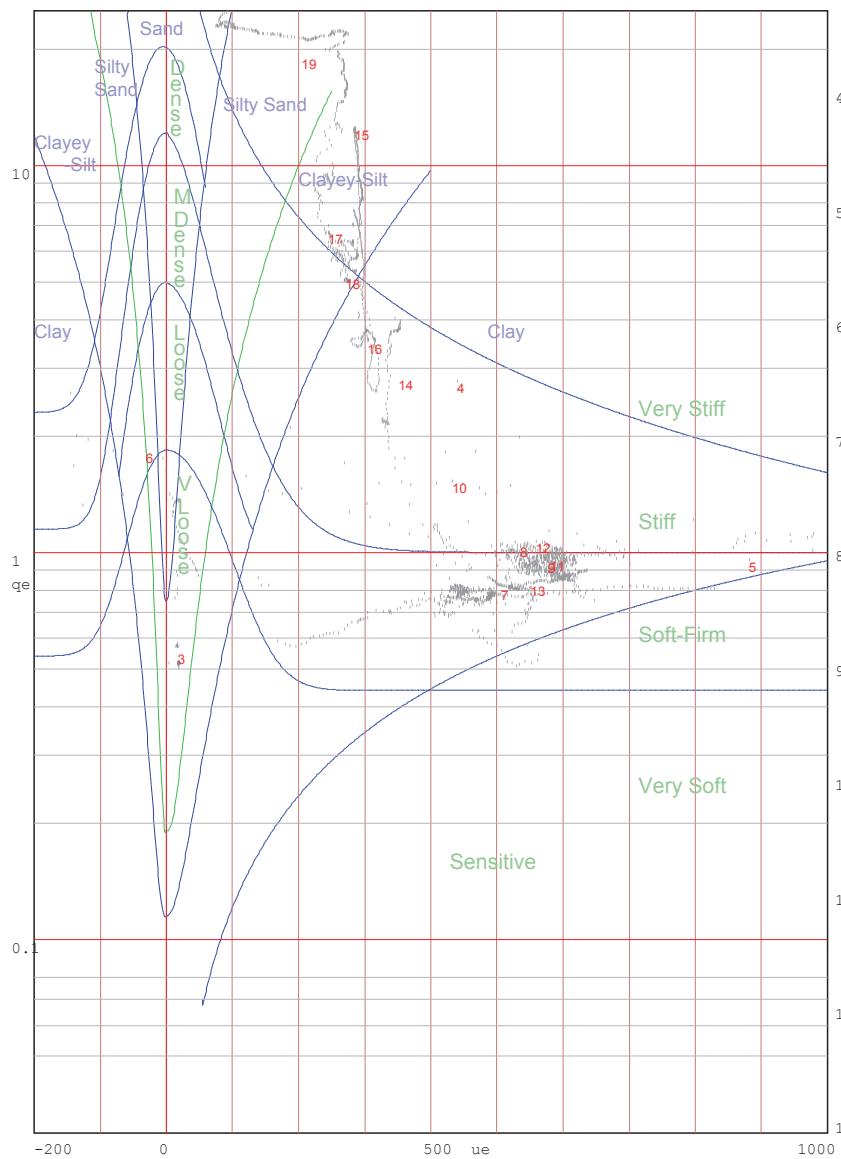
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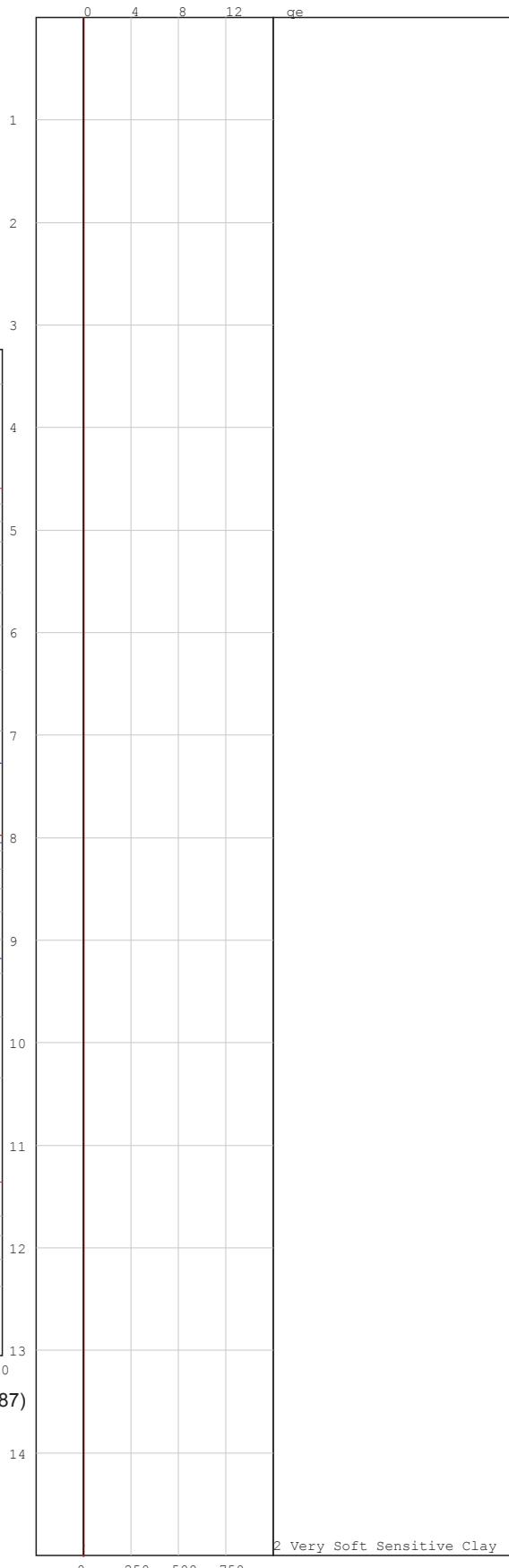
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Page:1

$q_e \text{ (MPa)} = q_t - s_{vo}$
$u_e \text{ (kPa)} = u_t - u_o$
Layers
1, 2, 3, ... Layer Number
Position on Soils ID chart



Jones and Rust Soils ID chart modified by Vermeulen and Rust(A.C.Meigh-CIRIA,1987)



Project: Durban Harbour Over Water

Site: Pier2

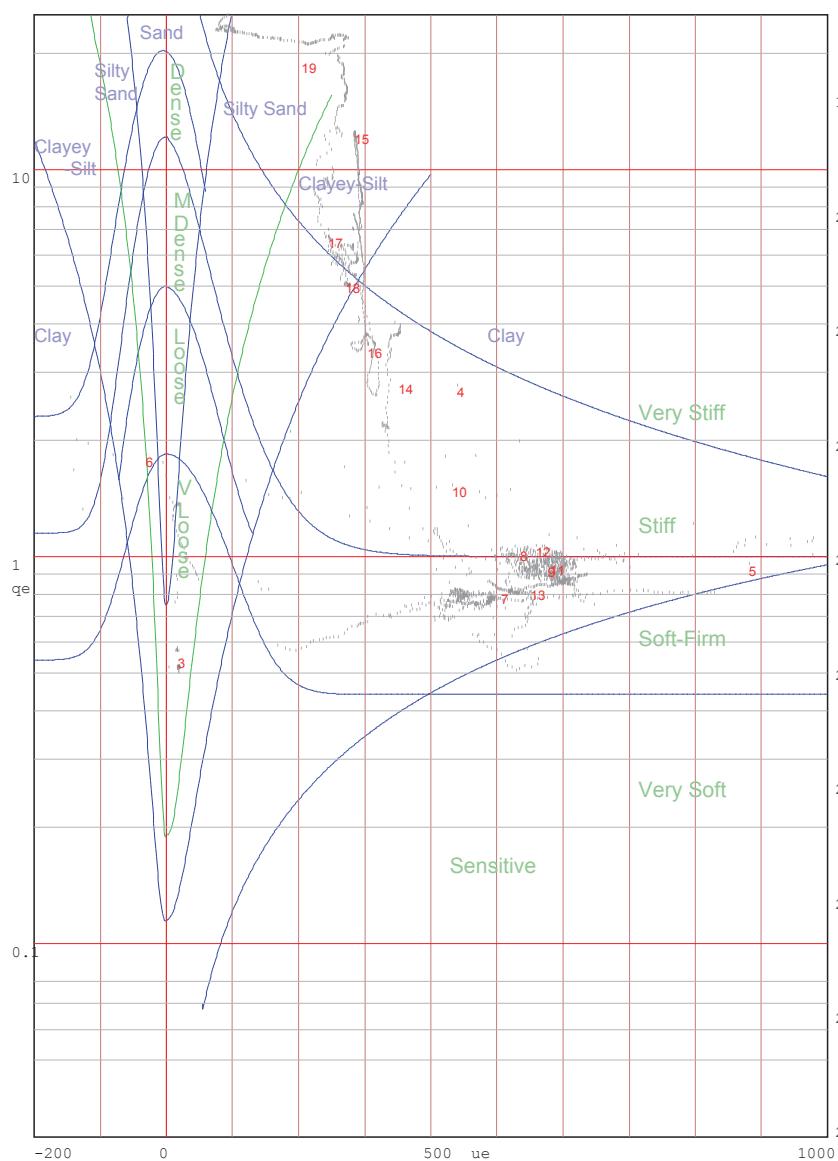
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Date: 2012/06/25

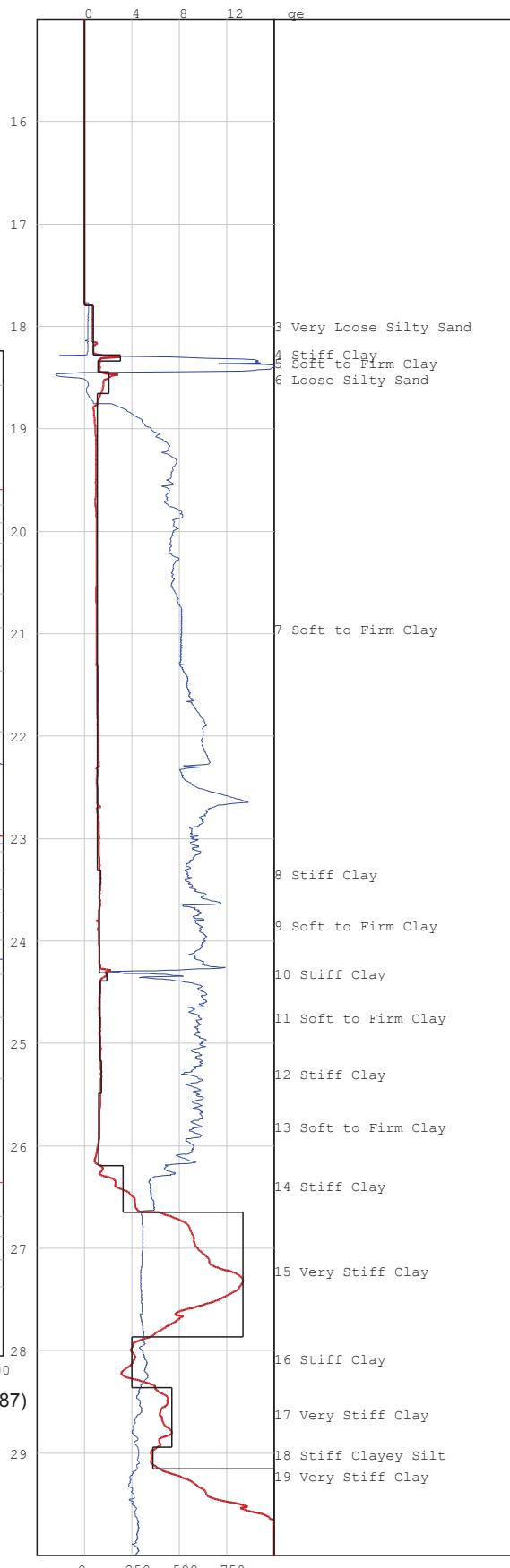
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q_e (MPa) = q_t - s_v
u_e (kPa) = u_t - u_o
Layers
1, 2, 3, ... Layer Number
Position on Soils ID chart



Jones and Rust Soils ID chart modified by Vermeulen and Rust(A.C.Meigh-CIRIA,1987)



Project: Durban Harbour Over Water

Site: Pier2

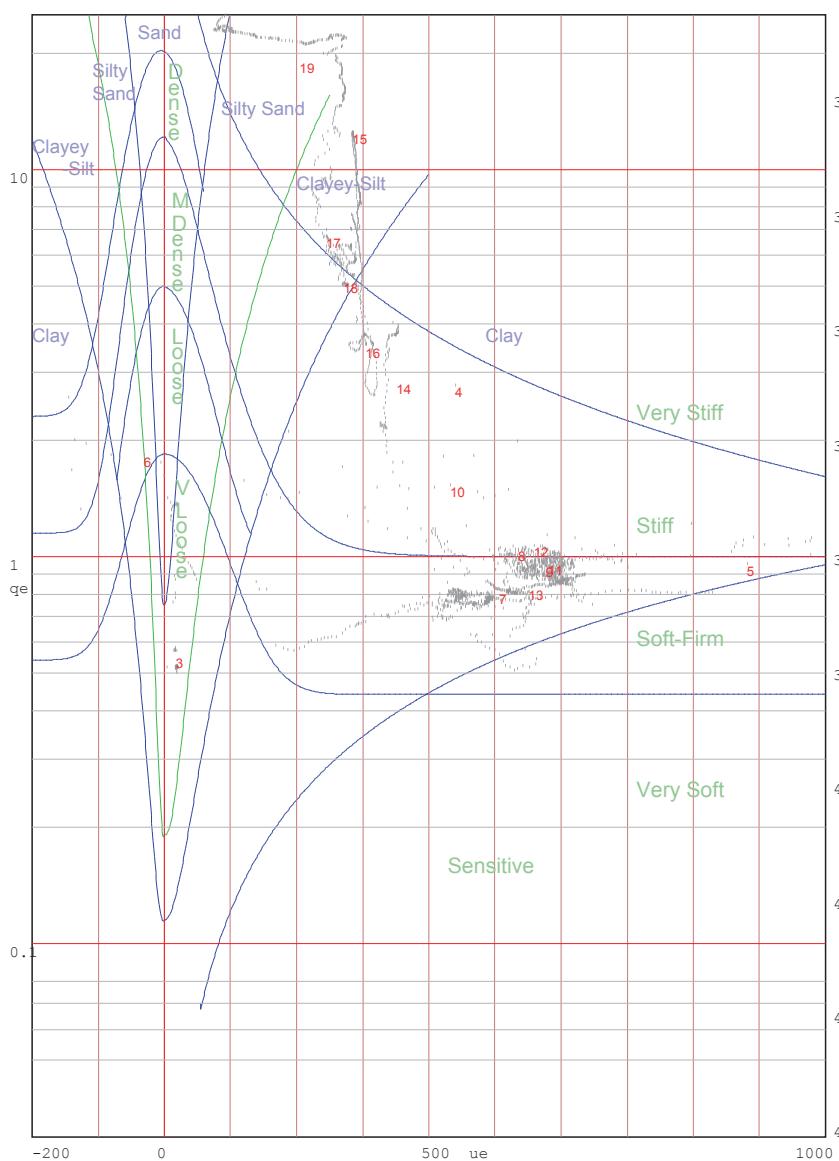
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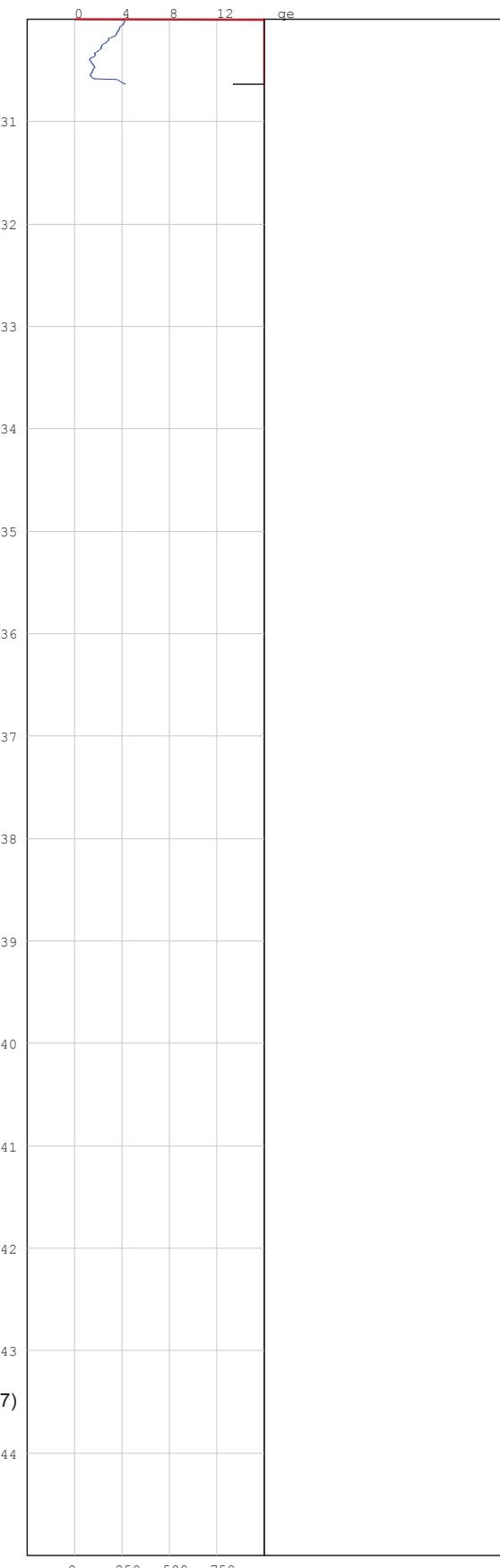
Probe: GHANA

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q_e (MPa) = $q_t - s_{vo}$
u_e (kPa) = $u_t - u_o$
Layers
1, 2, 3, ... Layer Number
Position on Soils ID chart



Jones and Rust Soils ID chart modified by Vermeulen and Rust(A.C.Meigh-CIRIA,1987)



PIEZOCONE PENETRATION TEST

PROJECT: Durban Harbour Over Water

SITE: Pier2

CPTU (RE): CPT02

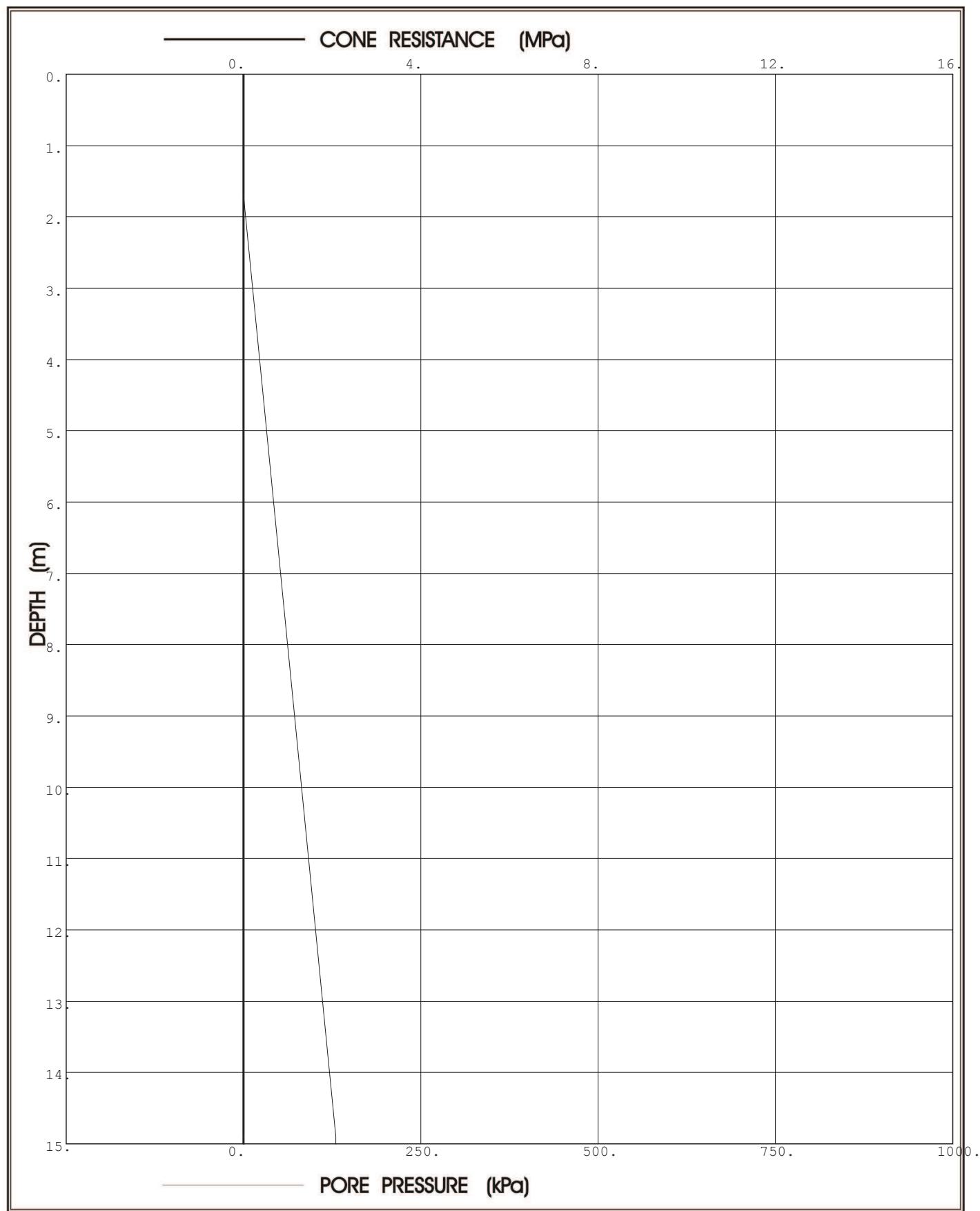
DATE: 2012/06/20

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Page:1

Probe: GHANA



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PIEZOCONE PENETRATION TEST

PROJECT: Durban Harbour Over Water

SITE: Pier2

CPTU (RE): CPT02

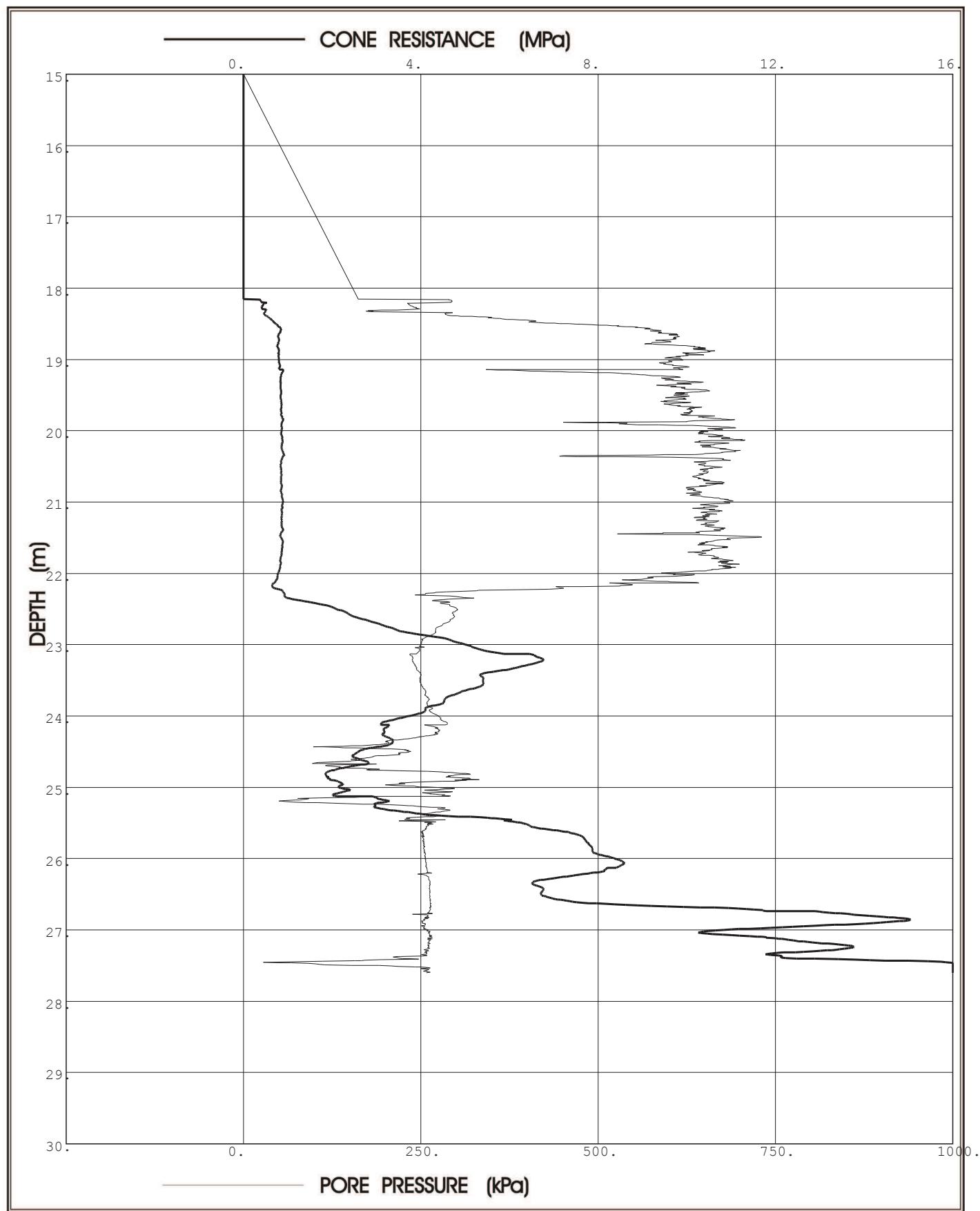
DATE: 2012/06/20

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Page: 2

Probe: GHANA



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Project: Durban Harbour Over Water

Site: Pier2

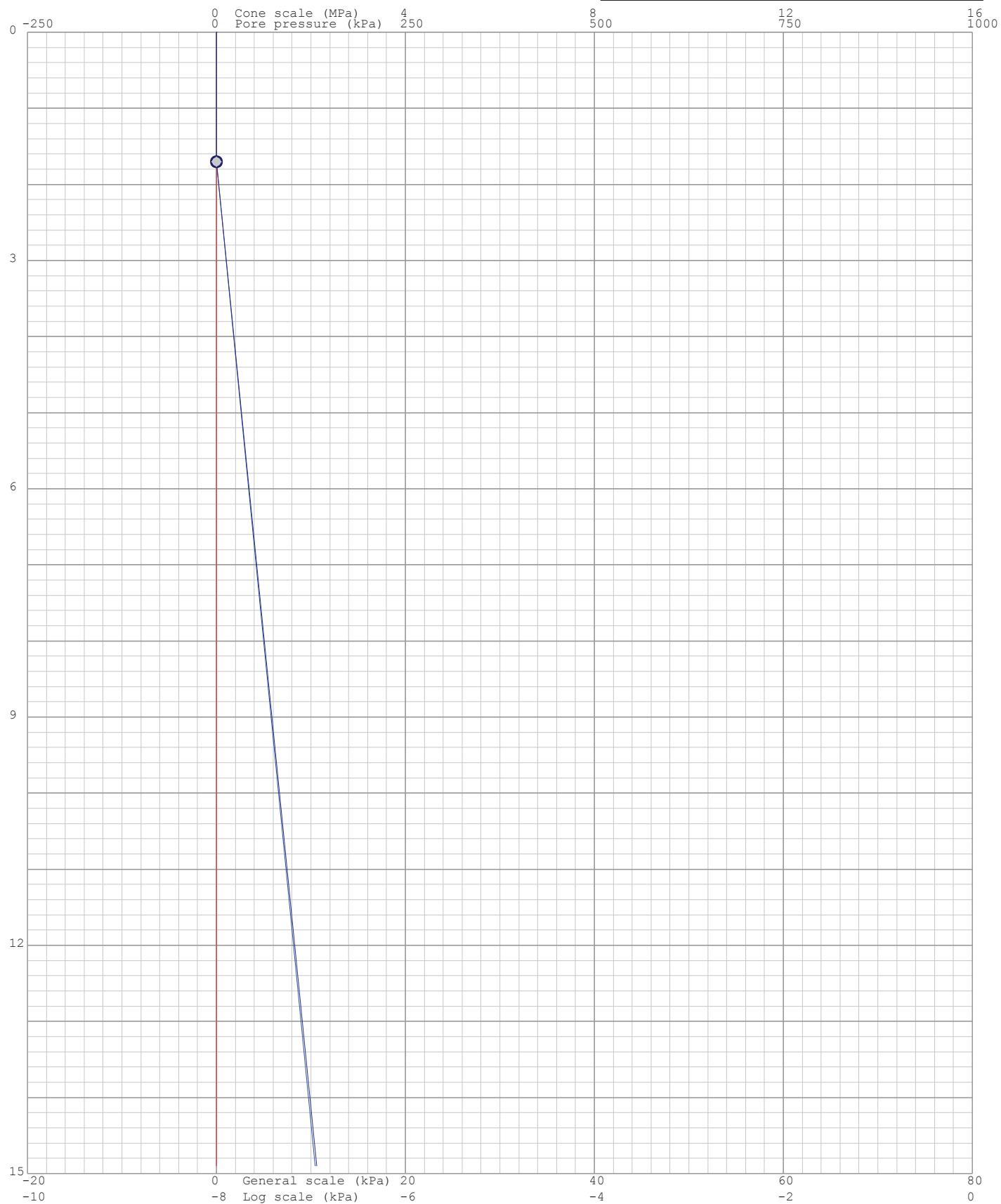
Hole No: CPT02

Date: 2012/06/20

Probe: GHANA

Page:1

qc (Cone Scale)
ut (Pore Pressure Scale)
uo (Pore Pressure Scale)
Diss uo (Pore Pressure Scale)



Project: Durban Harbour Over Water

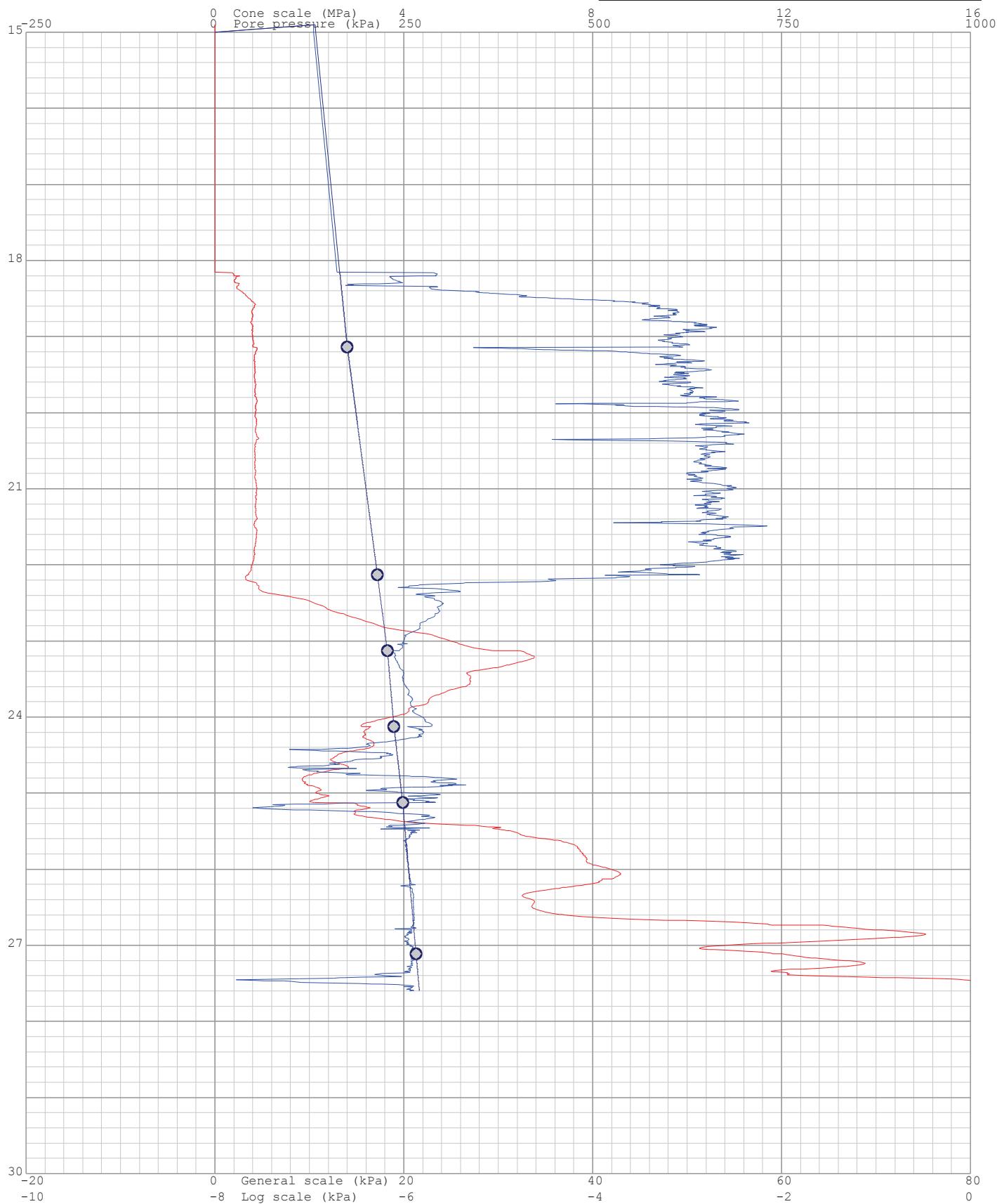
Site: Pier2

Hole No: CPT02
Date: 2012/06/20

Probe: GHANA

Page: 2

qc (Cone Scale)
ut (Pore Pressure Scale)
uo (Pore Pressure Scale)
Diss uo (Pore Pressure Scale)



Project: Durban Harbour Over Water

Site: Pier2

Hole: CPT02

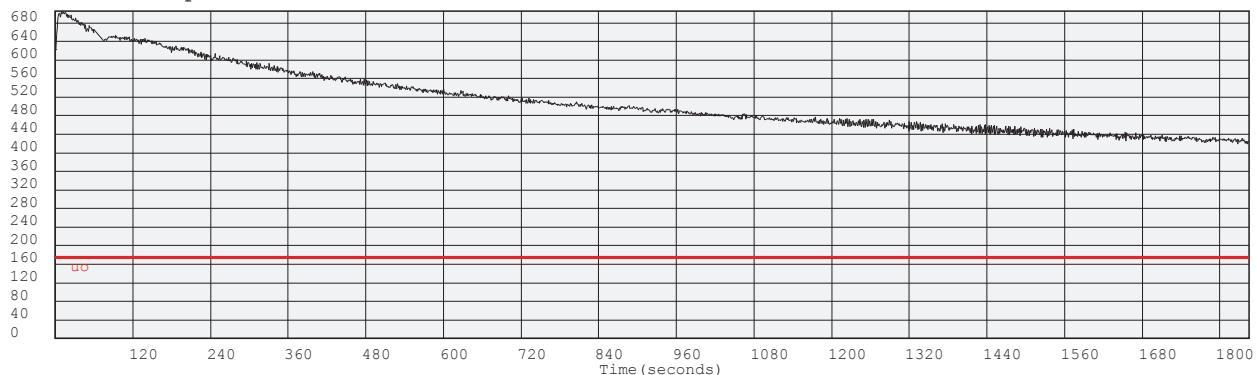
Date: 2012/06/20

Probe: GHANA
Page: 1

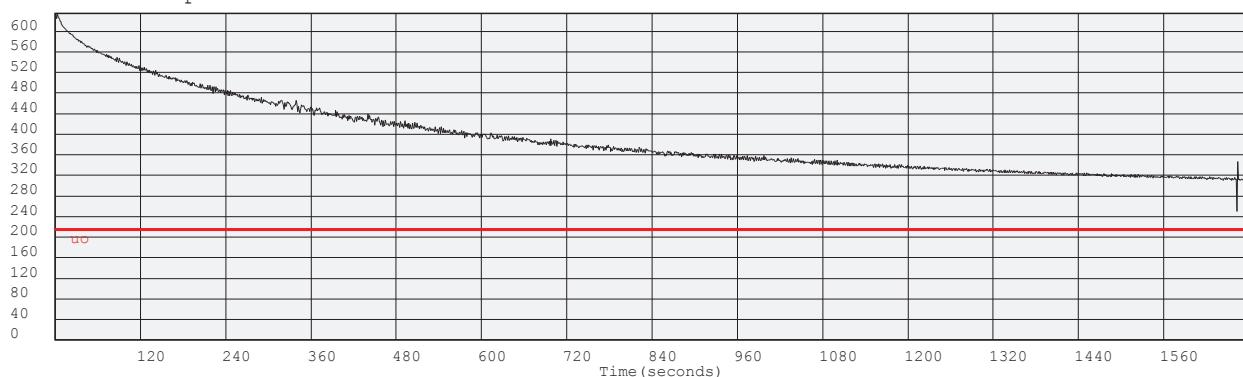
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(kPa) Dissipation at:19.14m



(kPa) Dissipation at:22.132m



(kPa) Dissipation at:23.126m



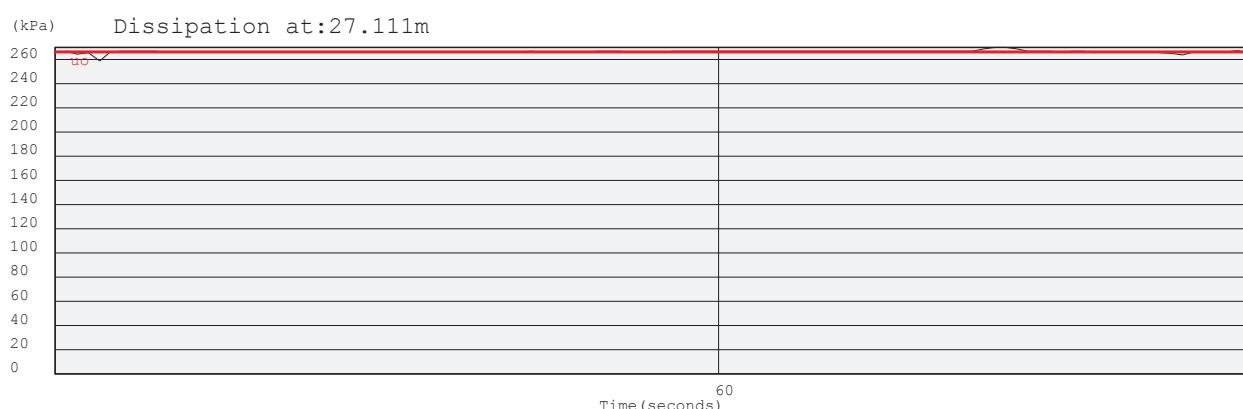
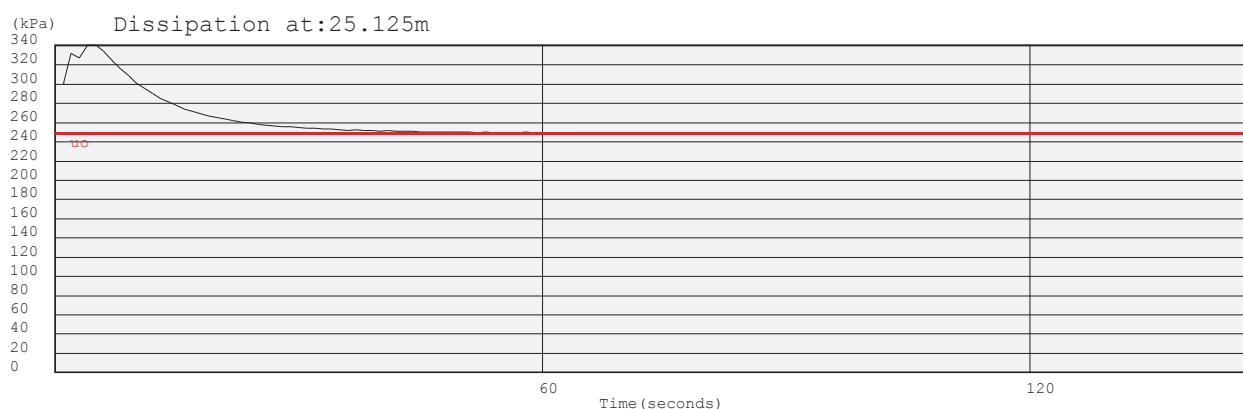
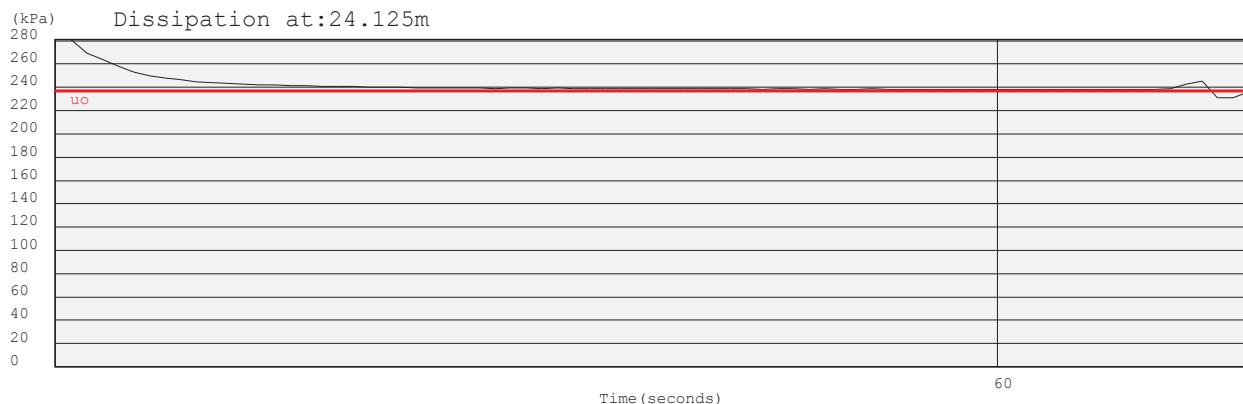
Project: Durban Harbour Over Water

Site: Pier2

Hole: CPT02

Date: 2012/06/20

Probe: GHANA
Page: 2



Project: Durban Harbour Over Water

Site: Pier2

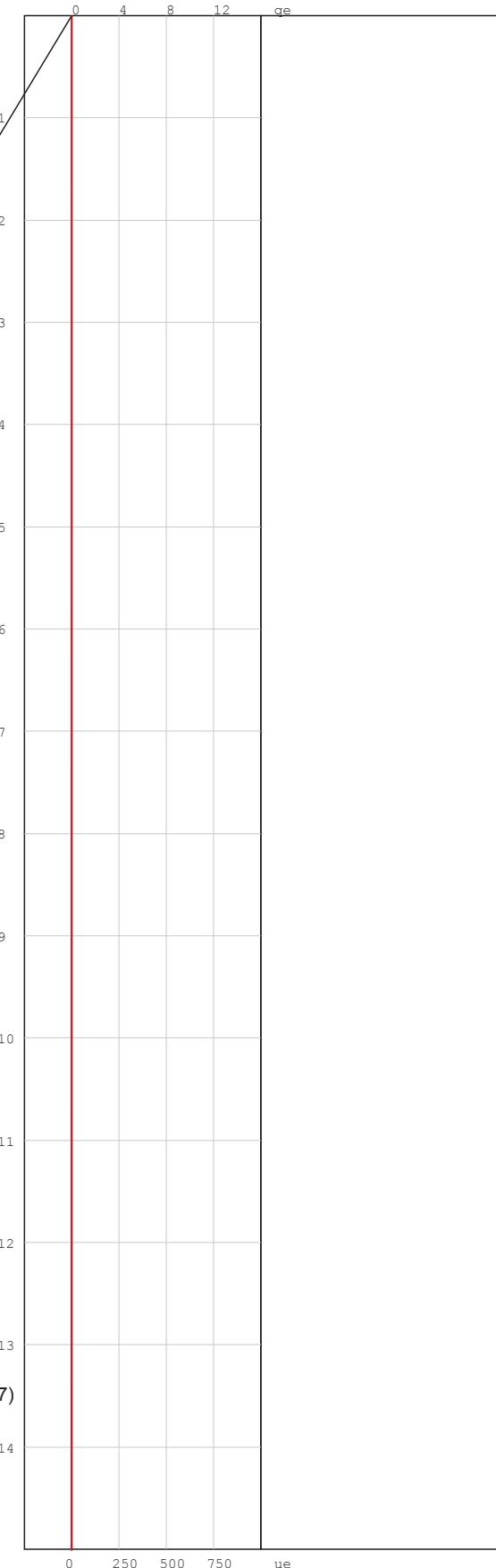
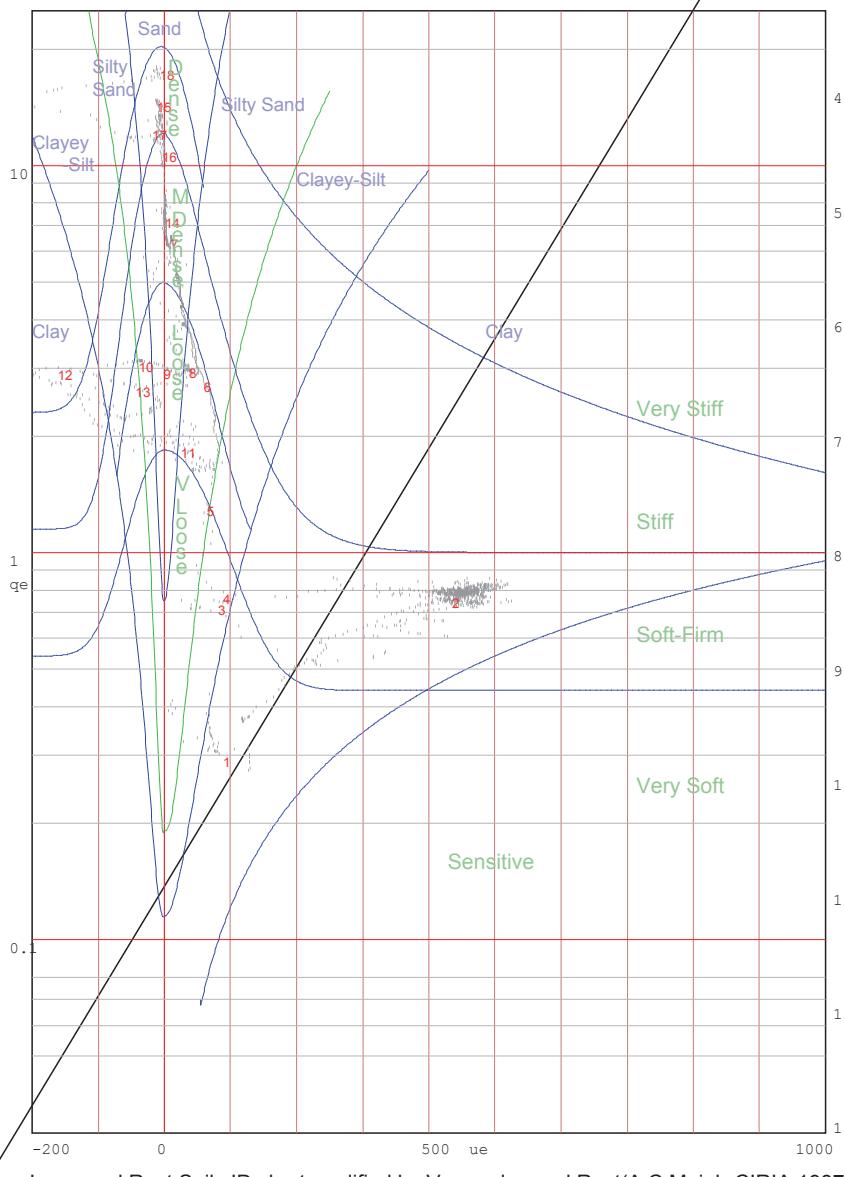
Hole No: CPT02

Date: 2012/06/20

Probe: GHANA

Page:1

qe (MPa) = qt - svo
ue (kPa) = ut - uo
Layers
1, 2, 3, ... Layer Number
Position on Soils ID chart



Project: Durban Harbour Over Water

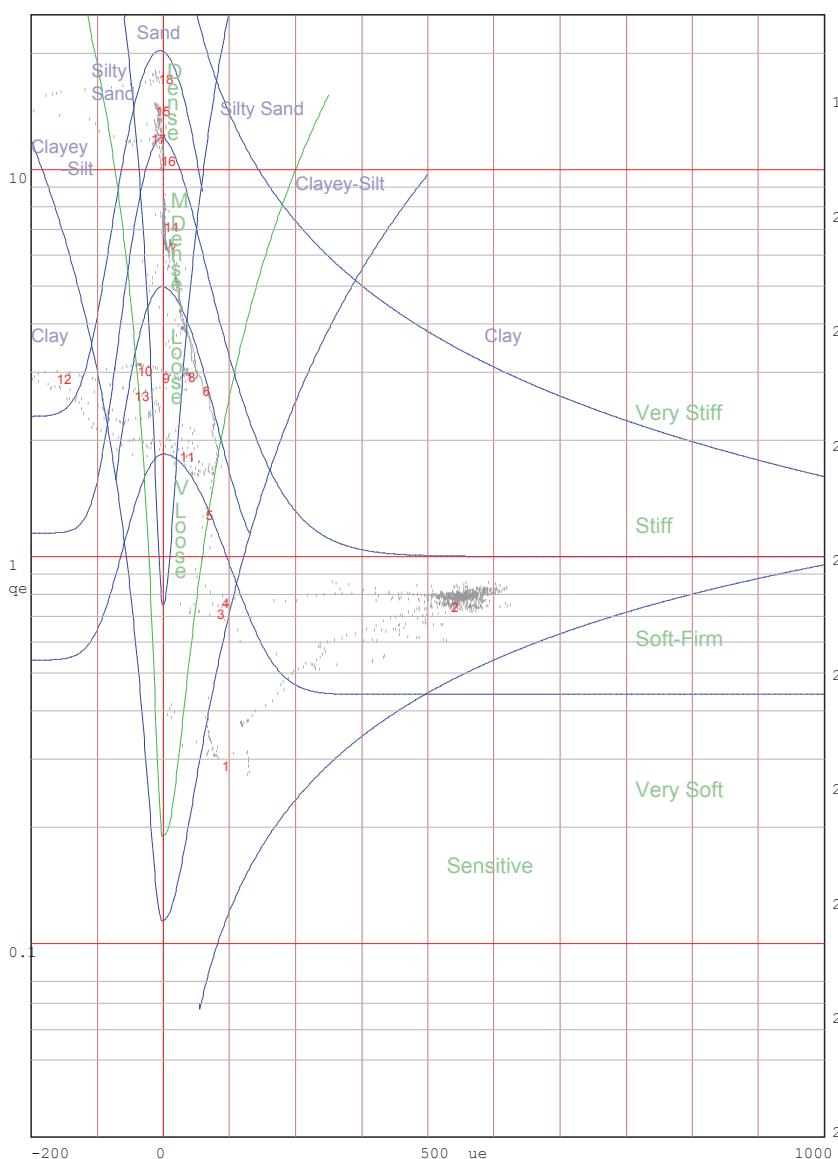
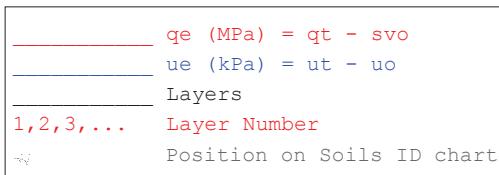
Site: Pier2

Hole No: CPT02

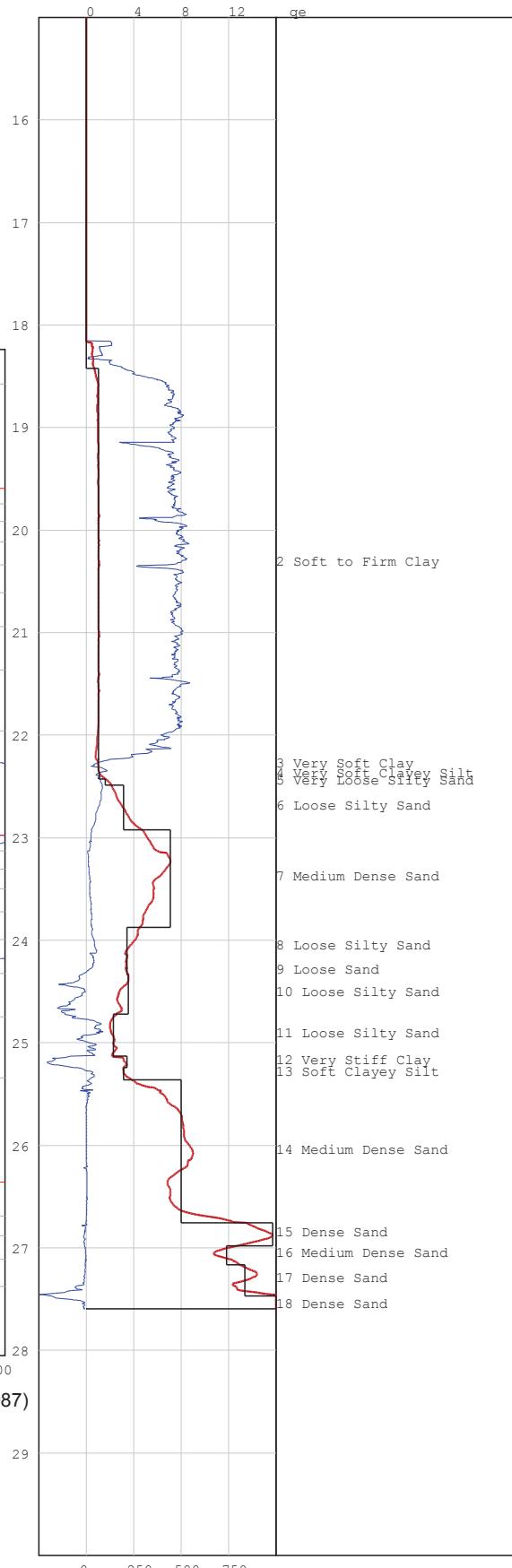
Date: 2012/06/20

Probe: GHANA

Page:2



Jones and Rust Soils ID chart modified by Vermeulen and Rust(A.C.Meigh-CIRIA,1987)



PIEZOCONE PENETRATION TEST

PROJECT: Durban Harbour Over Water

SITE: Pier2

CPTU (RE): CPT03

DATE: 2012-06-18

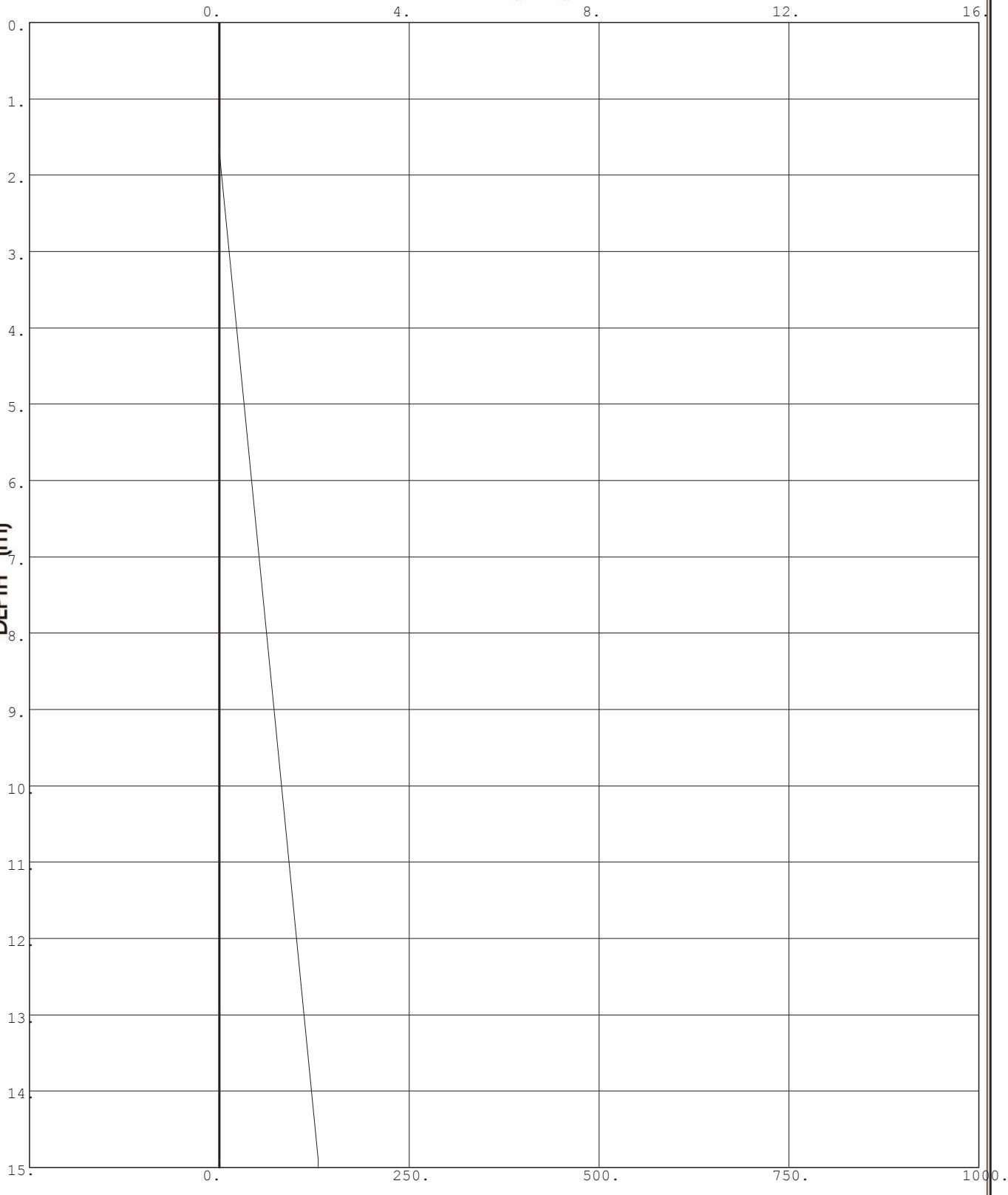
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Page:1

Probe: GHANA

CONE RESISTANCE (MPa)



PORE PRESSURE (kPa)

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PIEZOCONE PENETRATION TEST

PROJECT: Durban Harbour Over Water

SITE: Pier2

CPTU (RE): CPT03

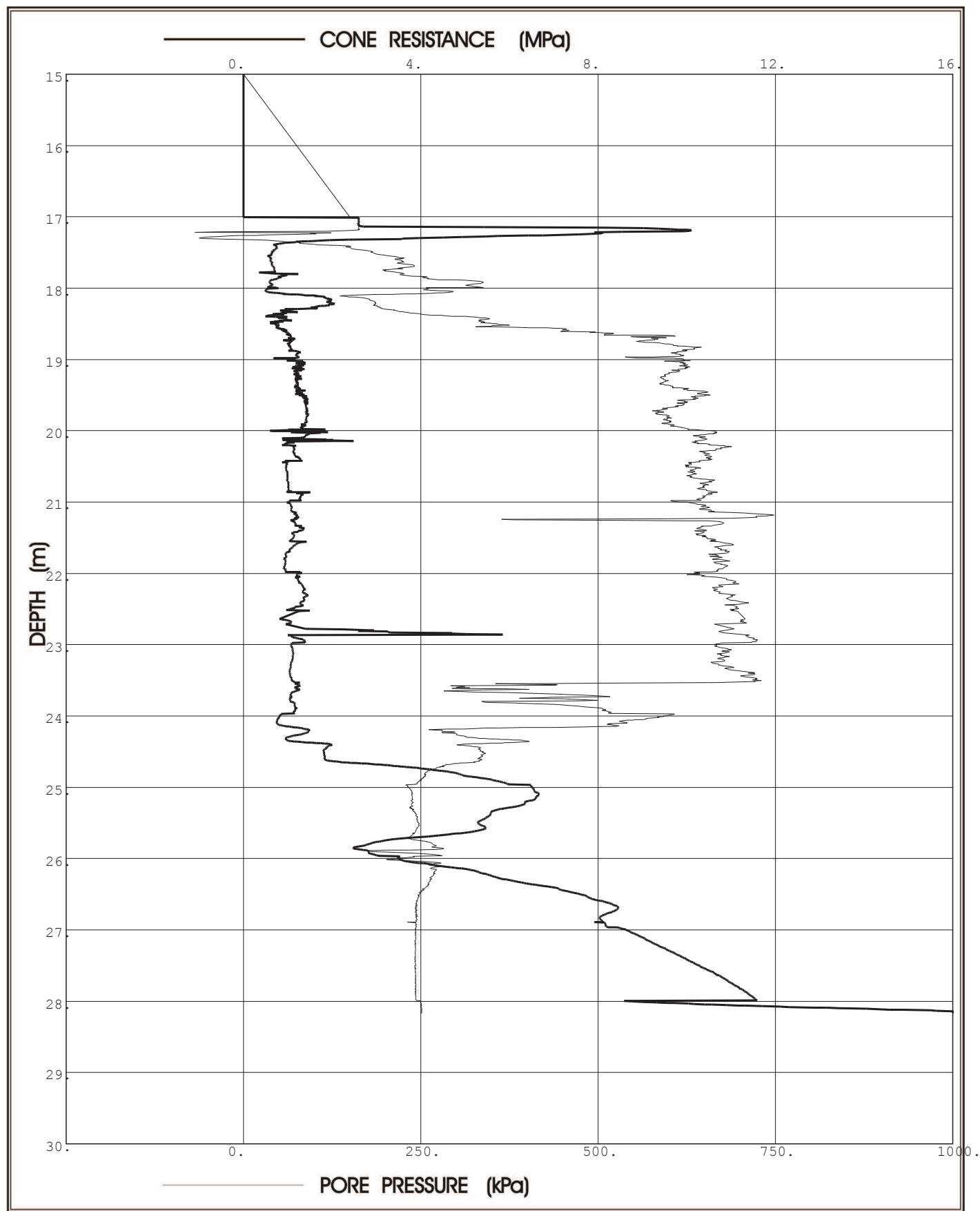
DATE: 2012-06-18

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Page: 2

Probe: GHANA



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Project: Durban Harbour Over Water

Site: Pier2

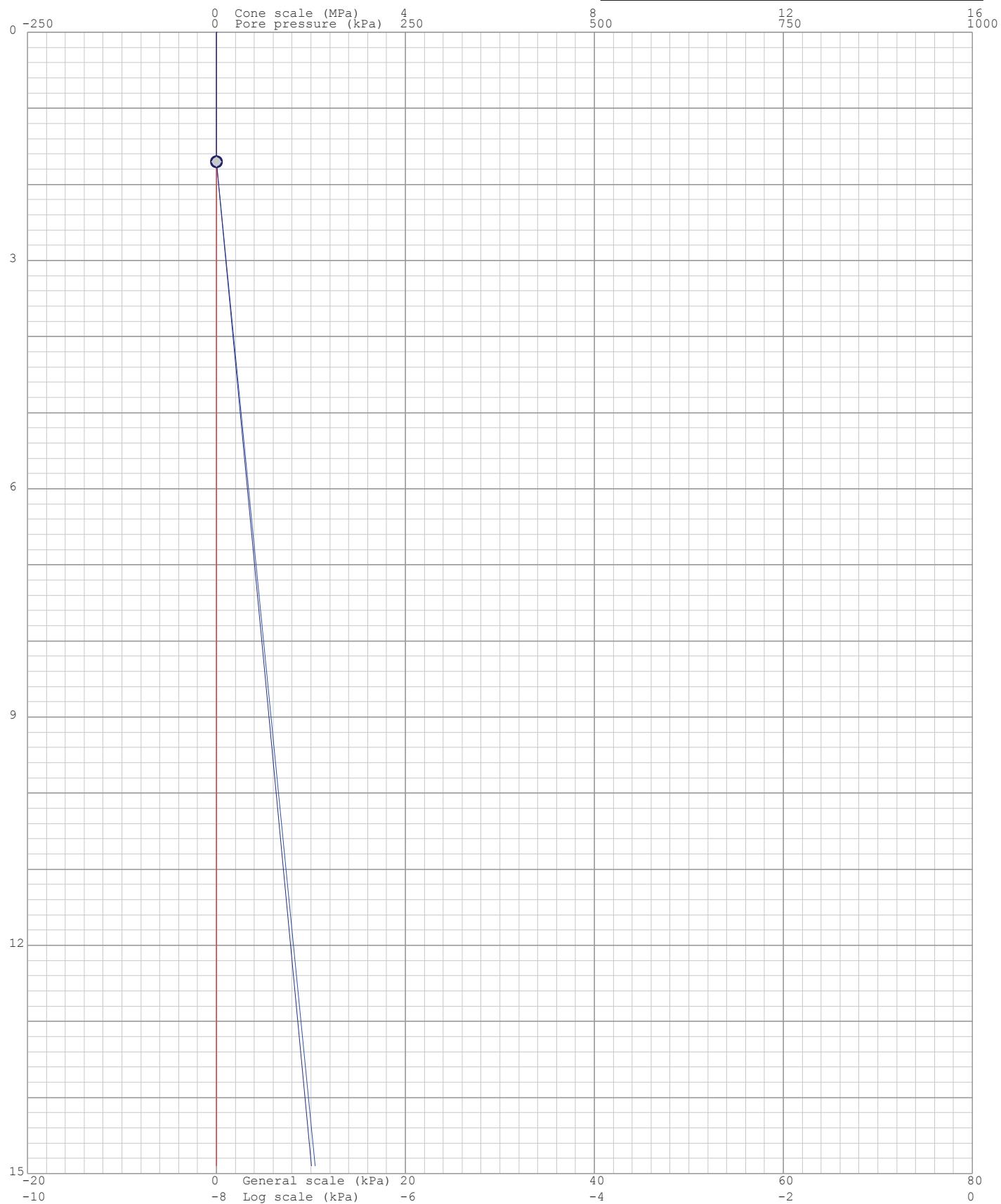
Hole No: CPT03

Date: 2012-06-18

Probe: GHANA

Page:1

qc (Cone Scale)
ut (Pore Pressure Scale)
uo (Pore Pressure Scale)
Diss uo (Pore Pressure Scale)



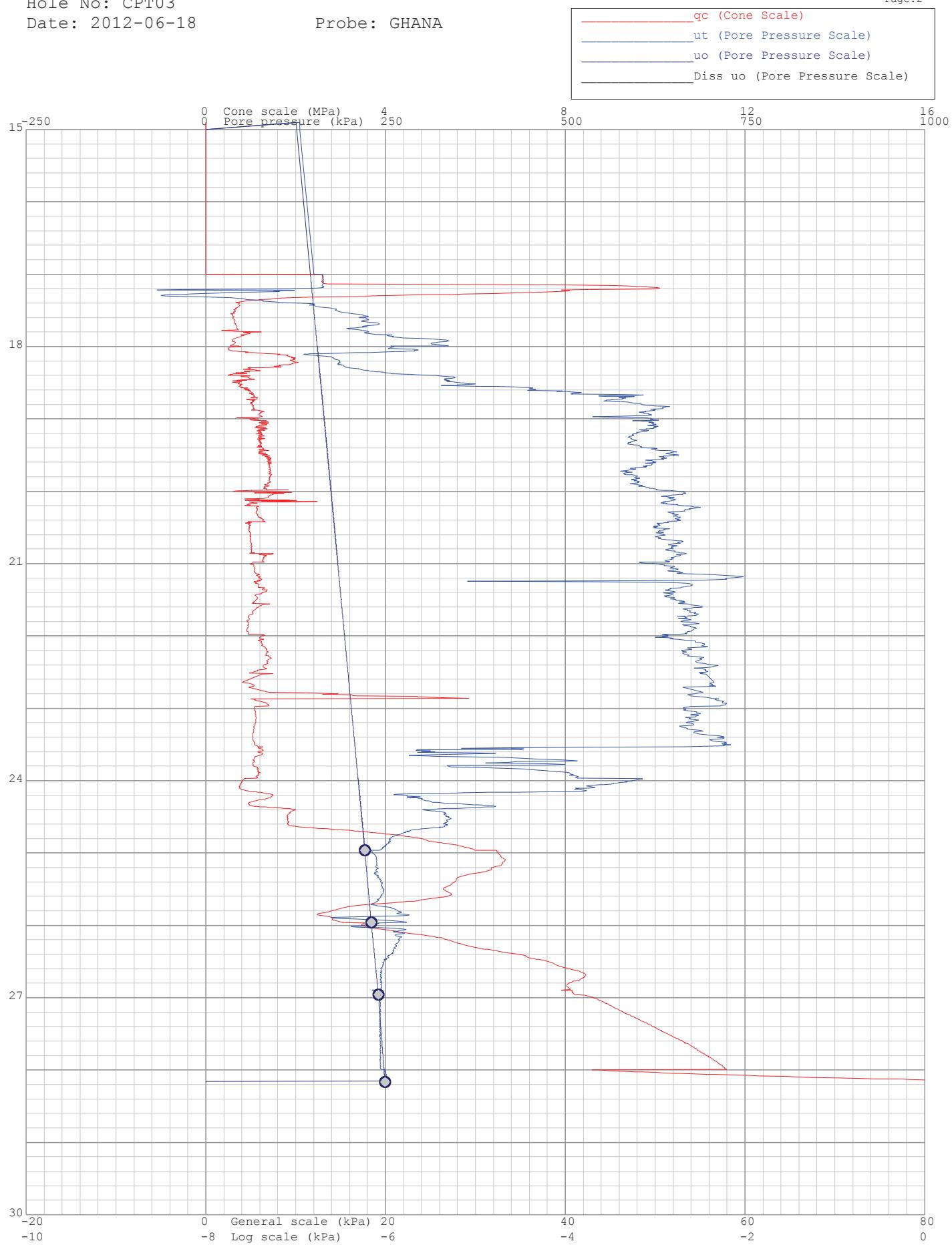
Project: Durban Harbour Over Water

Site: Pier2

Hole No: CPT03
Date: 2012-06-18

Probe: GHANA

Page: 2



Project: Durban Harbour Over Water

Site: Pier2

Hole: CPT03

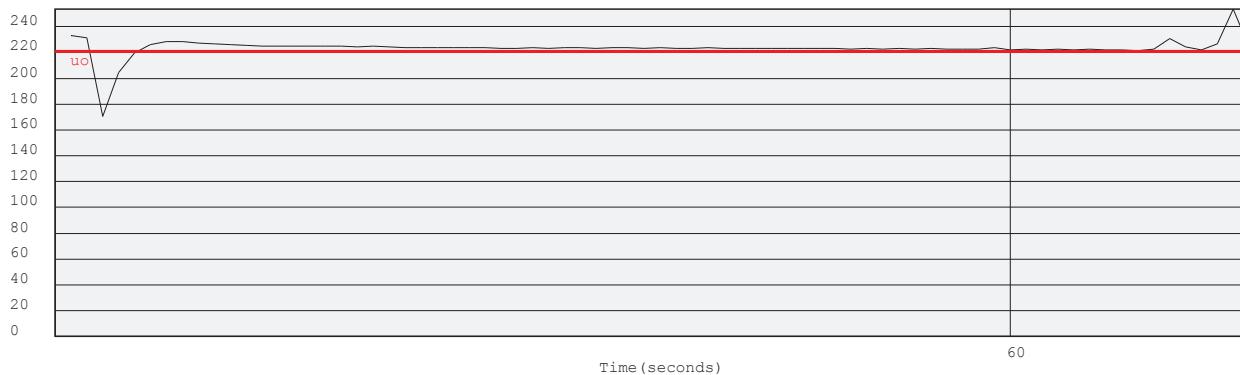
Date: 2012-06-18

Probe: GHANA
Page: 1

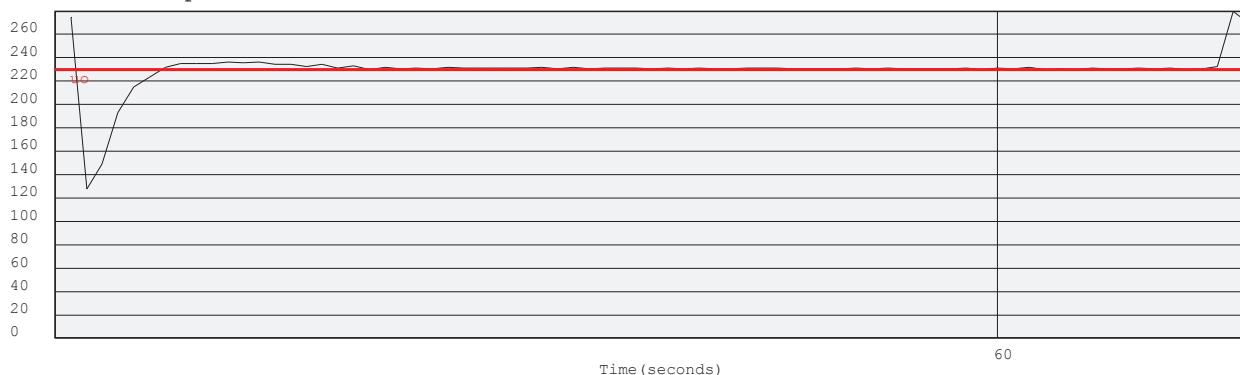
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(kPa) Dissipation at:24.96m



(kPa) Dissipation at:25.963m



(kPa) Dissipation at:26.96m



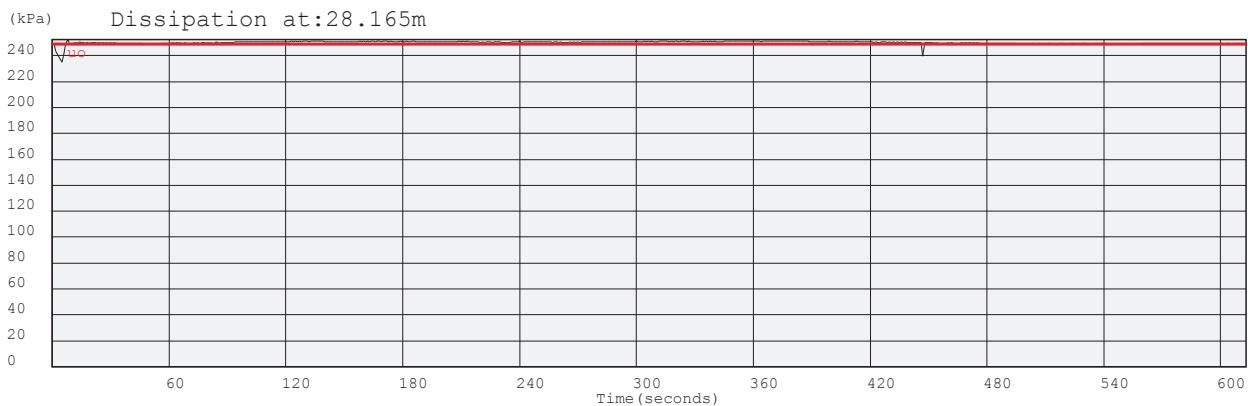
Project: Durban Harbour Over Water

Site: Pier2

Hole: CPT03

Date: 2012-06-18

Probe: GHANA
Page: 2



Project: Durban Harbour Over Water

Site: Pier2

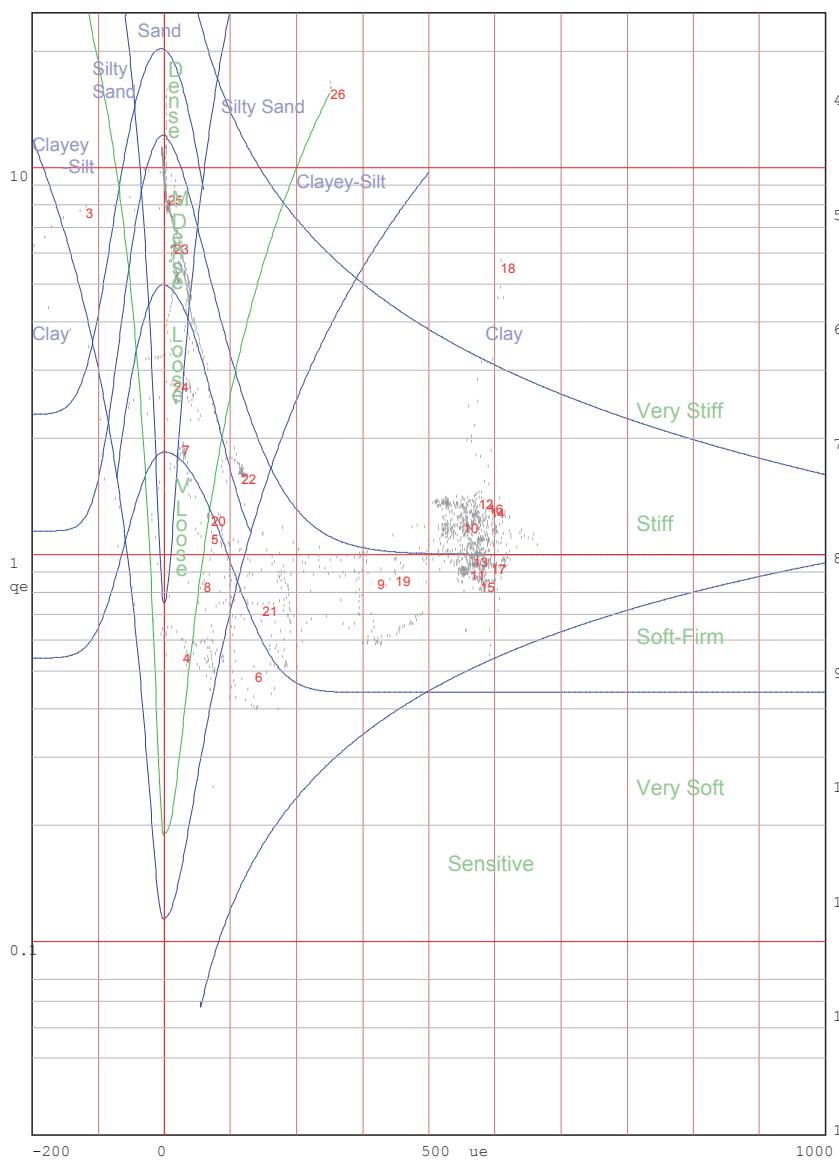
Hole No: CPT03

Date: 2012-06-18

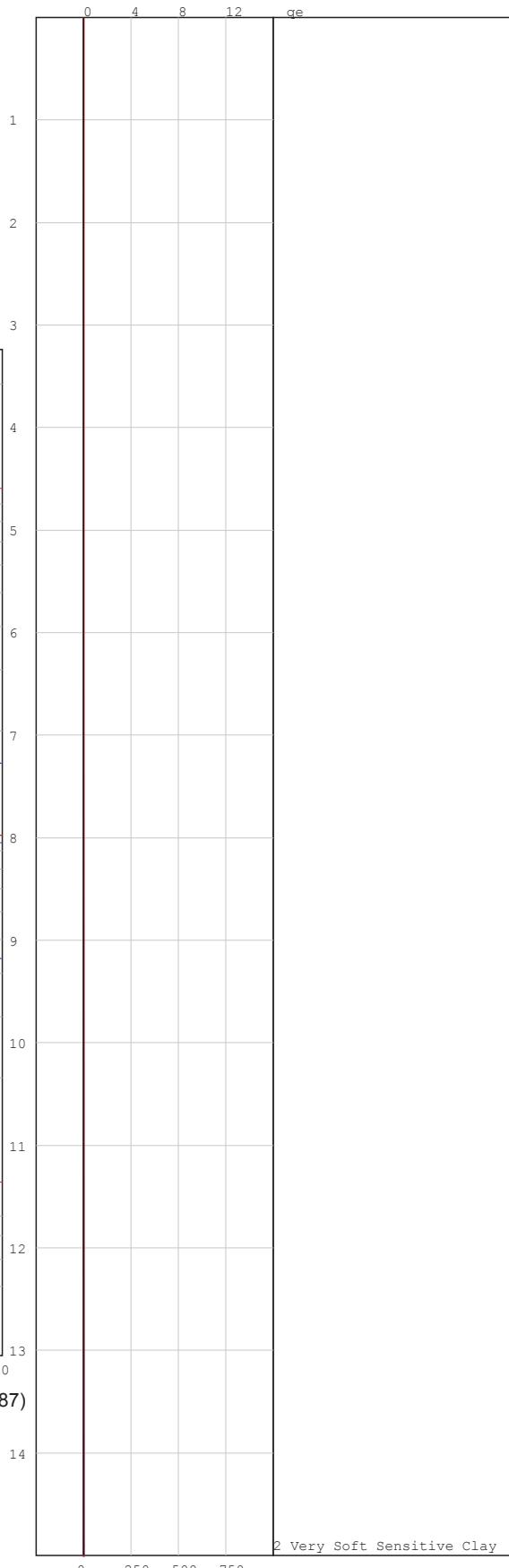
Probe: GHANA

Page:1

$q_e \text{ (MPa)} = q_t - s_{vo}$
$u_e \text{ (kPa)} = u_t - u_o$
Layers
1, 2, 3, ... Layer Number
Position on Soils ID chart



Jones and Rust Soils ID chart modified by Vermeulen and Rust(A.C.Meigh-CIRIA,1987)



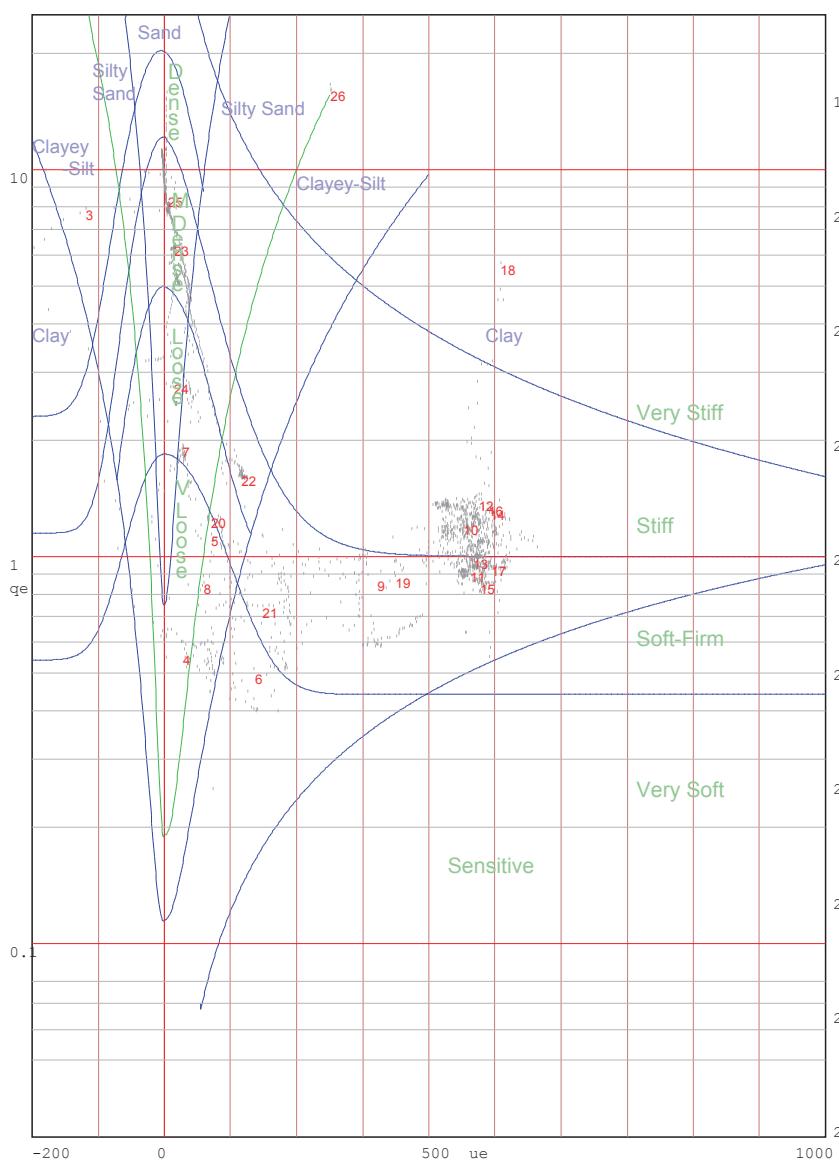
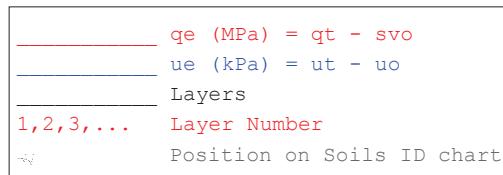
Project: Durban Harbour Over Water

Site: Pier2

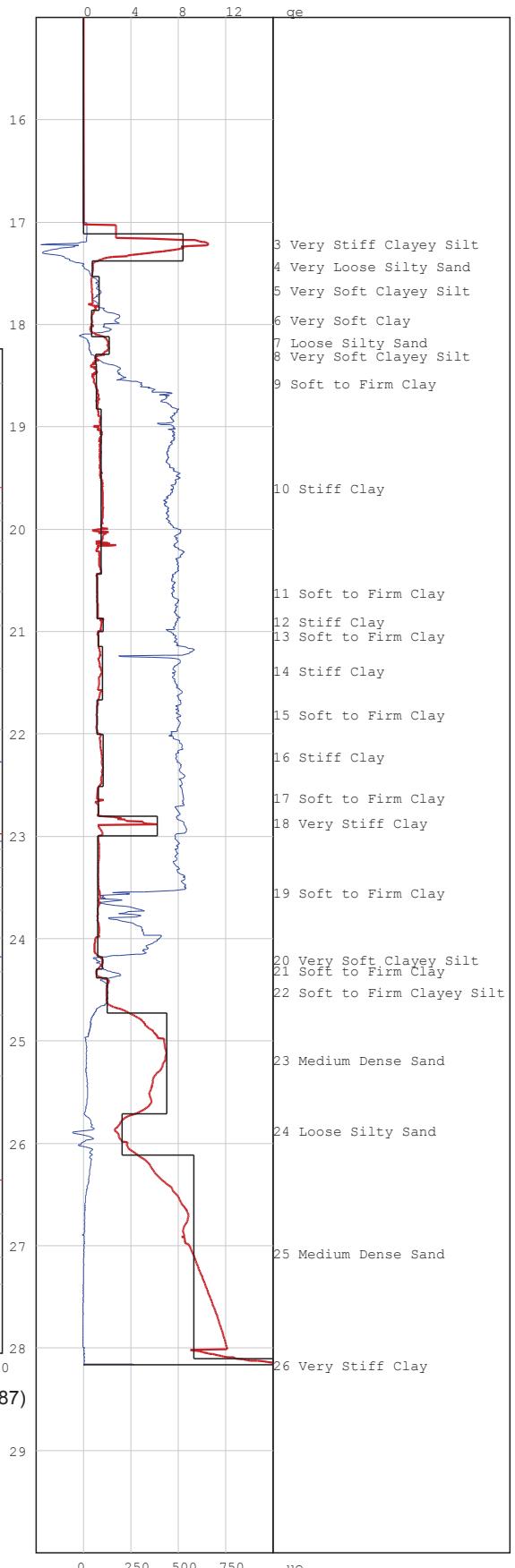
Hole No: CPT03

Date: 2012-06-18

Probe: GHANA



Jones and Rust Soils ID chart modified by Vermeulen and Rust(A.C.Meigh-CIRIA,1987)



PIEZOCONE PENETRATION TEST

PROJECT: Durban Harbour Over Water

SITE: Pier2

CPTU (RE): CPT04

DATE: 2012-06-21

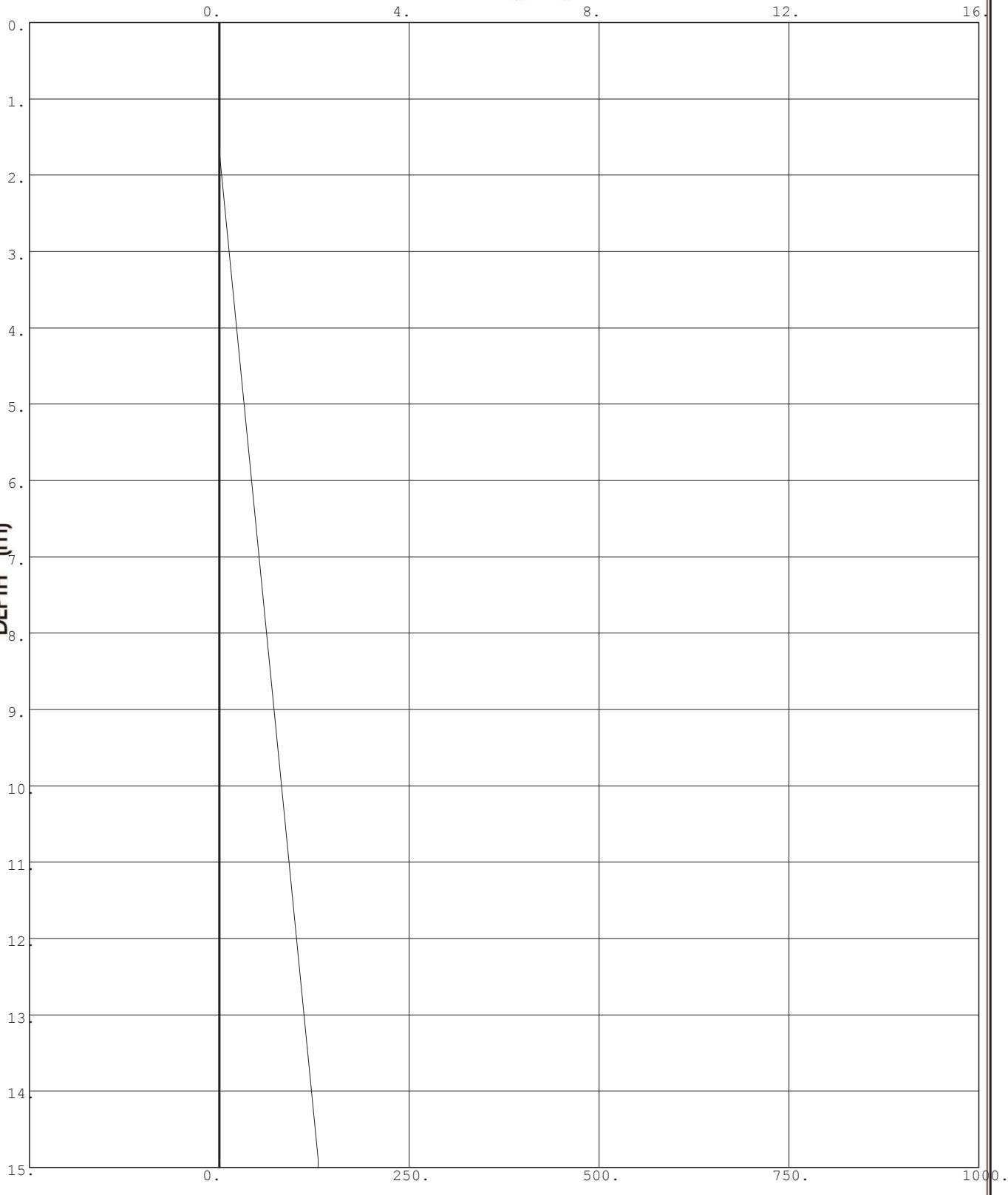
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Page:1

Probe: GHANA

CONE RESISTANCE (MPa)



PORE PRESSURE (kPa)

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PIEZOCONE PENETRATION TEST

PROJECT: Durban Harbour Over Water

SITE: Pier2

CPTU (RE): CPT04

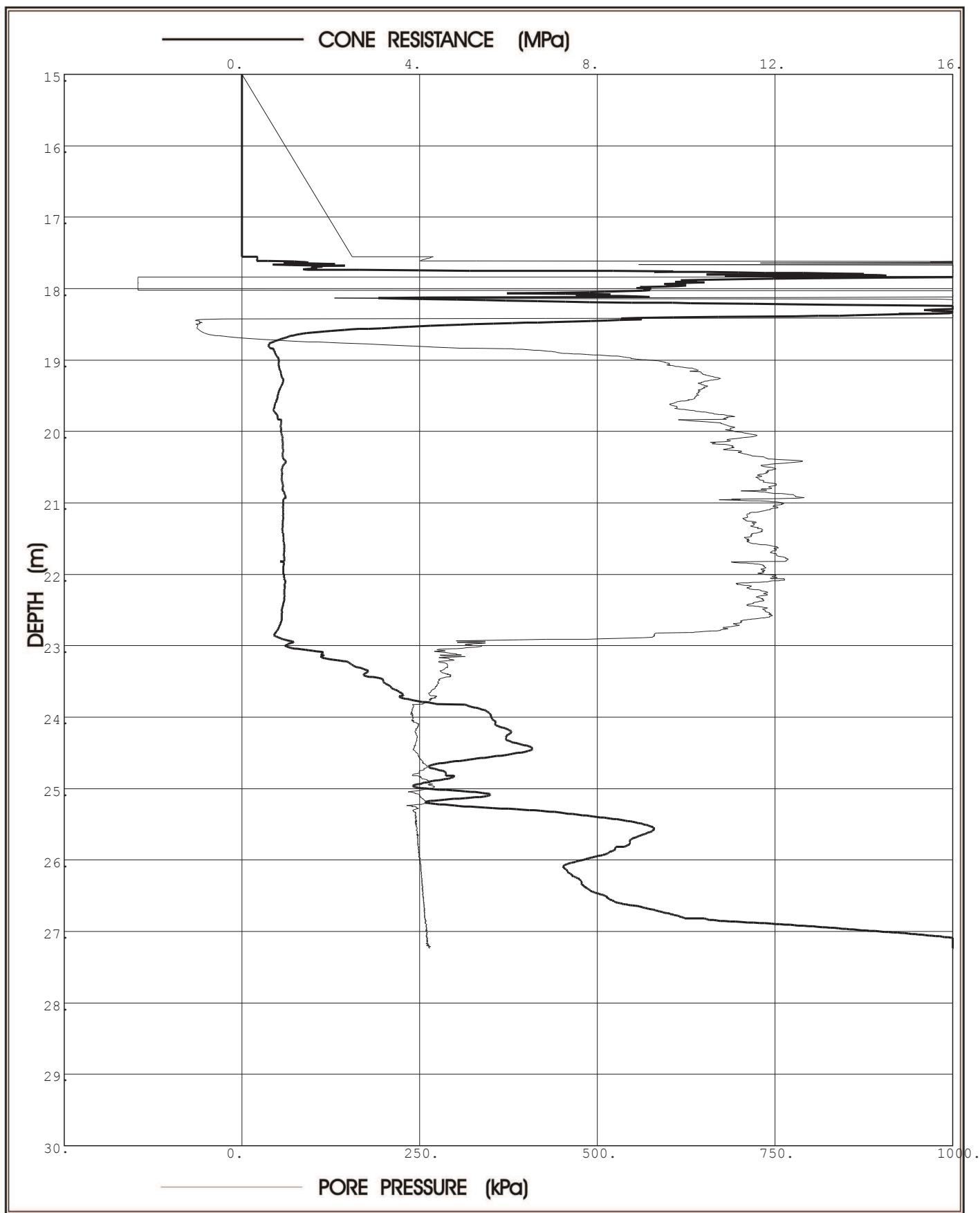
DATE: 2012-06-21

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Page: 2

Probe: GHANA



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Project: Durban Harbour Over Water

Site: Pier2

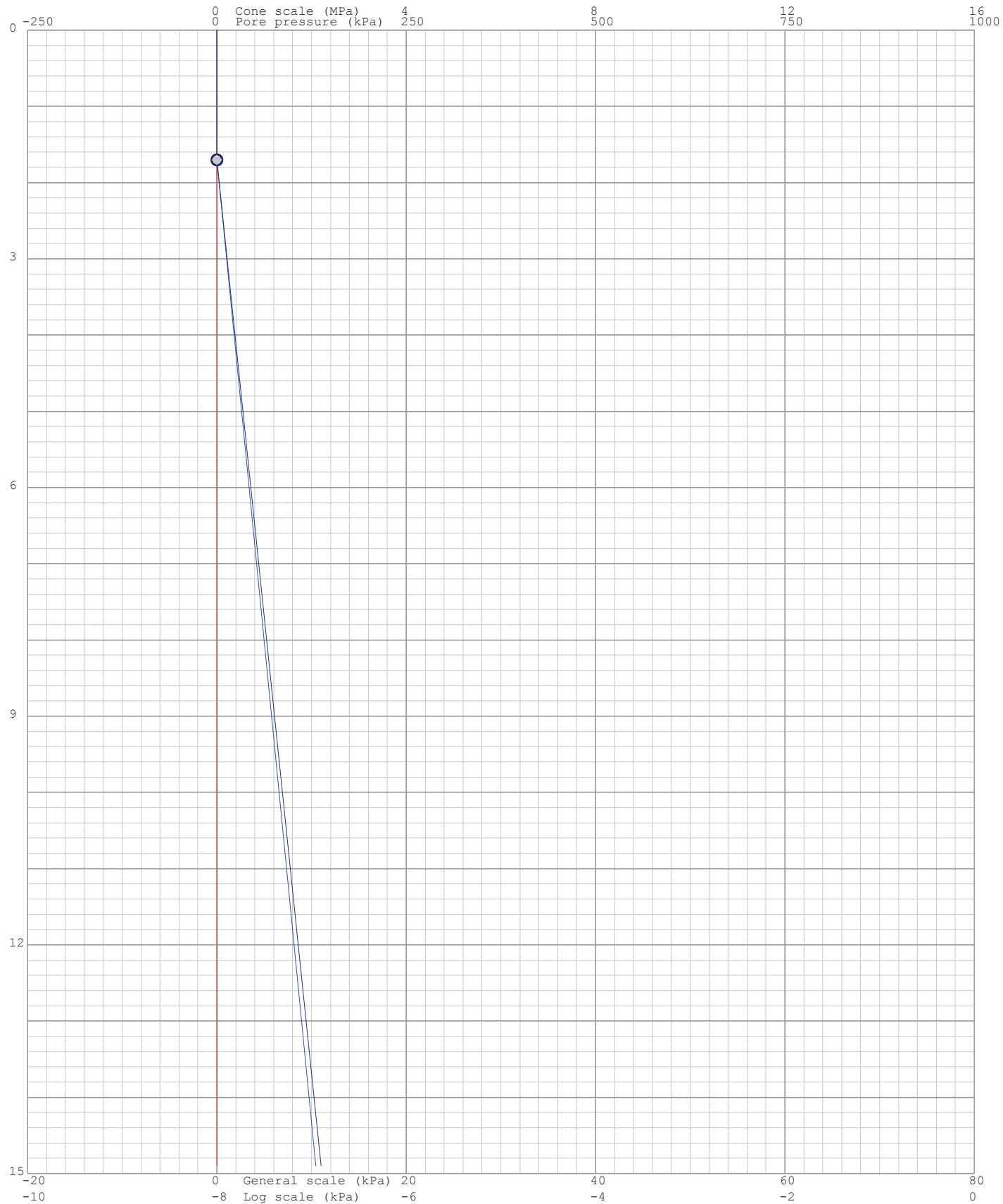
Hole No: CPT04

Date: 2012-06-21

Probe: GHANA

Page:1

qc (Cone Scale)
ut (Pore Pressure Scale)
uo (Pore Pressure Scale)
Diss uo (Pore Pressure Scale)



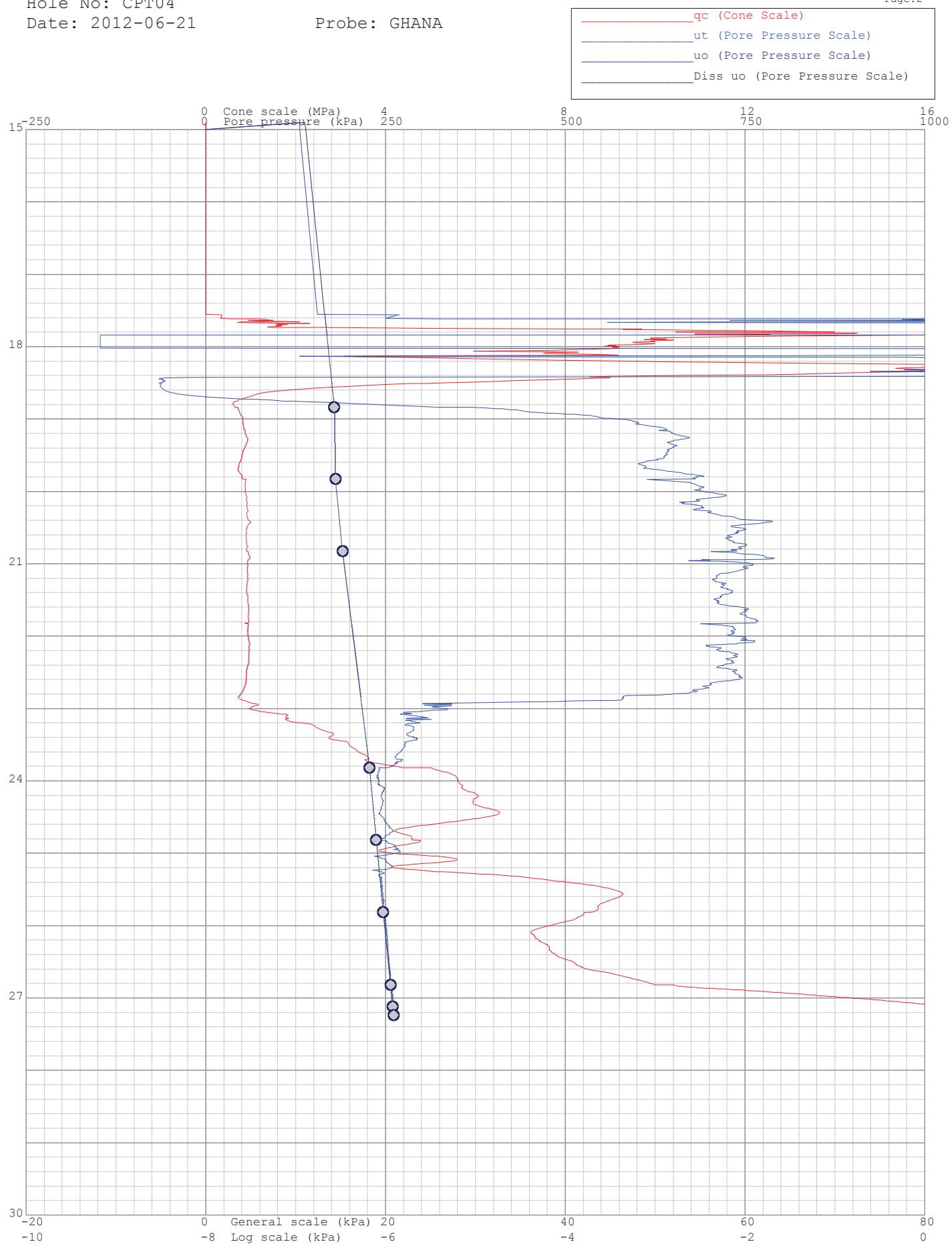
Project: Durban Harbour Over Water

Site: Pier2

Hole No: CPT04
Date: 2012-06-21

Probe: GHANA

Page: 2



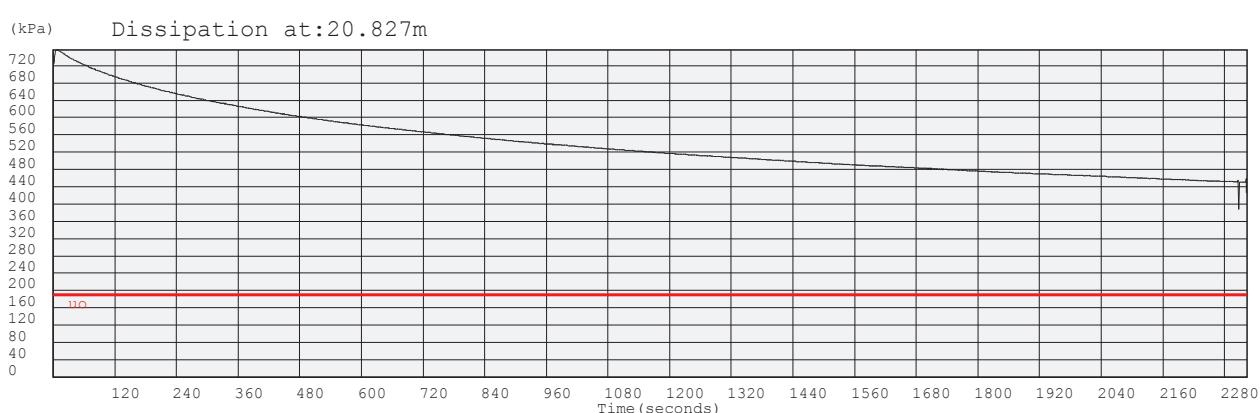
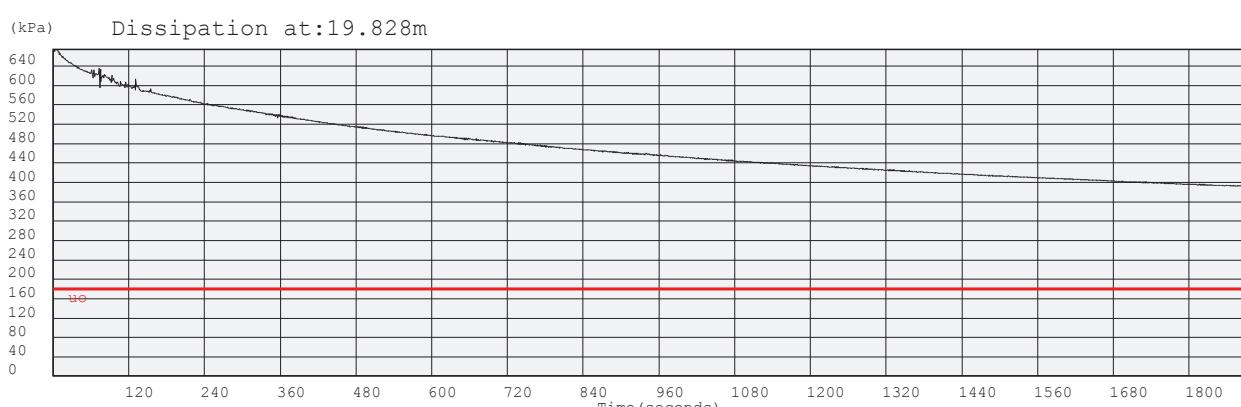
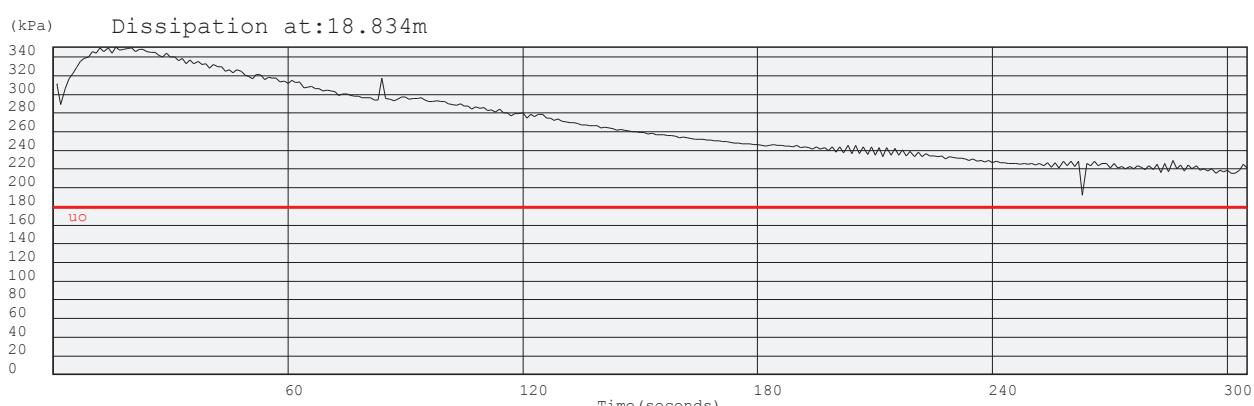
Project: Durban Harbour Over Water

Site: Pier2

Hole: CPT04

Date: 2012-06-21

Probe: GHANA
Page: 1



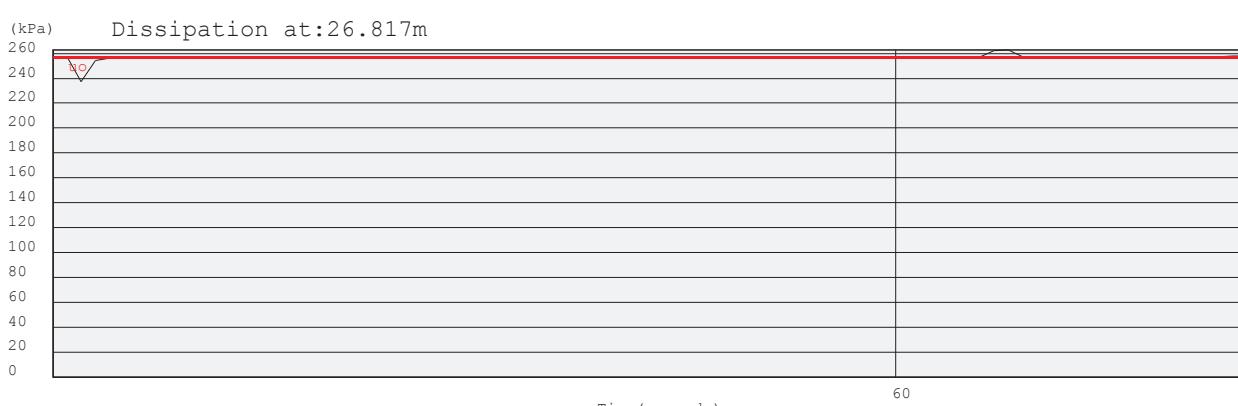
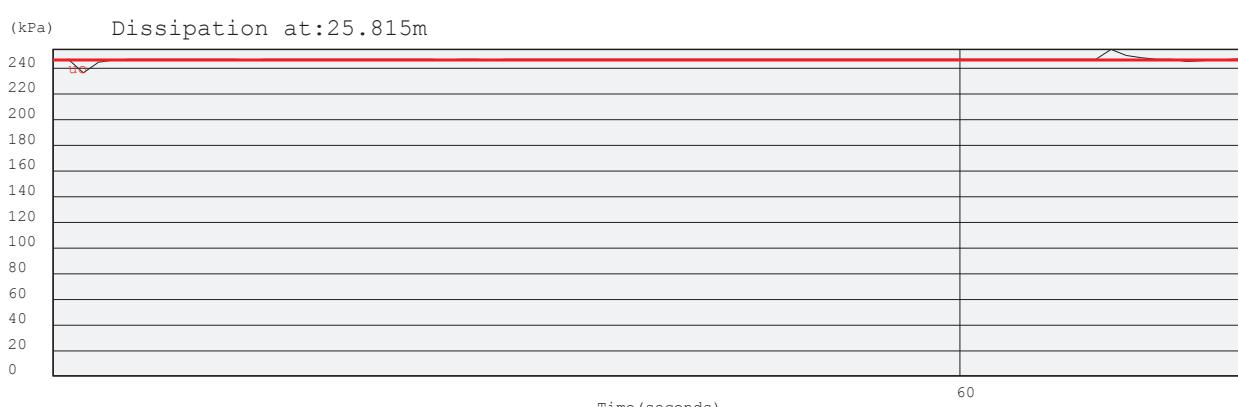
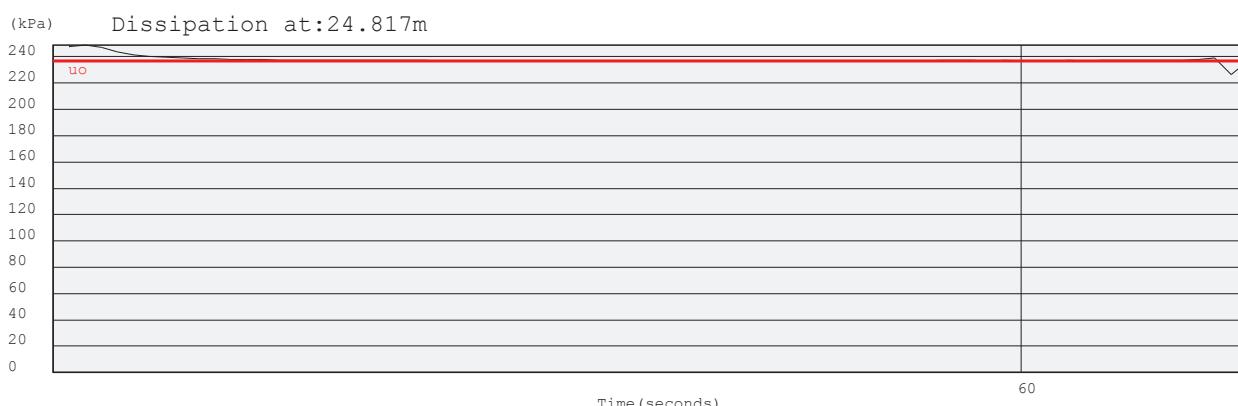
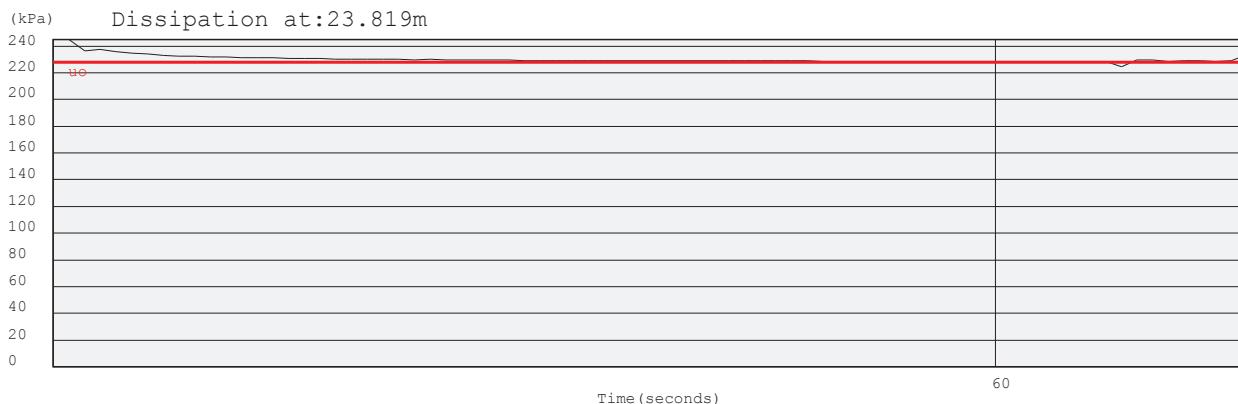
Project: Durban Harbour Over Water

Site: Pier2

Hole: CPT04

Date: 2012-06-21

Probe: GHANA
Page: 2



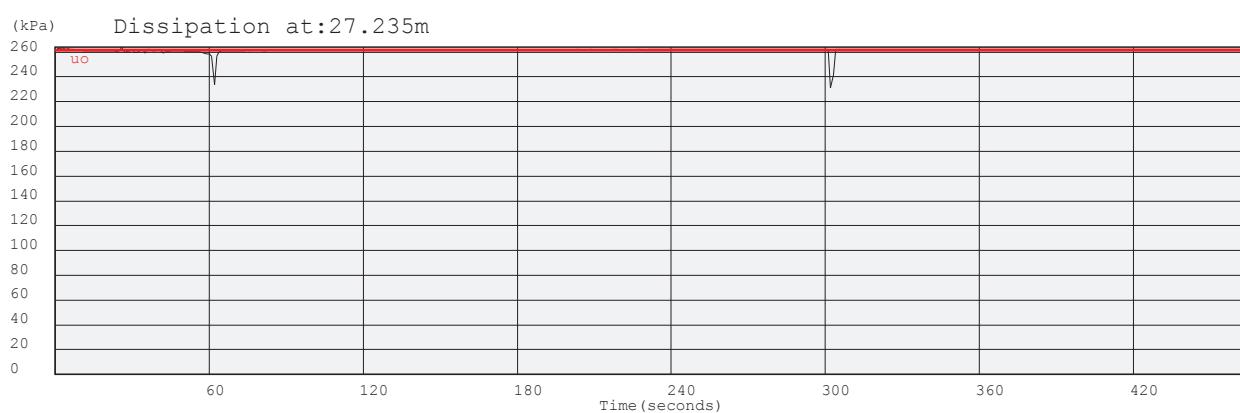
Project: Durban Harbour Over Water

Site: Pier2

Hole: CPT04

Date: 2012-06-21

Probe: GHANA
Page: 3



Project: Durban Harbour Over Water

Site: Pier2

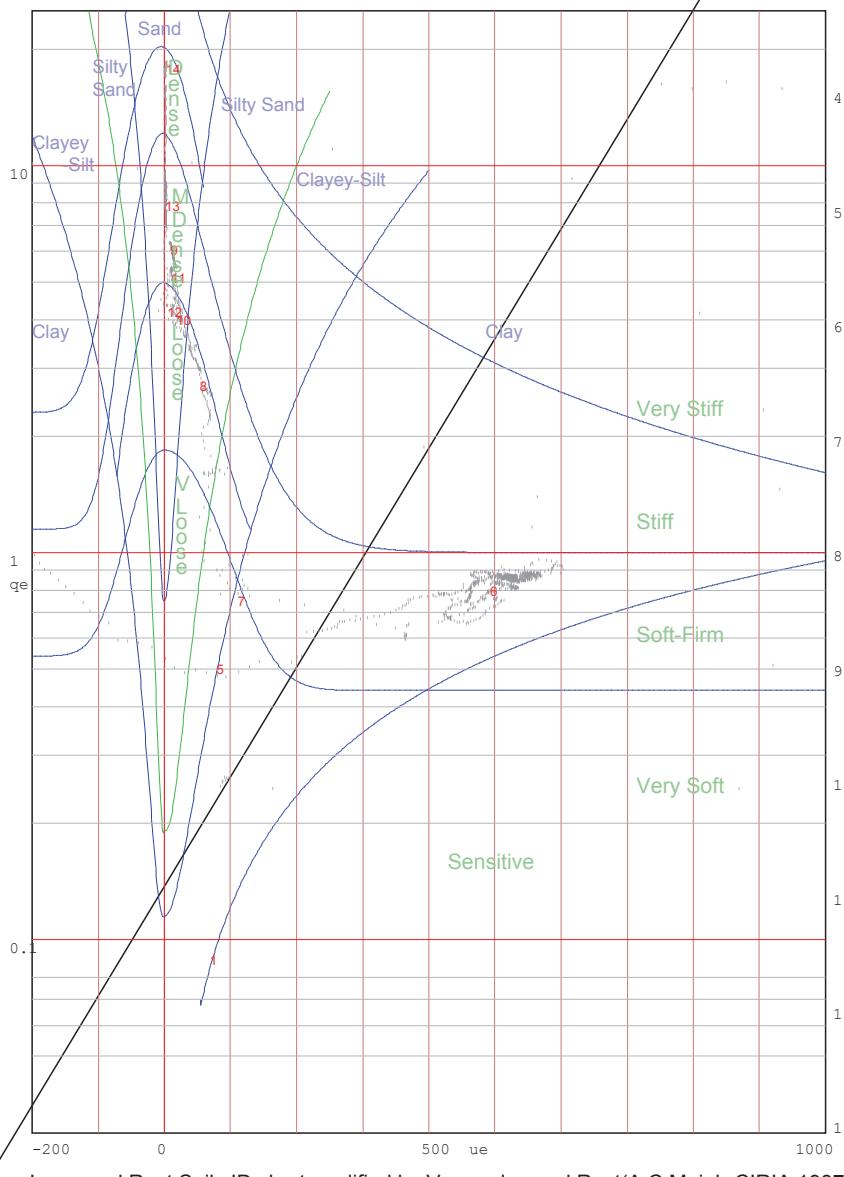
Hole No: CPT04

Date: 2012-06-21

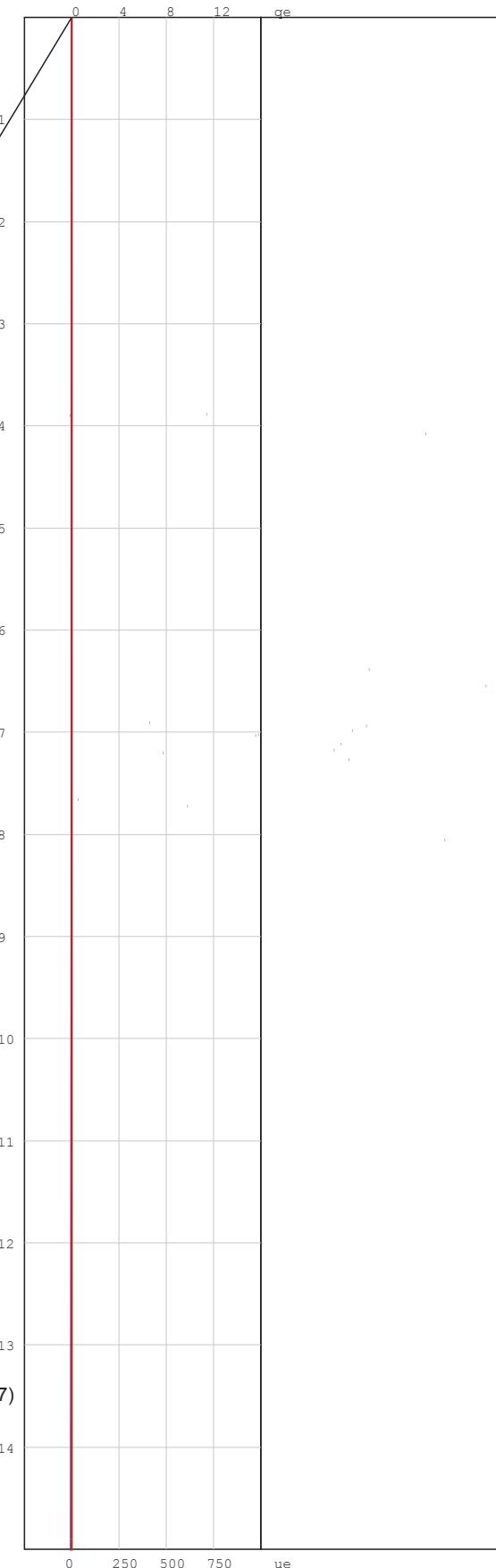
Probe: GHANA

Page:1

$q_e \text{ (MPa)} = q_t - s_{vo}$
$u_e \text{ (kPa)} = u_t - u_o$
Layers
1, 2, 3, ... Layer Number
Position on Soils ID chart



Jones and Rust Soils ID chart modified by Vermeulen and Rust(A.C.Meigh-CIRIA,1987)



Project: Durban Harbour Over Water

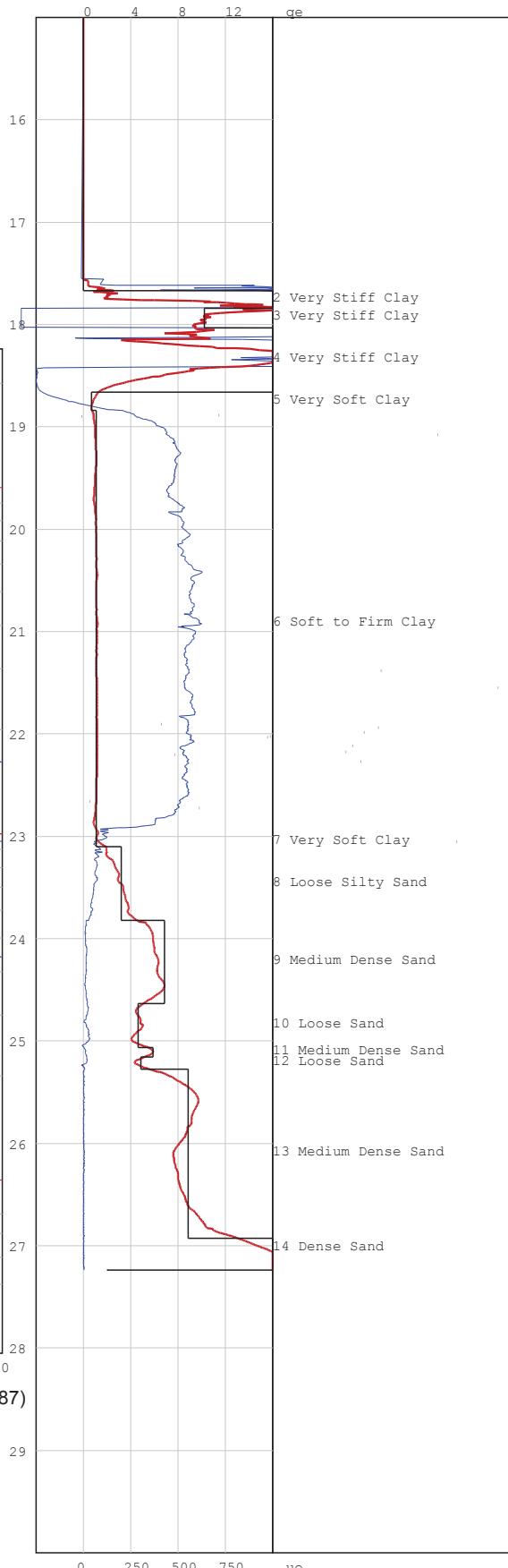
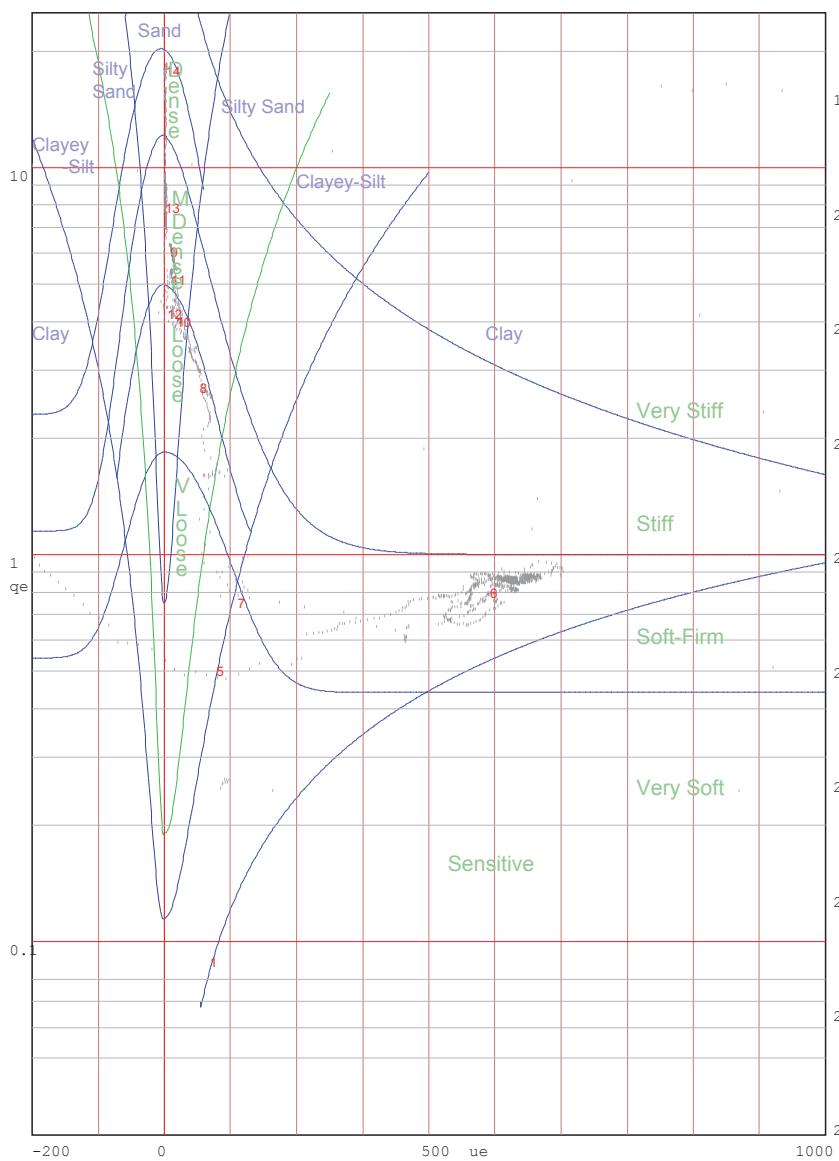
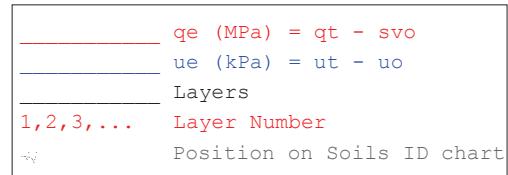
Site: Pier2

Hole No: CPT04

Date: 2012-06-21

Probe: GHANA

Page:2



PIEZOCONE PENETRATION TEST

PROJECT: Durban Harbour Over Water

SITE: Pier2

CPTU (RE): CPT05

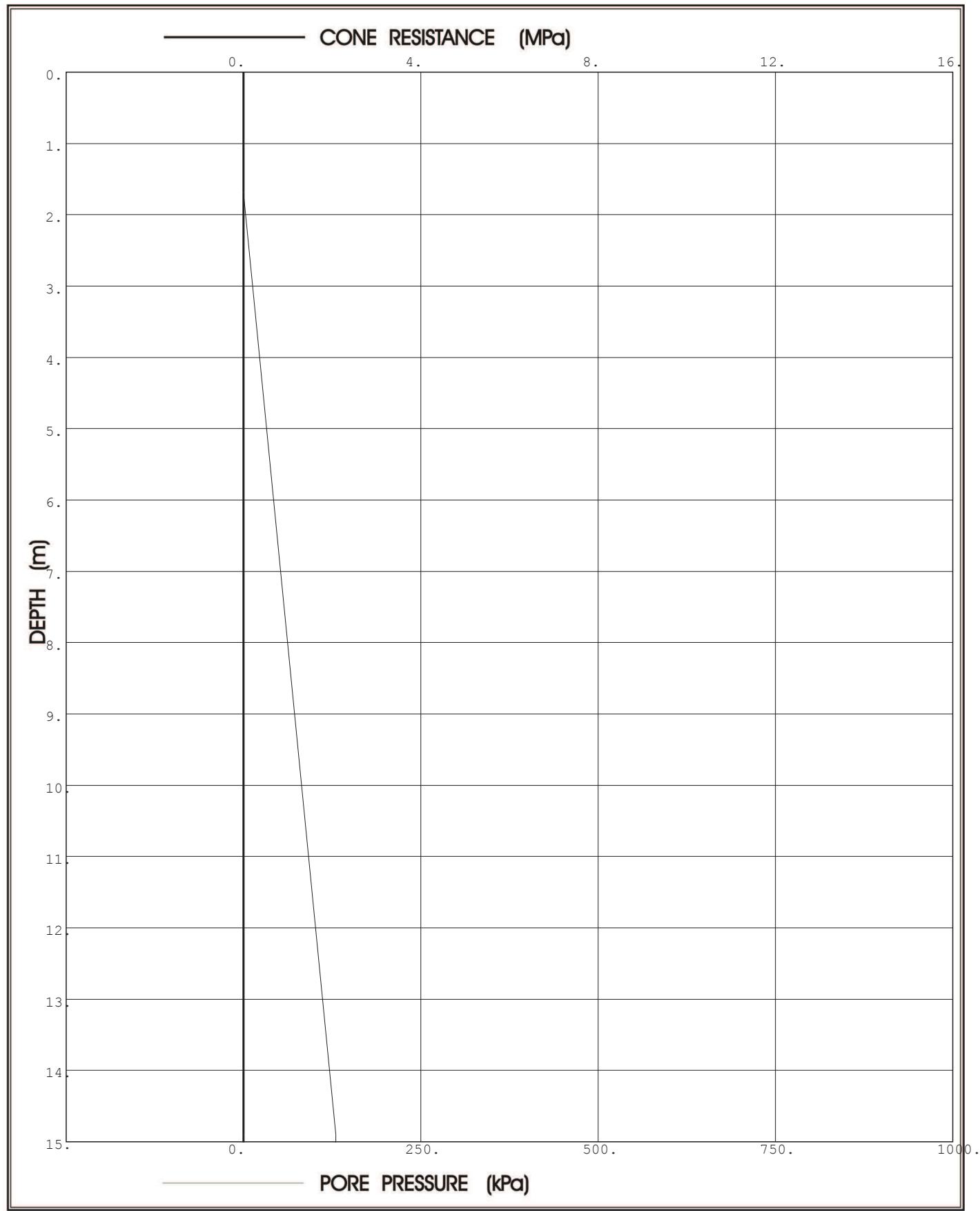
DATE: 2012/06/26

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Page:1

Probe: GHANA



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PIEZOCONE PENETRATION TEST

PROJECT: Durban Harbour Over Water

SITE: Pier2

CPTU (RE): CPT05

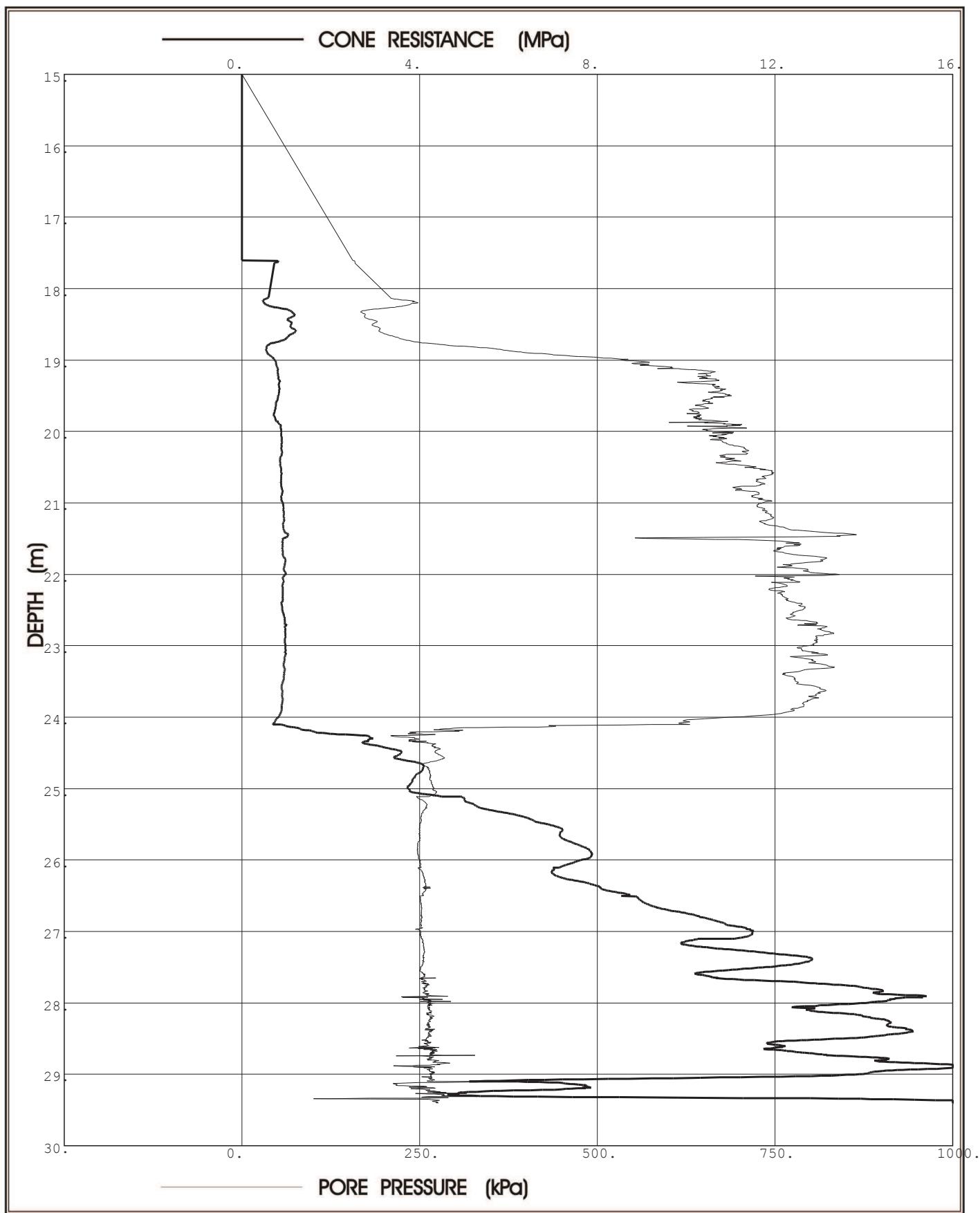
DATE: 2012/06/26

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Page: 2

Probe: GHANA



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Project: Durban Harbour Over Water

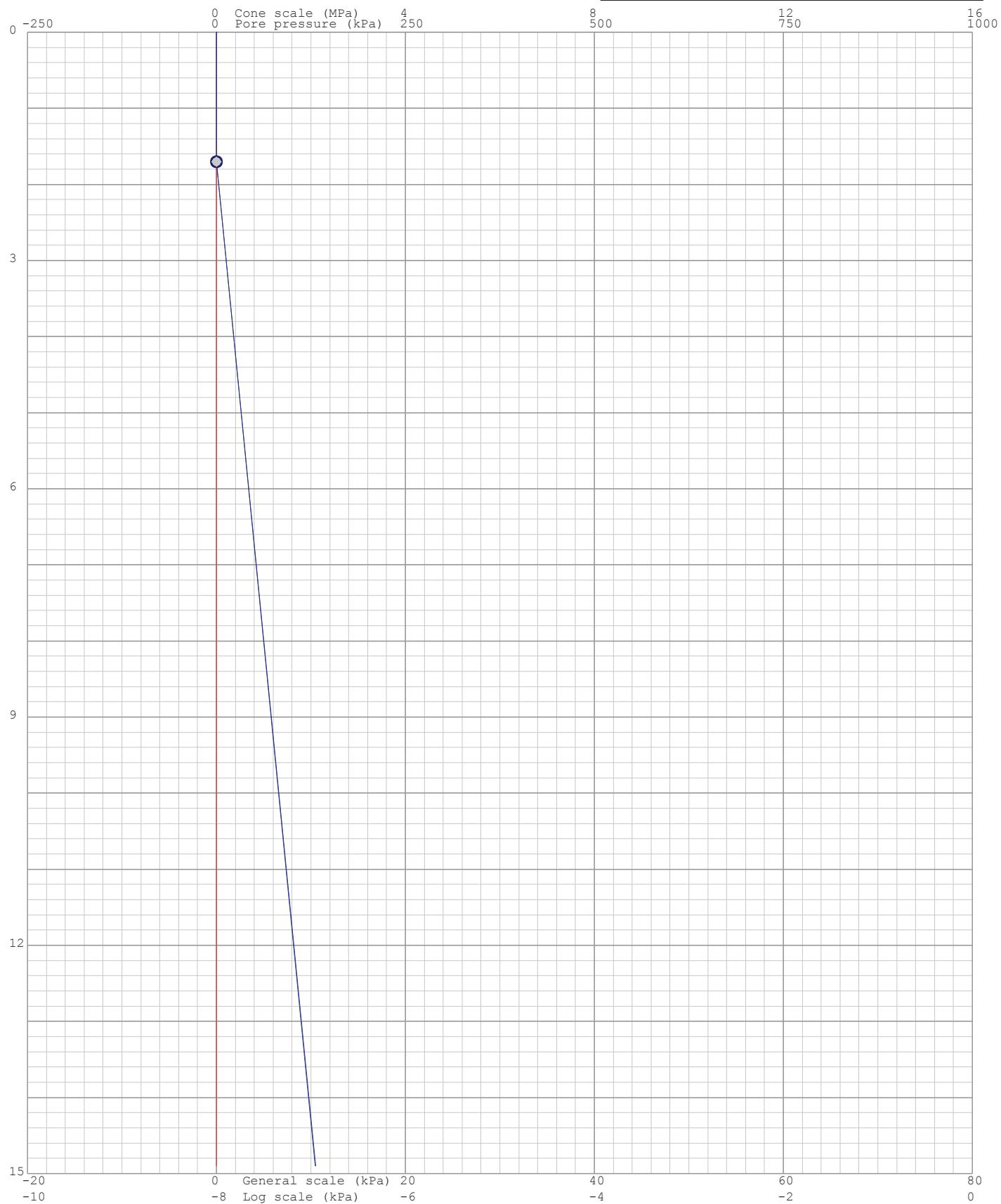
Site: Pier2

Hole No: CPT05
Date: 2012/06/26

Probe: GHANA

Page:1

qc (Cone Scale)
ut (Pore Pressure Scale)
uo (Pore Pressure Scale)
Diss uo (Pore Pressure Scale)



Project: Durban Harbour Over Water

Site: Pier2

Hole No: CPT05
Date: 2012/06/26

Probe: GHANA

Page: 2

